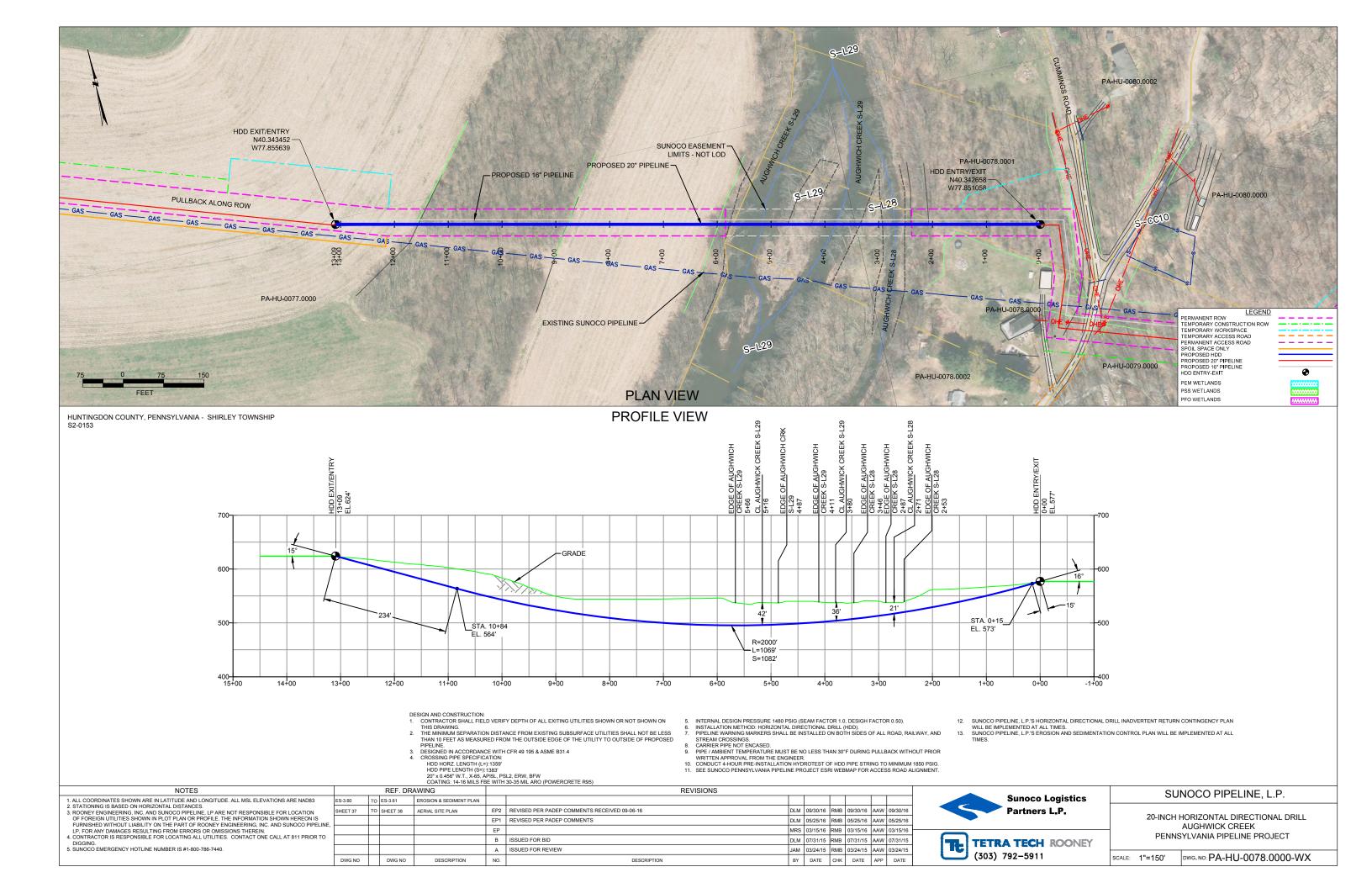
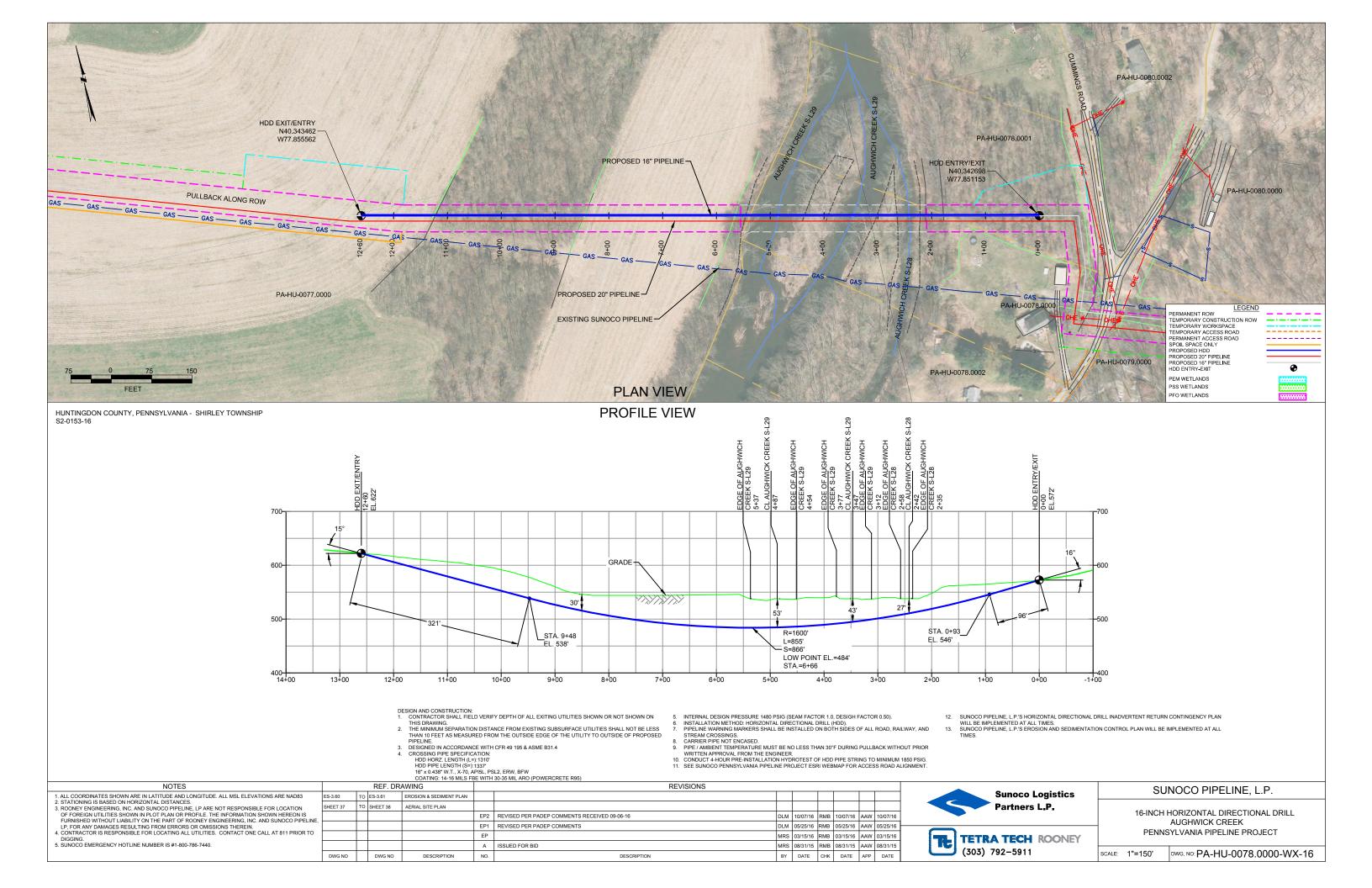
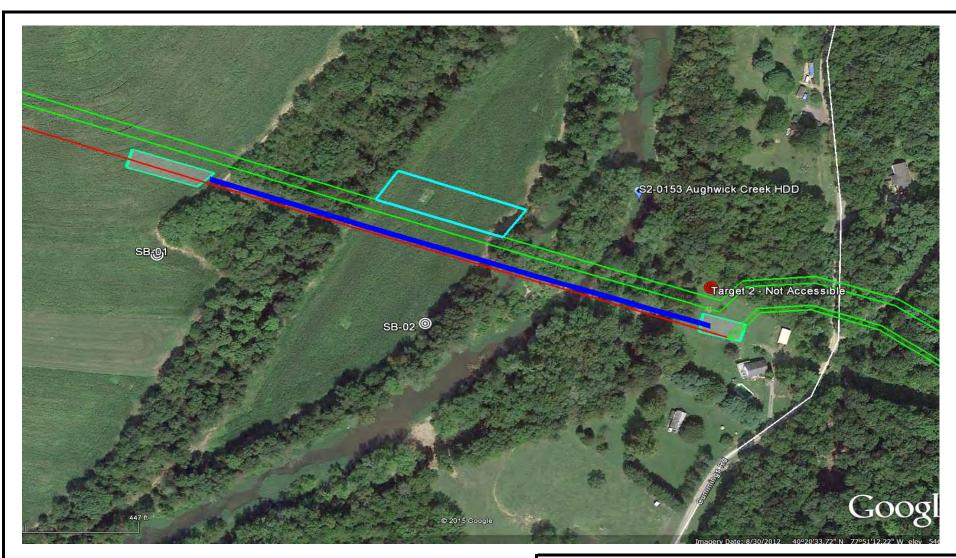
HDD PA-HU-0078.0000-WX (S-L29) (W-W46B) (S-L28)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be *low*. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 750 feet from the edge of the western most boundary of the stream S-L29. The drill will travel beneath stream S-L29 for 60 feet. The drill will then travel under wetland W-W46B for 90 feet and then travel under stream S-L29 for another 60 feet. The drill will pass beneath the western most boundary of stream S-L28 and travel under stream S-L28 for 50 feet. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be grey weathered limestone. The drill will continue beneath stream S-L28 and will enter/exit 250 feet from the eastern most edge of stream S-L28.







