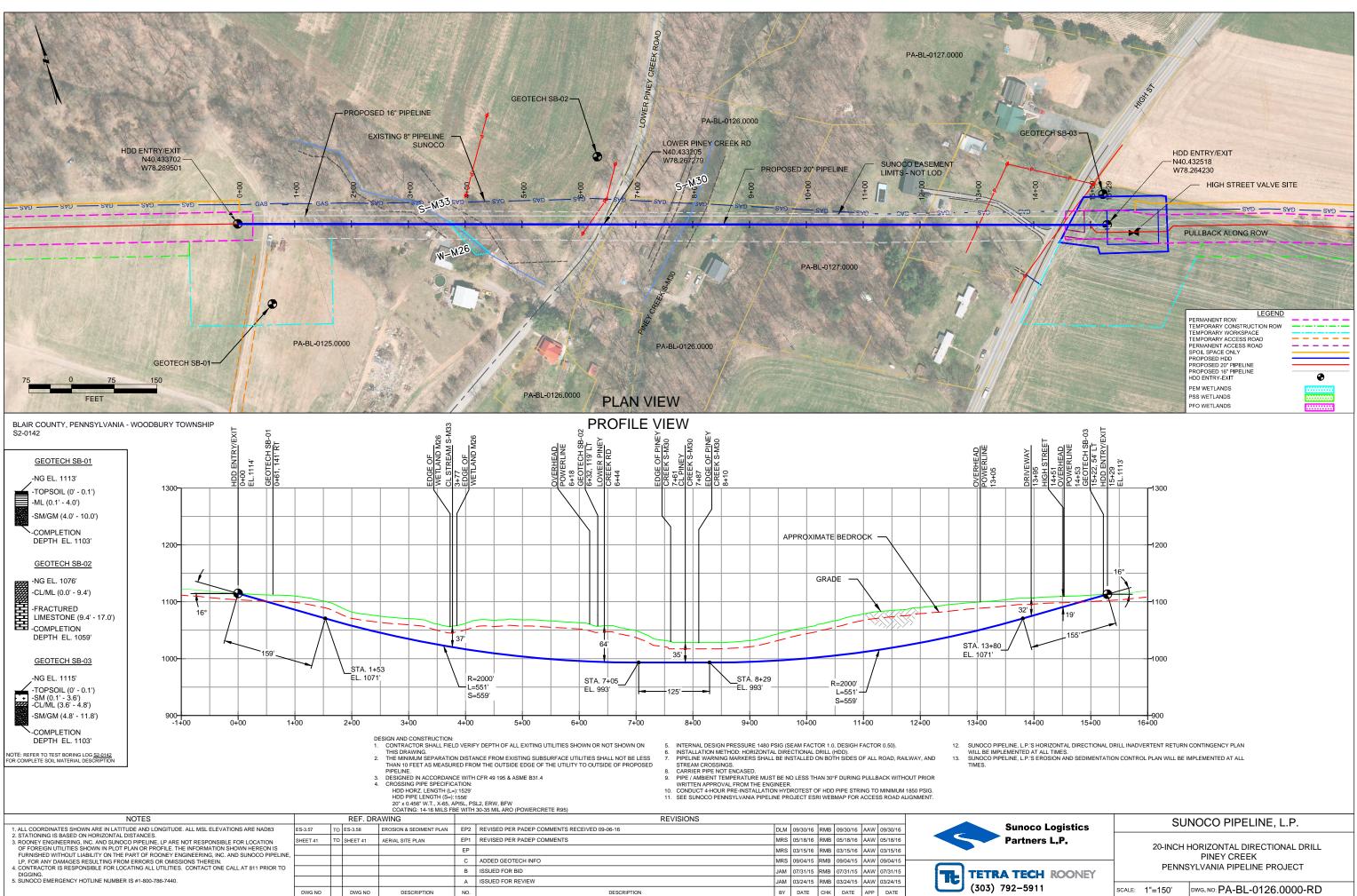
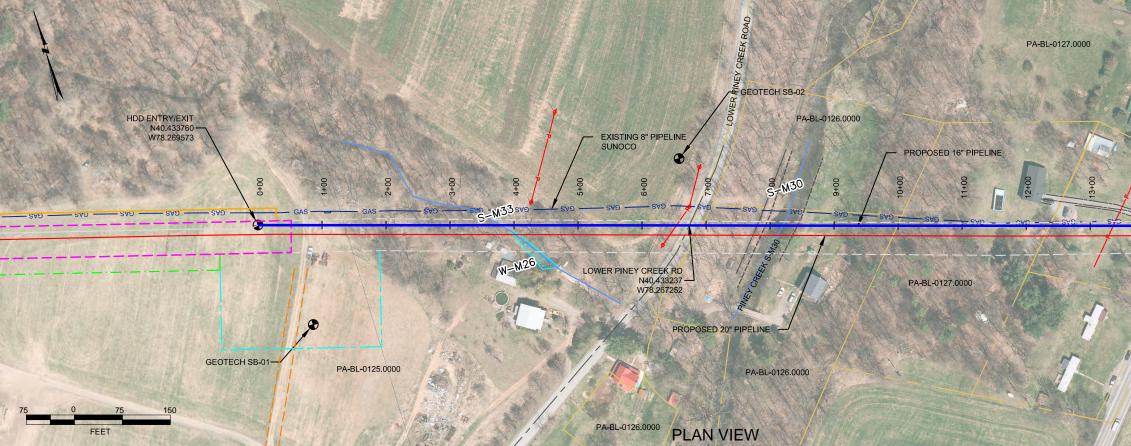
HDD PA-BL-0126.0000-RD (W-M26) (S-M33) (S-M30)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be *low*. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 400 feet from the edge of the western most boundary of the wetland W-M26. The drill will travel beneath W-M26 and stream S-M33 for 15 feet. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be fractured limestone. The drill will continue beneath the eastern most boundary of the wetland W-M26 and stream S-M33 and will travel 380 feet from the eastern most edge of wetland W-M26 and stream S-M33 to the western most edge of stream S-M30. The drill will pass 32 feet under the stream S-M30 starting at the western most boundary. The majority of the substrate that will be passed through is estimated to be fractured limestone. The drill will continue solution will enter/exit 580 feet from the eastern most edge of stream S-M30.



