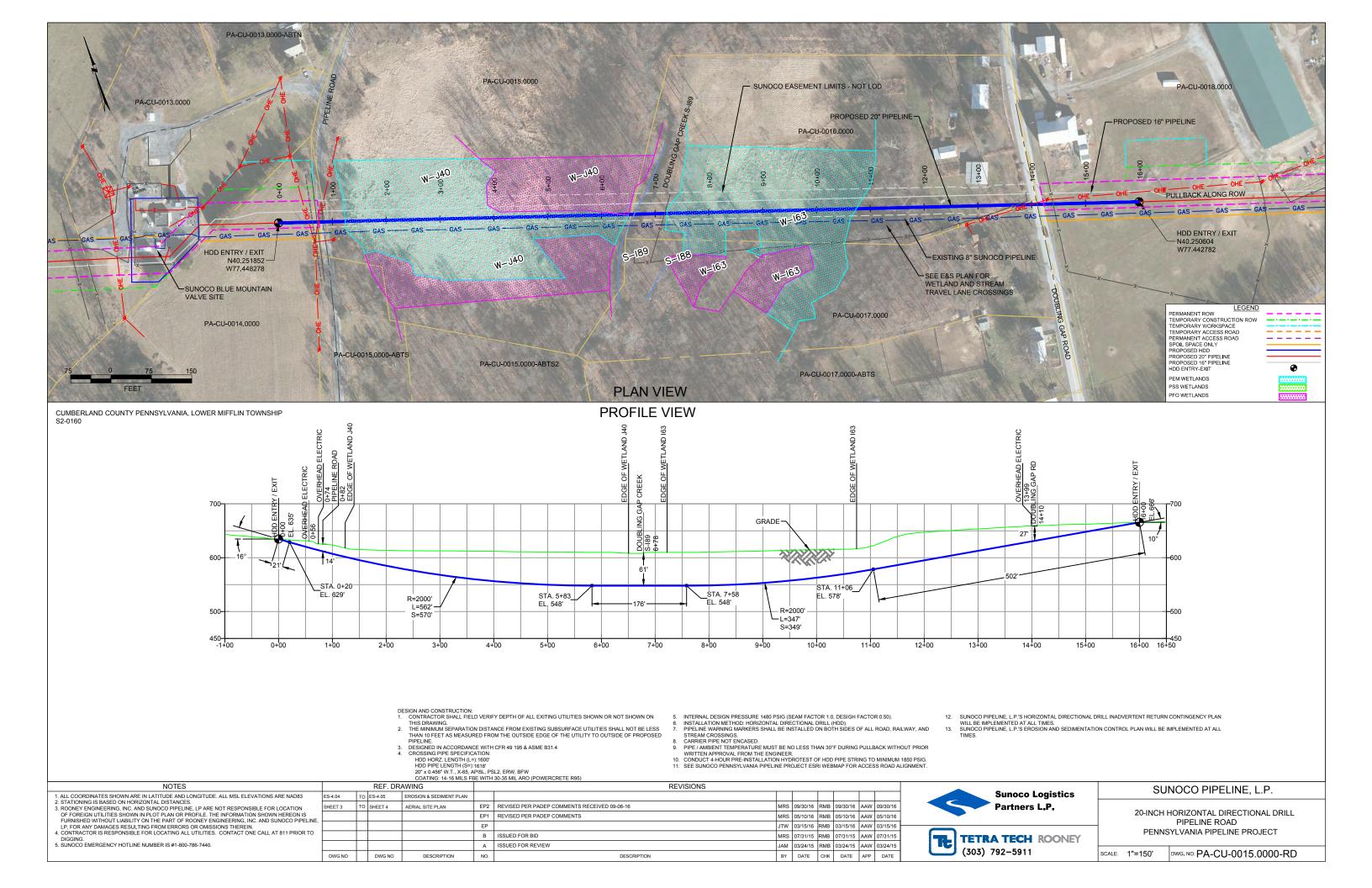
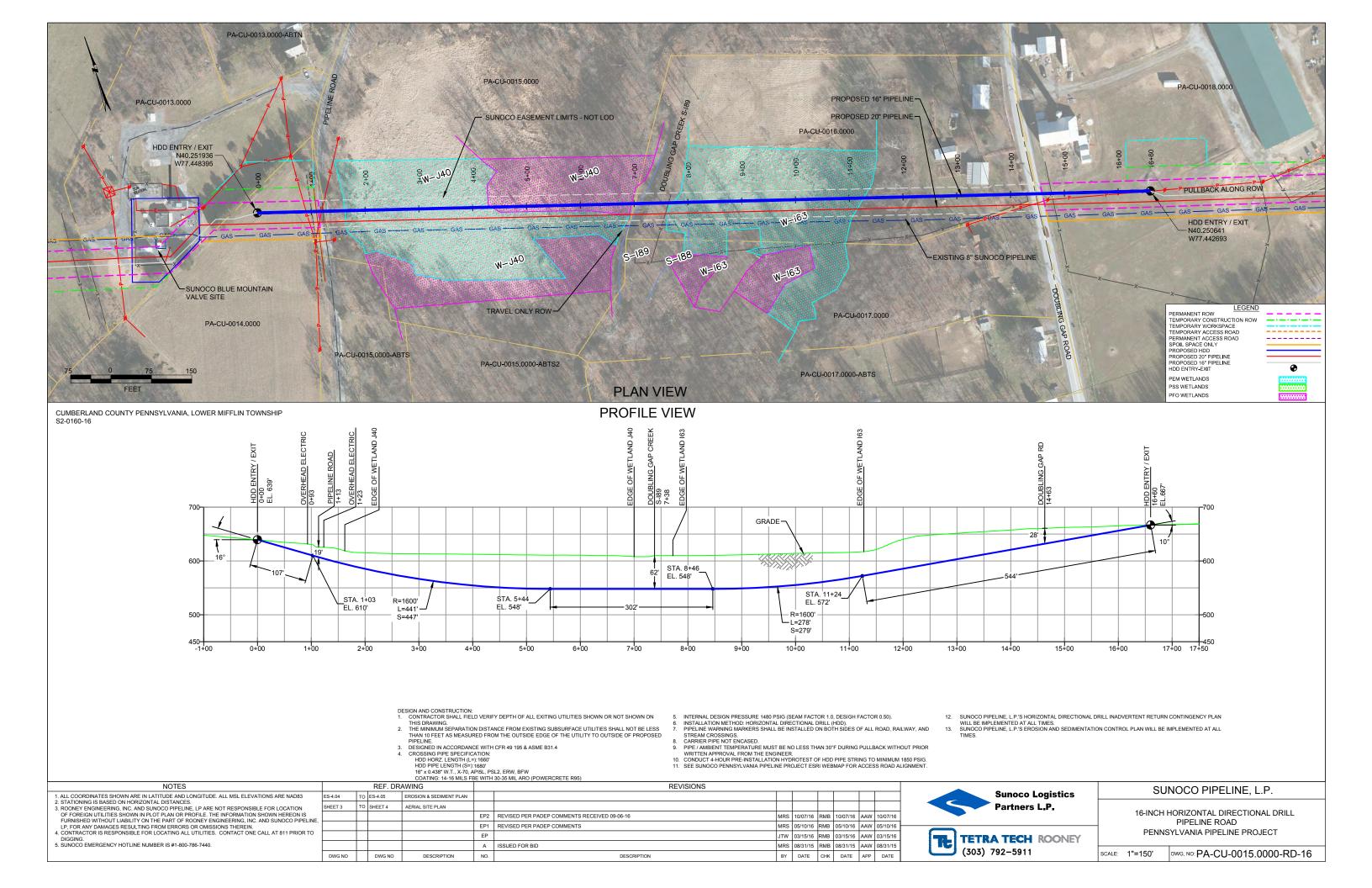
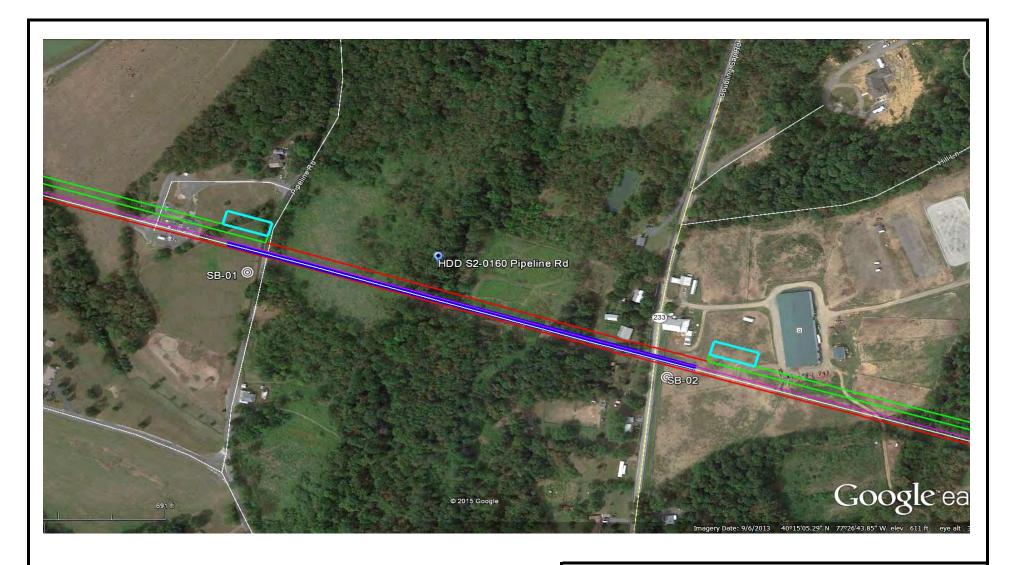
HDD PA-CU-0015.0000-RD (PEM-J40, S-I89, PEM-I63)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 160 feet from the western edge of Grassy Wetland J40 (PEM-J40) and enter/exit 970 feet from the eastern edge. The drill will enter/exit 700 feet from the western edge of Doubling Gap Creek (S-I89) and enter/exit 950 feet from the eastern edge. The drill will also enter/exit 760 feet from the western edge of Grassy Wetland (I63) and enter/exit 550 feet from the eastern edge. The drill will pass below PEM-J40 starting at 25 feet (western edge) peaking at a depth of 60 feet. It will continue 60 feet below Doubling Gap Creek and rise up to 45 feet along the eastern edge of PEM-I63. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report primary substrates being drilled through are silty sands (west end of drill) and clay with silt (east end of drill) and fractured shale rock below 30 feet. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.