LEGEND:

© Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S2-0153
HUNTINGDON COUNTY, SHIRLEY TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Projec	t Name:		SUNOCO PENNSYLVANIA PIPELINE PROJECT						Project No.: 103IP3406						
rojec	t Locatio	า:	US 522,	SHIRLE	YSBU	RG, PA			Page 1	of 1					
HDD N			S2-0153	}			Dates(s) Drilled: 01-22-15	-1	E. WAT						
Boring			SB-01				Drilling Method: SPT - ASTM D1586		S. HOF	FER					
	Contrac		HAD DR	RILLING			Groundwater Depth (ft): NOT ENCOUNTERED	1 ()	6.7						
Boring	Location				.	Τ.	40°20'34.37"N	77°51'20.29"W						ı	
Sample No.	Sample I From	Depth (ft) To	Strata D	Depth (ft)	Recov. (in)	Strata (USCS)	Description of Materia	als		6" Ir	creme	ent Blov	<i>N</i> S *	N	
			0.0	0.4			TOPSOIL (5").								
1	3.0	5.0	0.4		24		MOTTLED BROWN, ORANGE BROWN, RED BF	ROWN AND YELLOV	V BRWN	5	12	10	15	22	
						CL	SILTY CLAY WITH SOME F-SAND, A LITTLE F	-GRAVEL. (USCS: 0	CL).						
2	6.5	6.7			1.5	CL	ORANGE BROWN SILTY CLAY WITH SOME FIN	NE SAND, AND		50/2"				>50	
				6.7			SOME SANDSTONE GRAVEL.								
							AUGER REFUSAL AT 6.5'. OFF-SET 23' EAST	AND CONTINUOUS	LY						
							AUGERED TO REFUSAL AT 6.5'. OFF-SET C	LOSER TO ORIGINA	٩L						
							LOCATION AND CONTINUOUSLY AUGERED	TO 6.2'.							
							CAVED AT 5'.								
							REFUSAL MATERIAL MAY BE DUE TO BOULD!	ERY SUBSURFACE,	,						
							COULD NOT PENETRATE DEEPER.								
							SANDSTONE OUTCROPS OBSERVED AT SUR	FACE IN VICINITY ()F						
							BORING.								

