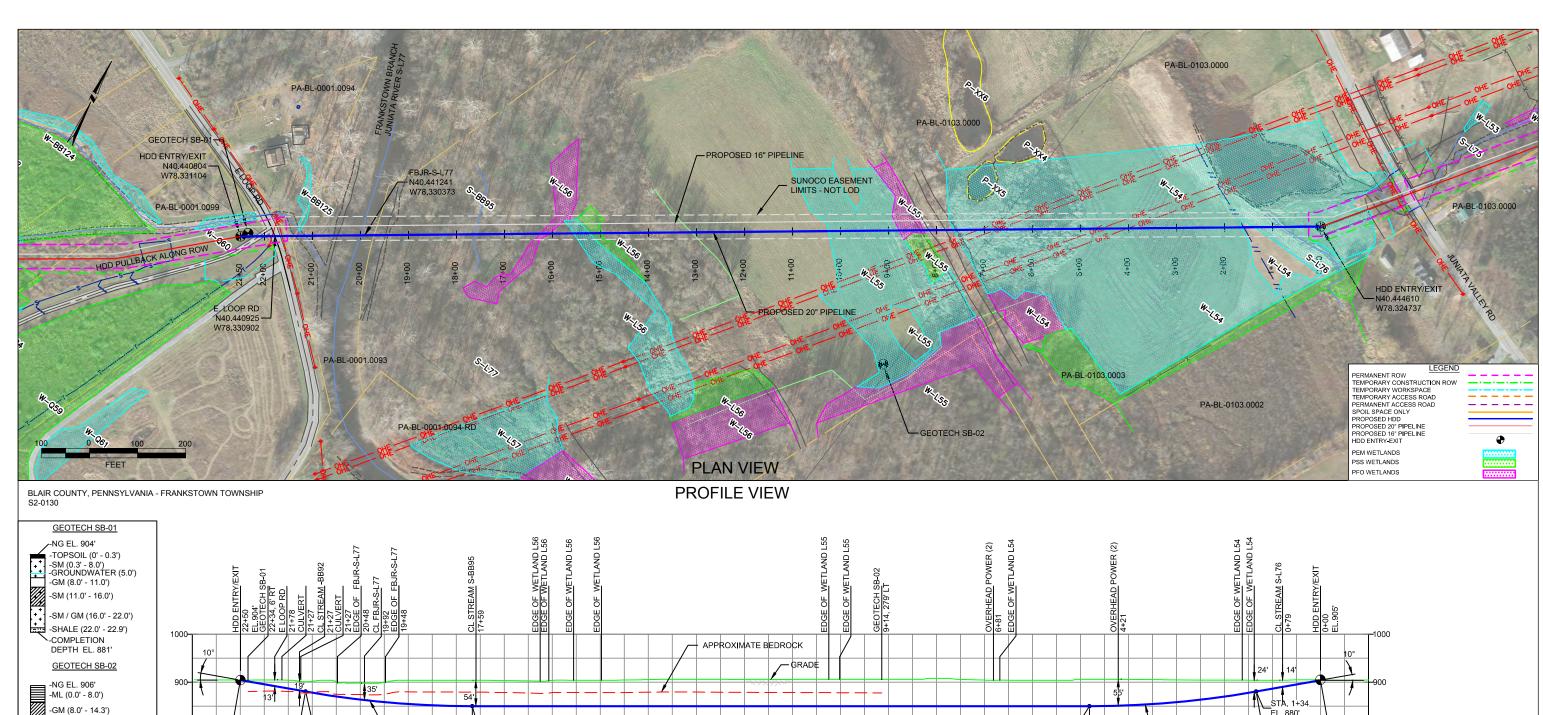
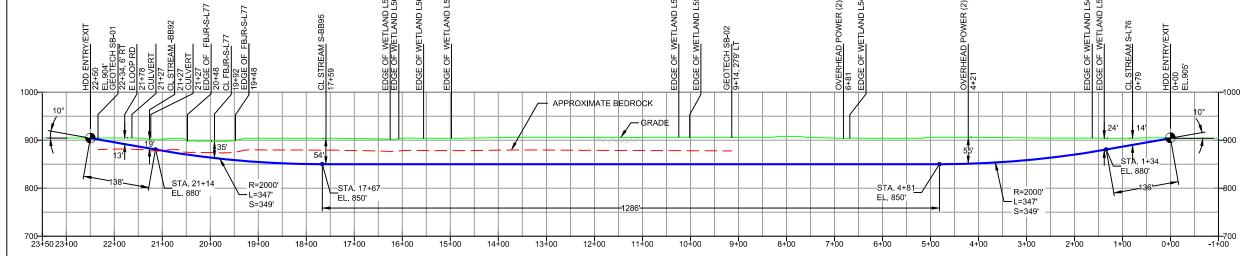
HDD PA-BL-0001.0094-WX (S-BB92) (S-L77) (S-BB95) (W-L56) (W-L55) (W-L54) (S-L76)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be *medium*. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 130 feet from the edge of the western most boundary of the stream S-BB92. The drill will travel beneath stream S-BB92 for 7 feet. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be decomposed shale, weathered to a silty fine to medium sand and gravel. The drill will continue beneath the eastern most boundary of the stream S-BB92 and will travel 110 feet from the eastern most edge of stream S-BB92 to the western most edge of stream S-L77. The drill will pass 55 feet under the stream S-L77. The majority of the substrate that will be passed through is estimated to be partially weathered shale. The drill will travel from the eastern most edge of stream S-L77 for 180 feet to the western most boundary of stream S-BB95. The drill will pass 8 feet under stream S-BB95. The majority of the substrate that will be passed through is estimated to be partially weathered shale. The drill will travel from the eastern most edge of stream S-BB95 for 130 feet to the western most boundary of wetland W-L56. The drill will travel 130 feet under wetland W-L56. The majority of the substrate that will be passed through is estimated to be partially weathered shale. The drill will travel from the eastern most edge of stream W-L56 for 450 feet to the western most boundary of wetland W-L55. The drill will travel 200 feet under wetland W-L55. The majority of the substrate that will be passed through is estimated to be partially weathered shale. The drill will travel from the eastern most edge of stream W-L55 for 60 feet to the western most boundary of wetland W-L54. The drill will travel 685 feet under wetland W-L54. The majority of the substrate that will be passed through is estimated to be partially weathered shale to highly fractured gray shale. The drill will travel under stream S-L76 for 7 feet. The majority of the substrate that will be passed through is estimated to be partially weathered shale to highly fractured gray shale. The drill will continue to travel for 96 feet under wetland W-L54 where it will enter/exit at the eastern most edge of wetland W-L54. The majority of the substrate that will be passed through is estimated to be highly fractured gray shale. The drill will travel 330 feet from the eastern most edge of stream W-L54 where it will enter/exit.