LEGEND:

© Geotechnical Soil Boring (SB) Locations



TETRATECH

GEOTECHNICAL BORING LOCATIONS
HDD S2-0160
CUMBERLAND COUNTY, LOWER MIFFLIN TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENN	SYLVA	ANIA PI	PELINE PROJECT		Project	t No.: 103IP3406	
Project Location:	PIPELINE ROAD	, NEV	/VILLE,	Page 1	of 1			
HDD No.:	S2-0160			Dates(s) Drilled: 10-21-14	Inspector:	E. WA	TT	
Boring No.:	SB-01			Drilling Method: SPT - ASTM D1586	Driller:	S. HOF	FER	
Drilling Contractor:	HAD DRILLING			Groundwater Depth (ft): 15.5/7.0	Total Depth (ft):	32.8		
Boring Location Coordinates:		40°15'5.78"N	77°26'53.49"W					
Sample Depth (ft) Strata Depth (ft)		Strata					

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6"	ncreme	ent Blov	ws *	N
No.	From	То	From	То	Re (i	(USCS)	·		1010111	J. 11 D. 10 1		
			0.0	0.3			TOPSOIL (3 ")					
1	3.0	4.7	0.3		9		BROWN AND ORANGE BROWN FINE TO COARSE SAND AND FINE	3	50	50/3"		>50
							TO COARSE GRAVEL, WITH SOME SILT.					
2	8.0	9.2			10		DR FISSILE SHALE WEATHERED TO A GRAY F-C SAND	2	33	50/3"		>50
						SM/	AND F-C GRAVEL, WITH A LITTLE SILT (WHEN DISTURBED).					
3	13.0	13.8			7	GM	DR SHALE WEATHERED TO A GRAY F-C SAND	3	50/4"			>50
							AND F-C GRAVEL, WITH A LITTLE SILT (WHEN DISTURBED).					
4	18.0	18.8			10		DR SHALE WEATHERED TO A GRAY F-C SAND	10	50/4"			>50
				20.0			AND F-C GRAVEL, WITH A LITTLE SILT (WHEN DISTURBED).					
5	23.0	23.2	20.0		2		PARTIALLY WEATHERED GRAY TO DARK GRAY SHALE.	50/2"				>50
6	25.0	25.3		25.3	4		PARTIALLY WEATHERED GRAY TO DARK GRAY SHALE.	50/4"				>50
							AUGER REFUSAL AT 25'.					
							ROCK CORING					
RUN 1	25.3	28.3	25.3		32		HIGHLY FRACTURED GRAY TO DARK GRAY SHALE WITH	TCR: 8	9%, SC	R: 19%,	RQD: 0	%
						ROCK	CALCITE DEPOSITS.					
RUN 2	28.3	32.8		32.8	39	ď	SAME.	TCR: 7	2%, SC	R: 17%,	RQD: 1	1%
							CORE TESTING RESULTS (DEPTH 32.2'):					
							COMPRESSIVE STRENGTH: 530 PSI					
							UNIT WEIGHT: 165.0 PCF					
							WET ON SPOON AT 13.2'.					
							WATER LEVEL THROUGH AUGERS AT 15.5'.					
							WATER LEVEL PRIOR TO CORING AT 7'.					
							CAVED AT 21'.					
-												

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Nam	ne:	SUNOCO PENN	SYLV	ANIA PI	PELINE PROJECT		Project	No.: 103IP3406		
Project Loca	ation:	BOUBLING GAP	ROAL) (RT 2		Page 1 of 1				
HDD No.:		S2-0160			Dates(s) Drilled: 10-21-14	Inspector:	E. WA	ТТ		
Boring No.:		SB-02			Drilling Method: SPT - ASTM D1586	Driller:	S. HO	FER		
Drilling Cont	ractor:	HAD DRILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	28.8			
Boring Location Coordinates:				40°15'2.04"N	77°26'35.77"W					
Cample Sami	nle Denth (ft)	Strata Depth (ft)	>	Strata						

Builing	LUCALIUI	i Coordii	iales.				40 13 2.04 N					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials		ncreme	ent Blov	vs *	N
No.	From	То	From	То	Re	(USCS)	·		1			- '
			0.0	0.4			TOPSOIL (5 ")					
1	3.0	5.0	0.4		12		DR WEATHERED TO A VARI-COLORED CLAY/SILT AND FINE SAND,	3	8	10	17	18
							TRAVE FINE GRAVEL.					
2	8.0	10.0			14	CL/	DR WEATHERED TO A VARI-COLORED CLAY AND SILT WITH A LITTLE	3	6	7	8	13
						ML	FINE SAND, TRACE FINE GRAVEL. (USCS: CL/ML)					
3	13.0	15.0			20		DR WEATHERED TO A VARI-COLORED CLAY AND SILT WITH A LITTLE	1	9	17	17	26
				17.0			FINE SAND, TRACE FINE GRAVEL.					
4	18.0	20.0	17.0		18		DR WEATHERED TO A FINE TO MEDIUM SAND AND CLAY/SILT,	2	9	5	20	14
						SC/	TRACE F-GRAVEL.					
5	23.0	24.4			14	SM	DR WEATHERED TO A FINE TO MEDIUM SAND AND CLAY/SILT,	1	14	50/5"		>50
				23.5			TRACE F-GRAVEL. (USCS: SC/SM)					
6	28.0	28.8	23.5		6	SM/	DR WEATHERED TO A LIGHT BROWN TO GRAY F-C AND F-C GRAVEL	8	50/3"			>50
				28.8		GM	(UNWEATHERED SHALE) WITH SOME SILT.					
							CAVED AND DRY AT 28'.					
									1			
									1			
									1			<u> </u>
									1			
								<u> </u>				-