Notes/Comments:

Pocket Pentrometer Testing

S1: >4 TSF

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.



5

6

RUN 1

RUN 2

23.0

27.0

27.7

31.9

23.9

27.3

31.9

36.9

21.0

27.70

29.50

30.10

30.70

31.90

33.88

TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551

TEST BORING LOG

13

50/4"

50/5'

TCR: 83%, SCR: 48%, RQD: 44%

TCR: 55%, SCR: 27%, RQD: 27%

>50

>50

fax: 302.454.5988														
Projec	t Name:		SUNOC	O PENN	SYLVA	NIA PI	PELINE PROJECT		Project N	No.: 103	IP34	-06		
Projec	t Locatio	n:	US 522,	WEST S	SIDE O	F AUG	HWIZK CREEK, MOUNT UNION, PA	Page 1 of 1						
HDD No.: S2-0153							Dates(s) Drilled: 01-23-15	Dates(s) Drilled: 01-23-15 Inspector: E. WATT						
Boring No.: SB-02							Drilling Method: SPT - ASTM D1586	g Method: SPT - ASTM D1586 Driller: S. HOFFER						
Drilling Contractor: HAD DRILLING							Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	36.9					
Boring	Boring Location Coordinates: 40°20'32.84"N 77°51'13.34"W													
Sample Sample Depth (ft)		ole Depth (ft) Strata Depth (ft)			Strata	Description of Materials			C!! Incr		nt Dia	*	N	
No.	From	То	From	То	Recov. (in)	(USCS)	Description of Materials			Description of Materials 6" Incre				
			0.0	0.3			TOPSOIL (4")							
1	3.0	5.0	0.3		20		ORANGE BROWN SILTY CLAY WITH A L	ITTLE FINE SAN	D.	3	6	7	8	13
						CL								
2	8.0	10.0			20	CL	MOTTLED (BROWN AND DARK BROWN) SILTY CLAY WITH SOME				10	12	22	22
				9.9			FINE SAND. (USCS: CL).							
3	13.0	15.0	9.9		24		YELLOWISH BROWN FINE TO MEDIUM SAND	AND SILTY CLAY, \	WITH	5	11	12	18	23
						SC/	SOME UNWEATHERED SHALE GRAVEL.							
4	18.0	18.9			12	CL	YELLOWISH BROWN FINE TO MEDIUM SAND AND SILTY CLAY, WITH				0/5"			>50
				21.0		1	SOME UNWEATHERED SHALE GRAVEL. (US							

GRAY AND GREENISH GRAY WEATHERED SHALE.

29.14, 29.5. OXIDATION LENSES THROUGHOUT.
GRAY INTENSELY FRACTURED LIMESTONE.

GRAY MODERATELY FRACTURED LIMESTONE.

GRAY MODERATELY FRACTURED LIMESTONE.

CORE TESTING RESULTS (RUN 1, DEPTH 30.2):

COMPRESSIVE STRENGTH: 2,110 PSI

CORE TESTING RESULTS (RUN 2, DEPTH 32.5):

COMPRESSIVE STRENGTH: 840 PSI

GRAY VERY INSTENSELY FRACTURED LIMESTONE.

FRACTURE BREAK AT 33.88 APPEARS TO BE VERTICAL.

GRAY MODERATELY FRACTURED LIMESTINE. FRACTURES AT 28.24.