	SU	NOCO PIPELINE, L.P.
-		HORIZONTAL DIRECTIONAL DRILL PINEY CREEK SYLVANIA PIPELINE PROJECT
SCALE:	1"=150'	DWG. NO: PA-BL-0126.0000-RD
	SCALE:	20-INCH



A ISSUED FOR BID

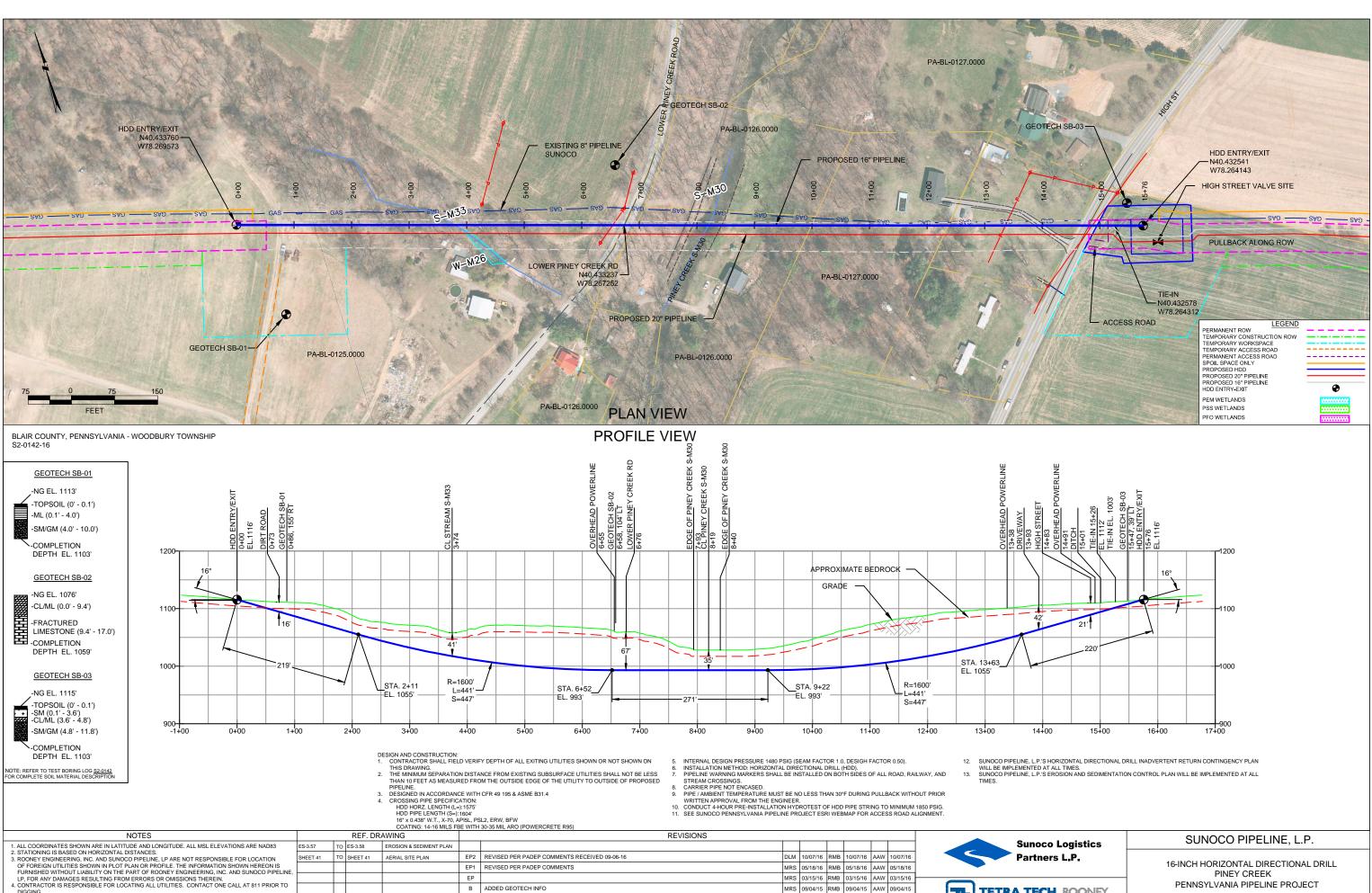
DESCRIPTION

NO.

DWG NO

DWG NO

DESCRIPTION