Notes/Comments:

Pocket Pentrometer Testing

S1: >4 TSF

S2: >4 TSF

S3: > 4 TSF

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0160

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of Sample (ft.)		Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	4.7	7.7	29.9	-	-	-	-
		2	8.0	9.2	7.5	20.1	-	-	-	-
	SB-01	4	18.0	18.8	9.8	16.6	-	-	-	-
		5	23.0	23.2	6.5	14.1	-	-	-	-
02.0460		6	25.0	25.3	6.5	9.8	-	-	-	-
S2-0160		1	3.0	5.0	18.5	65.1	-	-	-	-
		2	8.0	10.0	24.2	80.6	42	26	16	CL/ML
	SB-02	4	18.0	20.0	16.4	47.9	-	-	-	-
		5	23.0	24.4	12.3	45.3	31	23	8	SC/SM
		6	28.0	28.8	7.7	32.2	-	-	-	-

	Rock Core Testing Results									
Boring	Core	Approximate	Compressive	Unit						
No.	Run	Depth (ft)	Strength (psi)	Weight (pcf)						
SB-01	2	32.2	530	165.0						

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0160

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROXIMAX	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
52.0160	Dissoling Dd		Martinsburg Fm - buff-weathering, dark- gray to purple shale and slate with thin	Valley	Moutingly, up Too	Shale and slate with		20.50	
S2-0160	Pipeline Rd		interbeds of siltstone, metabentonite, and fine-grained sandstone.	Valley	Martinsburg Fm	interbedded siltstone		20-50	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0160

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
S2-0160	SB-1	1	25	28	89	19	0	25	22.5	Madausta	Delectore	Massiva		Heavily fractured, Avg.
S2-0160	SB-1	2	28	32.5	72	17	11	25	32.5	Moderate	Dolostone	Massive		Dip 65° (0° - 90°), calcite filling of minor fractures

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	N (blows)*	Particle S	ize Identifica	tion
Very Loose	5 or less	Boulders	8 in. diame	
Loose	6 to 10			
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
very bense	51 01 more	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proporti	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20		(. /	(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	Site, ciay	2000 111011 0	110. 200 5.616 (10.07 11111)

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	Degree of Plasticity	Plasticity Index
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock					
Quality Designation	Quality <u>Descripti</u>					
(RQD), %	<u>on</u>					
0-25	Very Poor					
25-50	Poor					
50-75	Fair					
75-90	Good					
90-100	Excellent					

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	ng dual syr	Not meeting C _u or C _c requiren	nents for GW
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size or than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	d gravel from grain size curve. stiton smaller than No. 200 sieve), classified as follows: GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols	
Coarse Grained Soils f material is larger tha	maller than	ands to fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (frac ed soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c required	ments for SW
N)	half of coa	n fines able fines)	SM	Silty sands, sand- silt mixtures	Determ		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils p When w _L	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	A Line:		
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50	U Line:	0.73(LL - 20) 0.9(LL - 8)	Or I
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %			, or or
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	мн		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		13/18/	MH or OH
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	blasi		Culton	
(More than	Silts ar g	ОН	Organic clays	s of medium to high anic silts	7 4	<u> </u>	ML or OL 20 30 40 50 6	0 70 80 90 100
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL	

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.