GRAY VERY INTENSELY FRACTURED AND WEATHERED LIMESTONE.

GRAY WEATHERED LIMESTONE

AUGER REFUSAL AT 27.7'.

ROCK CORING

Notes/Comments:

Pocket Pentrometer Testing

S1: > 4 TSF S2: > 4 TSF CAVED AT 27".

UNIT WEIGHT: 161.6 PCF

UNIT WEIGHT: 160.2 PCF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

WEATHERED ROCK

FRACTURED LIMESTONE ROCK

13

4

42

54

27.7

29.50

30.10

31.90

33.88

36.90

N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0153

	Test				Water	Percent	Atterburg	USCS		
HDD	Boring	Sample	Depth of S	Depth of Sample (ft.) Co		Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From			(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
	SB-01	1	3.0	5.0	16.8	71.1	32	20	12	CL
	36-01	2	6.5	6.7	16.0	72.4	-	-	-	-
	SB-02	1	3.0	5.0	19.9	86.8	-	-	-	-
SB-0153		2	8.0	10.0	20.4	75.1	35	21	14	CL
		3	13.0	15.0	16.8	42.5	-	-	-	-
		4	18.0	18.9	12.5	48.9	31	20	11	SC/CL
		5	23.0	23.9	13.5	42.6	-	-	-	-

	Rock Core Testing Results										
Boring	Core	Approximate	Compressive	Unit							
No.	Run	Depth (ft)	Strength (psi)	Weight (pcf)							
SB-02	1	30.2	2,110	161.6							
SB-02	2	32.5	840	160.2							

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0153

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	
C2 01E2	Aughwick		Onondaga and Old Port Formation - (undivided) consists of two members -	Rolling hills/ridges of	Onondaga	Limestone and calcareous	200 (Onondaga)	3-67 (average	Yields range from 20-
S2-0153 Creek		the upper Selinsgrove Limestone and the lower calcerous Needmore Shale.	med relief (<100 ft)	Old Port	occasional chert	100-200 (Old Port)	DTB range 5-30 ftbg)	75 gpm	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0153

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
		1	27.7	31.9	83	48	44	27.7	31.9	Slight to moderate	Limestone	Massive	to	Moderately fractured, ranging from 0° to 75°, Avg. 48°
S2-0153	SB-2	2	31.9	36.9	55	27	27	31.9	33.9	Slight	Limestone	Massive		Slightly fractured, ranging from 30° to 60°, Avg. 45°
		2	31.9	30.9	33	27	2/	33.9	36.9	Heavily	Limestone	Massive	_	Heavily fractured and weathered, rubble

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	N (blows)*	Particle S	ize Identifica	tion
Very Loose	5 or less	Boulders	8 in. diame	
Loose	6 to 10			
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
very bense	31 01 111010	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proporti	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20		(. ,	(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	Site, ciay	Less man d	110. 200 3.616 (10.07 411111)

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	<u>N (blows)*</u>	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock					
Quality Designation	Quality <u>Descripti</u>					
(RQD), %	<u>on</u>					
0-25	Very Poor					
25-50	Poor					
50-75	Fair					
75-90	Good					
90-100	Excellent					

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons		
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3		
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	ng dual syr	Not meeting C _u or C _c requiren	nents for GW		
o. 200 sieve		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size (than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are		
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	d gravel from grain size curve. sction smaller than No. 200 sieve), classified as follows: GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols			
Coarse Grained Soils f material is larger tha	maller than	ands io fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (fraced soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{(D_{30})2}{D_{10} \times D_{60}}$ between 1 and 3			
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c require	ments for SW		
N)	S half of coar No. 4	n half of cos No. No. siable if fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched		
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols		
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils p When w _{l.}	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.		
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	O A Line:				
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	5(U Line:	1 1	Or I		
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or		
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	МН		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH		
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	Plasi		Character			
(More than		ОН	Organic clays	s of medium to high anic silts	7		ML or OL	0 70 80 90 100		
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL			

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.