DIGGING. 5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

Tł TETRA TECH ROONEY (303) 792-5911

MRS 08/31/15 RMB 08/31/15 AAW 08/31/15

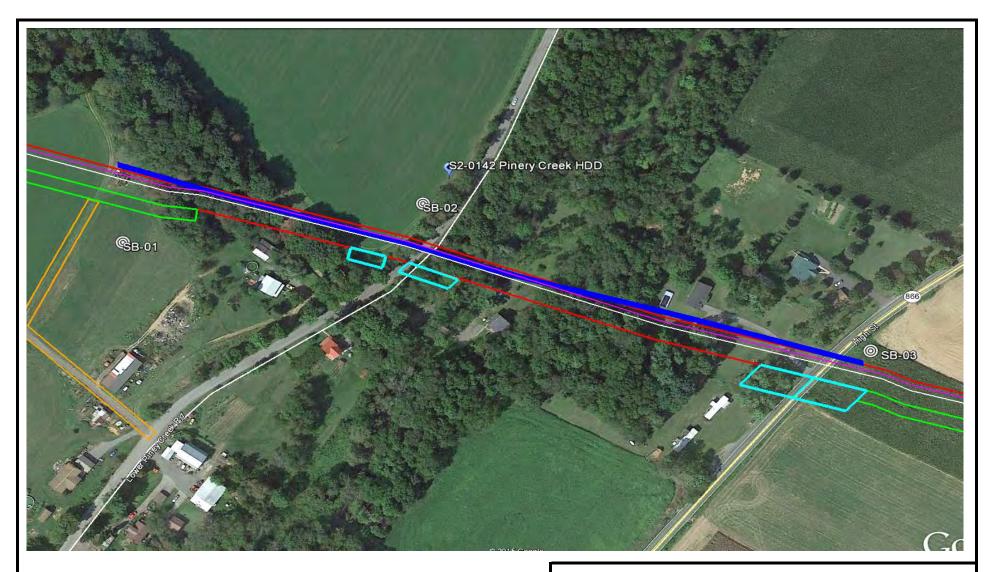
BY DATE CHK DATE APP DATE

noco	Log	istics
tner	s L.P	•

PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=150'