The medium designation for risk of inadvertent returns is based on a few factors. The drill will be travelling through fractured shale with layers of medium sand and fine to coarse gravel above which may allow the drilling fluid to reach the surface. The eastern entry/exit of the drill is within the wetland L54 where the drilling fluid could permeate the surface more readily. As such it is recommended that additional inspection in the area surrounding the drill be in place to monitor for potential inadvertent returns along the drill.





+ -SM (14.3' - 20.0')

(20.0' - 28.0')

-COMPLETION **DEPTH EL. 878'**

NOTE: REFER TO TEST BORING LOG S2-0130 FOR COMPLETE SOIL MATERIAL DESCRIPTION

-FRACTURED SHALE

- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
- THIS DRAWING.

 2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS
 THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPEL INF
- DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- DESIGNED IN ACCOMDANCE WITH CFR 49 199 & A CROSSING PIPE SPECIFICATION: HDD HORZ, LENGTH (L=):2250' HDD PIPE LENGTH (S=):2258' 20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50). INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
- PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.

- STREAM CAYOSINGS.

 CARRIER PIPE NOT ENCASED.

 PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.

 CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- 12. SUNOCO PIPELINE. L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN
- WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

				COATING: 14-16 MILS FE		30-35 MIL ARO (POWERCRETE R95)						
NOTES			REF. DRA	AWING		REVISIONS						
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-3.44	то	ES-3.46	EROSION & SEDIMENT PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	DLM	09/30/16	RMB	09/30/16	AAW	09/30/16
STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 29	то	SHEET 29	AERIAL SITE PLAN	EP1	REVISED PER PADEP COMMENTS	DLM	05/18/16	MRS	05/18/16	AAW	05/18/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING. INC. AND SUNOCO PIPELINE.					EP		MRS	11/23/15	RMB	11/23/15	AAW	11/23/15
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					С	ADDED GEOTECH INFO	MRS	09/03/15	RMB	09/03/15	AAW	09/03/15
4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.					В	ISSUED FOR BID	JAM	07/31/15	RMB	07/31/15	AAW	07/31/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR REVIEW	RTT	3/19/15	RMB	3/19/15	AAW	3/19/15
	DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE



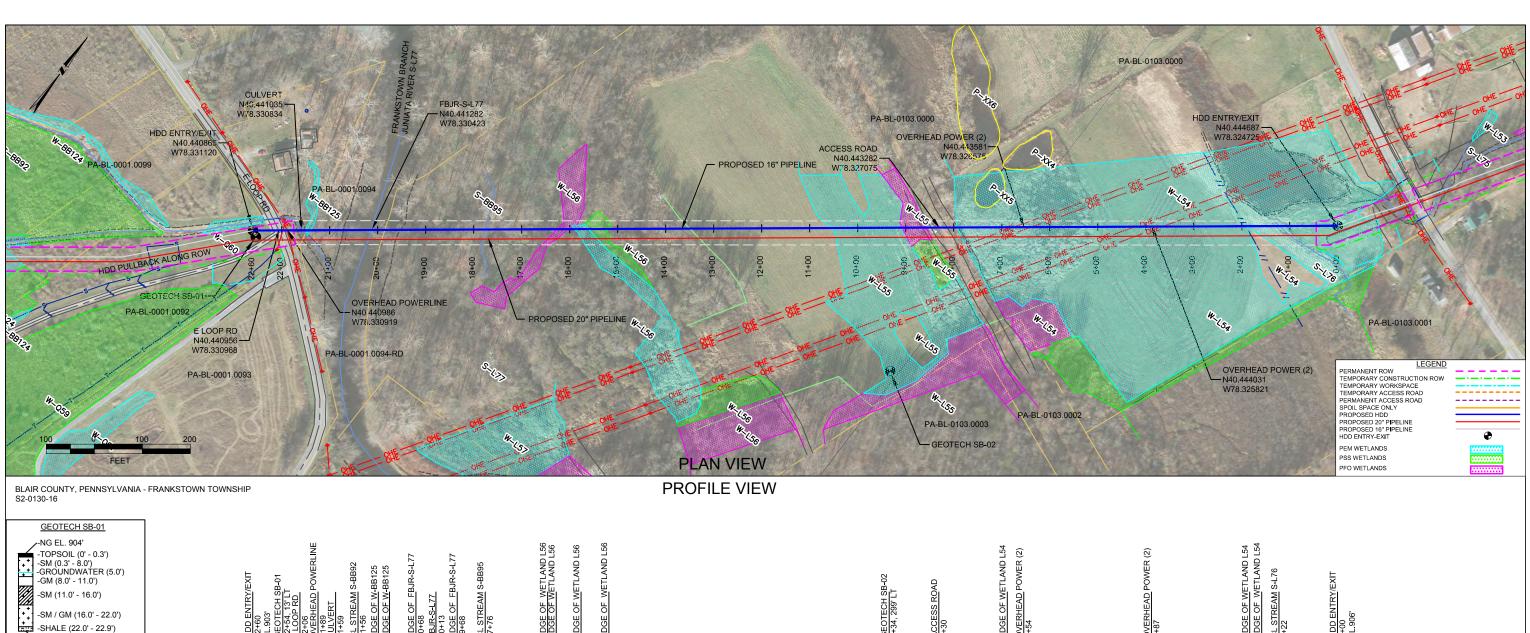
(303) 792-5911

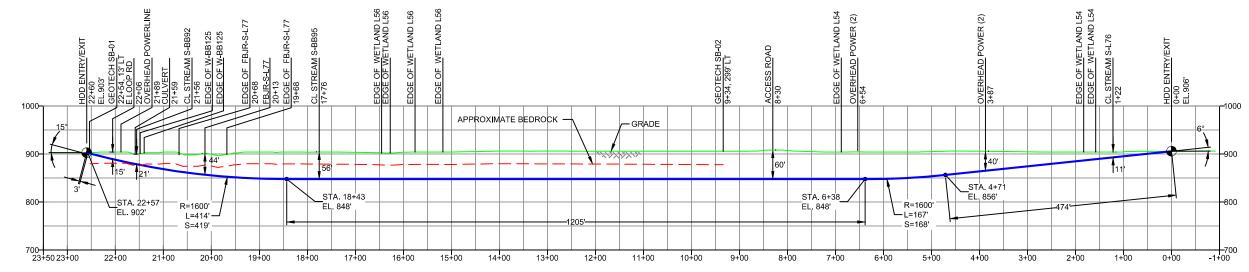
TETRA TECH ROONEY

20-INCH HORIZONTAL DIRECTIONAL DRILL FRANKSTOWN BRANCH JUNIATA RIVER PENNSYLVANIA PIPELINE PROJECT

SUNOCO PIPELINE, L.P.

DWG. NO: PA-BL-0001.0094-WX SCALE: 1"=200'





-COMPLETION

DEPTH EL. 881'

-SM (14.3' - 20.0')

(20.0' - 28.0')

COMPLETION DEPTH EL. 878' NOTE: REFER TO TEST BORING LOG <u>\$2-0130</u> FOR COMPLETE SOIL MATERIAL DESCRIPTION

-FRACTURED SHALE

GEOTECH SB-02 -NG EL. 906' -NG EL. 906' -ML (0.0' - 8.0') -GM (8.0' - 14.3' -GM (8.0' - 14.3')

- DESIGN AND CONSTRUCTION:

 1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.

 2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED
- DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4

DESIGNED IN ACCORDANCE WITH OFF AS 150 AND CROSSING PIPE SPECIFICATION:
HDD HORZ, LENGTH (L=):1786'
HDD PIPE LENGTH (S=):1794'
16' x 0.438' W.T., X-70, APISL, PSL2, ERW, BFW

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50).
 INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM GROSSINGS.
 CARRIER PIPE NOT ENCASED.
 DIREC AMBIENT TEMBERATURE MIST BE NOTESS THAN 30°E DUBING BUILD ROCK WITHOUT DRIPED.
- PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR

- WRITTEN APPROVAL FROM THE ENGINEER

 CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

				COATING: 14-16 MILS FE	BE WITH	30-35 MIL ARO (POWERCRETE R95)						
NOTES			REF. DR	AWING		REVISIONS						
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-3.44	то	ES-3.46	EROSION & SEDIMENT PLAN	EP2	REVISED PER PADEP COMMENTS RECIVED 09-06-16	DLM	10/07/16	RMB	10/07/16	AAW	10/07/16
STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 29	то	SHEET 29	AERIAL SITE PLAN	EP1	REVISED PER PADEP COMMENTS	DLM	05/18/16	RMB	05/18/16	AAW	05/18/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE.					EP		MRS	03/15/16	RMB	03/15/16	AAW	03/15/16
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					С	REDESIGN	MRS	10/08/15	RMB	10/08/15	AAW	10/08/15
CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.					В	ADDED GEOTECH INFO	MRS	09/03/15	RMB	09/03/15	AAW	09/03/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR BID	MRS	08/31/15	RMB	08/31/15	AAW	08/31/15
	DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE

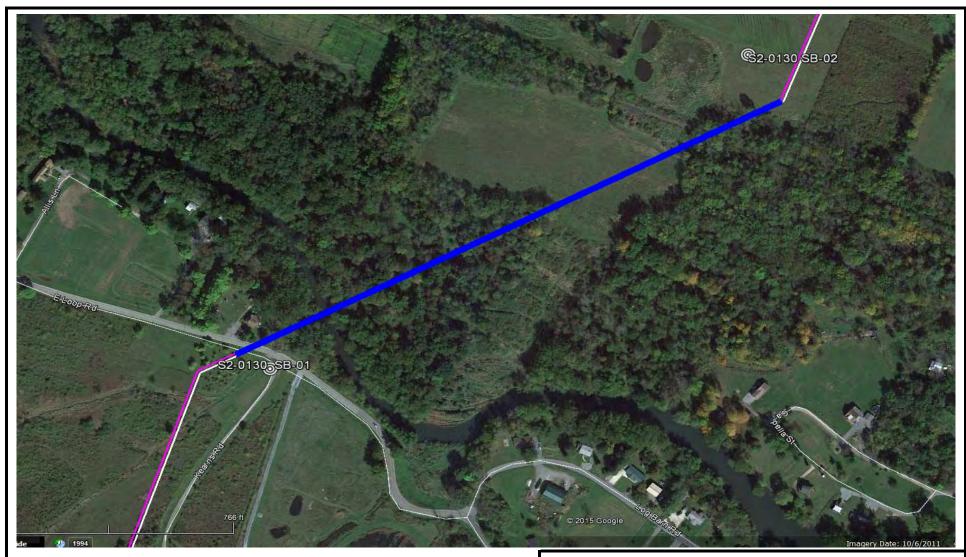


SUNOCO PIPELINE, L.P.

TETRA TECH ROONEY (303) 792-5911

16-INCH HORIZONTAL DIRECTIONAL DRILL FRANKSTOWN BRANCH JUNIATA RIVER PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=200' DWG. NO: PA-BL-0001.0094-WX-16



LEGEND:

© Geotechnical Soil Boring (SB) Locations

TE TETRATECH

GEOTECHNICAL BORING LOCATIONS
HDD S2-0130 E LOOP ROAD
BLAIR COUNTY, FRANKSTOWN TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVAI	NIA PI	PELINE PROJECT		Project No.: 103IP3406
Project Location:	EAST LOOP ROAD, HO	LLIDA	YSBURG, PA		Page 1 of 1
HDD No.:	S2-0130		Dates(s) Drilled: 04-23-15	Inspector:	E. WATT
Boring No.:	SB-01		Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING		Groundwater Depth (ft): 5.0	Total Depth (ft):	22.9
Boring Location Coordin	nates:		40° 26' 27.043" N	78° 19' 51.865" V	V

Doming	Location	1 0001 011					40 20 27.043 N					
Sample	Sample I	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials	6" Ir	ncreme	nt Blo	ws *	N
No.	From	То	From	То	Re	(USCS)	·	5 II	. 5. 51110	2.0		L. .
			0.0	0.3			TOPSOIL (4")					
1	3.0	5.0	0.3		15	SM	VARIEGATED BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, WITH	3	12	14	13	26
				8.0		Olvi	A LITTLE FINE TO COARSE GRAVEL.					
2	8.0	10.0	8.0		9	CM	FINE TO COARSE QUARTZ GRAVEL WITH SOME FINE TO COARSE	6	10	7	7	17
				11.0		GM	SAND, AND SOME SILT.					
3	13.0	13.5	11.0		5	014	GRAY FINE TO MEDIUM SAND WITH SOME SILT, WITH A LITTLE	50/6"				>50
				16.0		SM	FINE TO COARSE GRAVEL. (WEATHERED SHALE).					
4	18.0	18.8	16.0		5	SM/	DECOMPOSED SHALE, WEATHERED TO A SILTY FINE TO MEDIUM SAND	35	50/4"			>50
				22.0		GM	AND GRAVEL.					
5	22.0	22.9	22.0	22.9	1		PARTIALLY WEATHERED SHALE.	4	50/5"			>50
							AUGUR REFUSAL AT 22'.					
							7,000 MEL OSAL MEL.					
							WET ON SPOON AT 5'.					
							WATER LEVEL THROUGH AUGERS AT 7'.					-
							CAVED AT 11', WATER LEVEL ON CAVE AT 2.5'.					-
												L