DWG. NO: PA-BL-0126.0000-RD-16



LEGEND:

(6) Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS HDD S2-0142 BLAIR COUNTY, WOODBURY TOWNSHIP, PA SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Nam Project Loca HDD No.: Boring No.: Drilling Cont Sample No. 1 3.0 1 3.0 2 8.0	ocation: ontractor: ample Depth (ft)	LOWER S2-0142 SB-01 HAD DR Strata D	2 PINEY (PELINE PROJECT D, WILLIAMSBURG, PA		Project N Page 1 c	of 1)3IP34	106		
HDD No.: Boring No.: Drilling Cont Sample No. Fror 1 3.0	o.: ontractor: ample Depth (ft)	S2-0142 SB-01 HAD DF Strata D	2	CREEK	ROAE		1						
Boring No.: Drilling Cont Sample Sam No. Fror 1 3.0	ontractor: ample Depth (ft)	SB-01 HAD DF Strata D				Data (a) Della de 04.44.45	1						
Drilling Cont Sample Sam No. Fror 1 3.0	ontractor: ample Depth (ft)	HAD DF Strata [RILLING			Dates(s) Drilled: 01-11-15	Inspector:	E. WATT					
Sample Sample No. From 1 3.0	ample Depth (ft)	Strata I	RILLING			Drilling Method: SPT - ASTM D1586	Driller:	S. HOFF	ER				
No. Fror 1 3.0		_		1	r	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	10.0					
		From	Depth (ft) To	Recov. (in)	Strata (USCS)	Description of Materi	als		6" In	creme	ent Blov	ws *	Ν
		0.0	0.1			TOPSOIL (<1")							
2 8.0	3.0 5.0	0.1	4.0	11	ML	ORANGE BROWN SILT WITH SOME FINE SAN	D.		1	2	33	45	35
2 8.0		4.0				GRAY FINE TO COARSE SAND AND GRAVEL	WITH A LITTLE SIL	Т					
2 8.0					SM/	(WEATHERED LIMESTONE?)							
	3.0 8.4			3	GM	GRAY FINE TO COARSE SAND AND FINE GRA	VEL (WEATHERE	D 5	50/5"				
			10.0			LIMESTONE?).							
						AUGER REFUSAL AT 10'. OFF-SET BORING 2							
						CONTINUOULSY AUGERED TO REFUSAL A		AIN					
						AND CONTINUOUSLY AUGERED TO REFUS	ALAI 8.5 FEET.						
						DRY AND CAVED AT 8'.							
									T				

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project	t Name:		SUNOC	O PENN	SYLVA	NIA PI	PELINE PROJECT		Project	No.: 1	03IP34	406		
Project	t Locatio	n:	LOWER	PINEY (CREE	ROAD), WILLIAMSBURG, PA	-1	Page 1	of 1				
HDD N			S2-0142	2			Dates(s) Drilled: 01-11-15	Inspector:	E. WAT	Т				
Boring			SB-02				Drilling Method: SPT - ASTM D1586	Driller:	S. HOF	FER				
Drilling	Contrac	tor:	HAD DR				Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	17.0					
Sample No.	Sample I From	Depth (ft) To	Strata D From	Depth (ft) To	Recov. (in)	Strata (USCS)	Description of Materi	als		6" Ir	creme	ent Blov	ws *	Ν
			0.0	0.0			NO DISCERNABLE TOPSOIL							
1	3.0	5.0	0.0		5		ORANGE BROWN CLAY AND SILT, TRACE FI	NE SAND.		1	3	6	6	9
				9.0		CL/								
2	8.0	9.4	9.0	9.4	12	ML	GRAY CLAY/SILT AND FINE SAND, TRACE LIN	IESTONE GRAVEL	,	2	9	50/5"		>50
							AUGER REFUSAL AT 9'.							
RUN 1	9.0	12.0	9.0	10.4	34		GRAY HIGHLY FRACTURED LIMESTONE WITH	H CALCITE DEPOS	TS.	TCR: 94	1%. SC	R: 22%,	RQD: 1	7%
	0.0	12.0	10.4	10.9	04		GRAY MODERATELY FRACTURED LIMESTINE				.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
			10.9	12.0			GRAY HIGHLY FRACTURED LIMESTONE WITH							
RUN 2	12.0	17.0	12.0	12.0	55	-	GRAY MODERATELY FRACTURED LIMESTON			TCR: 9	2%. SC	R: 56%,	RQD: 5	4%
				13.4		ROCK	DEPOSTS, FRATURES AT 12.65', 13.14', 13.2				,			<u> </u>
			13.4	13.7		R N	GRAY HIGHLY FRACTURED LIMESTONE.							
			13.7				GRAY MODERATELY FRACTURED LIMESTON	E WITH CALCITE						
				16.5			DEPOSTS, FRACTURES AT 14.96', 15.62', 16	.43'.						
			16.5	17.0			GRAY HIGHLY FRACTURED LIMESTONE.							
							CORE TESTING RESULTS (DEPTH 14'):							
							COMPRESSIVE STRENGTH: 16,180 PSI							
							· · · ·							
							UNIT WEIGHT: 174.5 PCF							<u> </u>
														+
														├──
														<u> </u>
														<u> </u>
														—
														_

S1: 2.5 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

-			fax: 302.45	4.5988									
Projec	ct Name:		SUNOC	O PENN	SYLVA	NIA PI	PELINE PROJECT	Project N	lo.: 10)3IP34	106		
Projec	ct Locatio	n:	PA 866	(HIGH S	TREET), WILL	IAMSBURG, PA	Page 1 c	of 1				
HDD I	No.:		S2-0142	2			Dates(s) Drilled: 01-13-15 Inspector:	E. WATT	-				
Boring	g No.:		SB-03				Drilling Method: SPT - ASTM D1586 Driller:	S. HOFF	ER				
Drilling	g Contrac	ctor:	HAD DR	RILLING			Groundwater Depth (ft): NOT ENCOUNTERED Total Depth (ft):	11.8					
Sample No.	Sample From	Depth (ft) To	Strata D From	Depth (ft) To	Recov. (in)	Strata (USCS)	Description of Materials		6" In	creme	ent Blov	ws *	Ν
			0.0	0.1			TOPSOIL (1")						
1	3.0	5.0	0.1	-	13		LIGHT GRAY FINE SAND WITH SOME SILT, WITH A LITTLE FINE		4	12	18	12	30
	0.0	0.0	0.1	26	10	SM	GRAVEL.		-	12	10	12	50
				3.6		CL/							
			3.6	4.8		ML	ORANGE BROWN CLAY AND SILT, TRACE FINE SAND.						
2	8.0	8.4	4.8		3	0.4/	GRAY FINE TO COARSE SAND AND GRAVEL WITH A SOME SILT	5	50/5"				
						SM/ GM	(WEATHERED LIMESTONE?)						
3	11.0	11.1		11.8	0		NO RETURN.	5	50/1"				
							AUGER REFUSAL AT 11'. OFF-SET BORING AND CONTINUOUSL	Y					
							AUGERED TO REFUSAL AT 11.8'.						
							STARTED GRINDING BETWEEN 6 AND 7'.						
							STARTED GRINDING BETWEEN 0 AND 7.						
							DRY AND CAVED AT 9.5'.						
					1								
Not	l tes/Comm <u>Pocket F</u>	Pentrome	eter Testir	ng	1	I	DR: DECOMPOSED ROCK						
	S1 (AT 4	4'): 2.5 T	SF	-									
							S1 SAMPLE TAKEN FROM CLAY/SILT PORTION.						
Strata	(USCS)	Designa	tions are	approxim	nated b	ased o	n visual review, except where indicated in Description of Materia	ıls.					
		-											
	ber of blow nber of blo						d to drive 2 in. split-spoon sampler in 6 in. increments.						

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0142

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of S	ample (ft.)	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
	SB-01	1	3.0	5.0	3.1	19.8	-	-	-	-
	30-01	2	8.0	8.4	6.2	30.5	-	-	-	-
S2-0142	SB-02	1	3.0	5.0	25.4	95.5	-	-	-	-
32-0142	30-02	2	8.0	9.4	27.7	63.2	32	23	9	CL/ML
	SB-03	1	3.0	5.0	30.5	96.6	34	25	11	CL/ML
	30-03	2	8.0	8.4	3.2	29.6	-	-	-	-

		Rock Core Te	esting Results	
Boring	Core	Approximate	Compressive	Unit
No.	Run	Depth (ft)	Strength (psi)	Weight (pcf)
SB-02	2	14	16,180	174.5