Notes/Comments:

Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:		SUNOCO PENN	SYLV	ANIA PI	IPELINE PROJECT		Project	t No.: 103IP3406	
Project Location:		JUNIATA VALLE	Y ROA	AD, HO	LLIDAYSBURG, PA		Page 1	l of 1	
HDD No.:		S2-0130			Dates(s) Drilled: 04-24-15	Inspector:	E. WA	TT	
Boring No.:		SB-02			Drilling Method: SPT - ASTM D1586	Driller:	S. HO	FFER	
Drilling Contractor:		HAD DRILLING			Groundwater Depth (ft): SEE BELOW	Total Depth (ft):	28.0		
Boring Location Co	ordin	ates:			40° 26' 37.589" N	78° 19' 32.863" \	V		
o . Sample Dent	h (ft)	Strata Denth (ft)	>	Strata					

Donnig	Location	i Coordii	iaics.				40 20 37.303 N					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" 1	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Ŗ.	(USCS)	Description of Materials	<u> </u>	TIOI CITIC	one bio	***	
			0.0	0.0			TOPSOIL (<1")					
1	3.0	5.0	0.0		20	ML	MOTTLED BROWN AND GRAY SILT AND FINE TO MEDIUM SAND.	1	4	6	11	10
				8.0		IVIL	(USCS: ML).					
2	8.0	10.0	8.0		13	GM	MULTICOLORED FINE TO COARSE GRAVEL WITH QUARTZ, AND	4	10	13	12	23
				14.3		Givi	MEDIUM TO COARSE SAND, WITH A LITTLE SILT.					
3	13.0	15.0	14.3		20		REDDISH BROWN AND GRAY DECOMPOSED SHALE, WEATHERED TO	2	7	8	11	15
						014	A SILTY FINE TO MEDIUM SAND, SOME F-C GRAVEL. (USCS: SM).					
4	18.0	18.8			8	SM	REDDISH BROWN AND GRAY DECOMPOSED SHALE, WEATHERED TO	11	50/5"			>50
				20.0			A SILTY FINE TO MEDIUM SAND, SOME F-C GRAVEL.					
							AUGER REFUSAL AT 20'.					
							ROCK CORING					
RUN 1	20.0	23.0	20.0		36	Щ	HIGHLY FRACTURED GRAY SHALE	TCR: 1	00%, SC	CR: 22%	, RQD:	0%
RUN 2	23.0	28.0		28.0	60	SHALE	HIGHLY FRACTURED GRAY SHALE	TCR: 1	00%, SC	CR: 17%	, RQD:	0%
								 				
								-				
								-				
								-				
								-				
								 				
								₩				
								—				
								—				
							WET ON SPOON AT 8'	-				
							WATER LEVEL THROUGH AUGERS AT 1'.	↓				
							CAVED AT 12'.	↓				

Notes/Comments:

Pocket Pentrometer Testing

S4: 2.75 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

N: Number of blows to drive spoon from 6" to 18" interval.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0130 E Loop Road

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of S	Sample (ft.)	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	5.0	11.6	20.2	-	-	-	-
	SB-01	2	8.0	10.0	18.3	24.3	-	-	-	-
	3D-01	3	13.0	13.5	9.6	22.8	-	-	-	-
S2-0130		4	18.0	18.8	5.7	28.7	NL	NP	NP	SM/GM
32-0130		1	3.0	5.0	16.3	53.3	33	25	8	ML
	SB-02	2	8.0	10.0	13.6	12.0	-	-	-	-
	36-02	3	13.0	15.0	11.4	42.1	30	24	6	SM
		4	18.0	18.8	14.0	34.8	-	-	-	-

Notes:

1) Sample depths based on feet below grade at time of exploration.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0130 E LOOP ROAD

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
£2 0120	CD O2	1	20	23	100	22	0	20	20	Hoovily		Very thin beds,	Light	Very broken up, nearly horizontal bedding;
S2-0130	SB-02	2	23	28	100	17	0	20	28	Heavily	Shale	Massive formation	gray	fractures along bedding surfaces, nearly horizontal

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0130 E Loop Road

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S2-0130	E. Loop		Brallier and Harrell Formations (undivided) - composed of interbedded light-gray, graded, siliceous siltstone	Valley Floor		The beds of <u>Brallier</u> are mostly thin; however, the shales may be very thick. The <u>Harrell</u> Formation consists of well-bedded, fissile to very thin, gray to	1,800 (Brallier)	11-30	Yields range from 5 to 20
32-0130	Rd.		beds and light-gray, hard, silty shales, sparsely fossiliferous.	valley Floor		black silty shale with thin argillaceous limestone, calcareous shale, and limestone nodules at the base.	100-200 (Harrell)	11-30	gpm

 $\underline{Note}: Source\ of\ well\ log\ data\ -\ http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm.\ All\ other\ sources\ as\ referenced\ in\ comments\ section.$

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	N (blows)*	Particle S	ize Identifica	tion
Very Loose	5 or less	Boulders	8 in. diame	
Loose	6 to 10			
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
very bense	51 01 more	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proporti	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20		(. /	(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	Site, ciay	2000 111011 0	110. 200 5.616 (10.07 11111)

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	Degree of Plasticity	Plasticity Index
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock					
Quality Designation	Quality <u>Descripti</u>					
(RQD), %	<u>on</u>					
0-25	Very Poor					
25-50	Poor					
50-75	Fair					
75-90	Good					
90-100	Excellent					

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions			Group Symbols	Typical Descriptions	Laboratory Classifications			
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM. GC, SM, SC 5 to 12 percent Borderline cases requiring dual symbols ⁽¹⁾	nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}} \text{ greater than 4:} C_{c=\frac{(D_{30})2}{D_{10} \times D_{60}}} \text{ between 1 and 3}$	
			GP	Poorly graded gravels, gravel- sand mixtures, little or no fines		ng dual syr	Not meeting C_u or C_c requirements for GW	
		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures		W, GP, SW, SP M. GC, SM, SC forderline cases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are borderline cases requiring use of dual symbols
			GC	Clayey gravels, gravel-sand-clay mixtures			Atterberg limits above A line with I p greater than 7	
	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	sw	Well graded sands, gravely sands, little or no fines			$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{L}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
			SP	Poorly graded sands, gravelly sands, little or no fines		Less than 5 More than 12 5 to 12	Not meeting C_u or C_c require	ments for SW
		Sands with fines (Appreciable amount of fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched
			SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	
Major Divisions		Group Symbols	Typical Descriptions		For soils p When w _{l.}	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and clays (Liquid limit less than 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60 A Line:			
		CL	Inorganic clays of low to medium plasticity, gravelly clays , sandy clays, silty clays, lean clays		PI = 0.73(LL - 20) U Line: PI = 0.9(LL - 8)			
		OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or
	Silts and Clays (Liquid limit greater than 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH
		СН	Inorganic clays of high plasticity, fat clays		Plasi		Character	
		ОН	Organic clays of medium to high plasticity, organic silts		7		ML or OL	0 70 80 90 100
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL	

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.