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0140

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S2-0142	Piney Creek Road		Coburn Formation - consists of medium-gray to very dark gray, fossiliferous limestone	Upland to mid- ridge	Coburn	Highly fossiliferous limestones and black shaly limestones to		5 27	Karst conditions may be present, numerous depressions are mapped Yields 6-25 gpm The lower and middle portions of the Coburn Formation consist of
		SB-03	and shaly limestone			increasingly argillaceous			interbedded crystalline, highly fossiliferous (conodonts) limestones and black shaly limestones, and the upper Coburn becomes increasingly argillaceous as it grades into the overlying Antes Shale.

Note: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0142

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
S2-142	SB-2	1	9	12	94	22	17	9	12	Moderate	Limestone	Massive	Light	Slightly fractured, Avg. Dip 37° (20° - 45°); Calcite infilling of small fractures
S2-142	SB-2	2	12	17	92	56	54	12	17	Moderate	Limestone	Massive	Grav	DID 28 (15 - 60): Calcite

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	<u>N (blows)*</u>	Particle S	ize Identifica	tion
Very Loose	5 or less		8 in. diamet	
Loose	6 to 10	Boulders	0 a.a	
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	ameter
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
Very Dense	51.01 11016	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clav	Less Than a	. , , .
And	36 - 50			, , , , , , , , , , , , , , , , , , ,
Little Some	11 - 20 21 - 35	Silt/Clay		No. 40 to No. 200 sieve (0.425 – 0.074mm) No. 200 sieve (<0.074mm)

COHESIVE SOILS

(Silt, Clay & Combinations)

<u>Consistency</u>	<u>N (blows)*</u>	Plasticity	
Very Soft	3 or less	Degree of Plasticity	Plasticity Index
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	6 , 6	

ROCK

(Rock Cores)

Rock	Rock
Quality Designation	Quality <u>Descripti</u>
<u>(RQD), %</u>	<u>on</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions			Group Symbols	Typical Descriptions	Laboratory Classifications		
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	d gravel from grain size curve. totion smaller than No. 200 sieve), classified as follows: GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	$(D_{30})^2_{D_{10} \times D_{60}}$ between 1 and 3
			GP	Poorly graded gravels, gravel- sand mixtures, little or no fines		Not meeting C_u or C_c requirements for GW	
		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size grain size ithan No. 2 illows: /, SP , SC ases requiri	Atterberg limits below A Line or I $_{\rm P}$ less than 4	Limits plotting in hatched zone with ! _p between 4 and 7 are borderline cases requiring use of dua! symbols
			GC	Clayey gravels, gravel-sand-clay mixtures	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM GC, SM, SC 5 to 12 percent Borderline cases requiring dual s)	Atterberg limits above A line with I _p greater than 7	
	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	sw	Well graded sands, gravely sands, little or no fines		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{10}}$	$(D_{30})2$ $D_{10} \times D_{60}$ between 1 and 3
			SP	Poorly graded sands, gravelly sands, little or no fines		Not meeting C_u or C_c require	ments for SW
		Sands with fines (Appreciable amount of fines)	SM	Silty sands, sand- silt mixtures	Determ bepending	Atterberg limits below A Line or I _p less than 4	Limits Plotting in hatched zone with I _p between 4 and 7 are borderline cases requiring use of dual symbols
			SC	Clayey sands, sand-clay mixtures		Atterberg limits above A line with I _p greater than 7	
Major Divisions		Group Symbols	Typical Descriptions		For soils plotting nea When w _L is near 50	rly on A line use dual symbols i.e ., l _p use CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays Organic silts and organic silty clays of low plasticity Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts		60 A Line: PI = 0.73(LL - 20) 50 U Line: PI = 0.9(LL - 8)		
		CL					
		OL			40 (Id) ×		N ^o O ^N
	Silts and Clays (Liquid limit greater than 50)	МН			Plasticity Index (PI), %	NUR A	MH or OH
		СН	Inorganic clays of high plasticity, fat clays				
		ОН	Organic clays of medium to high plasticity, organic silts			CL-ML ML or OL	
	Highly organic soils	Pt	Peat and oth soils	er highly organic		0 20 30 40 50 6 Liquid Limit (LL	0 70 80 90 100),%

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.