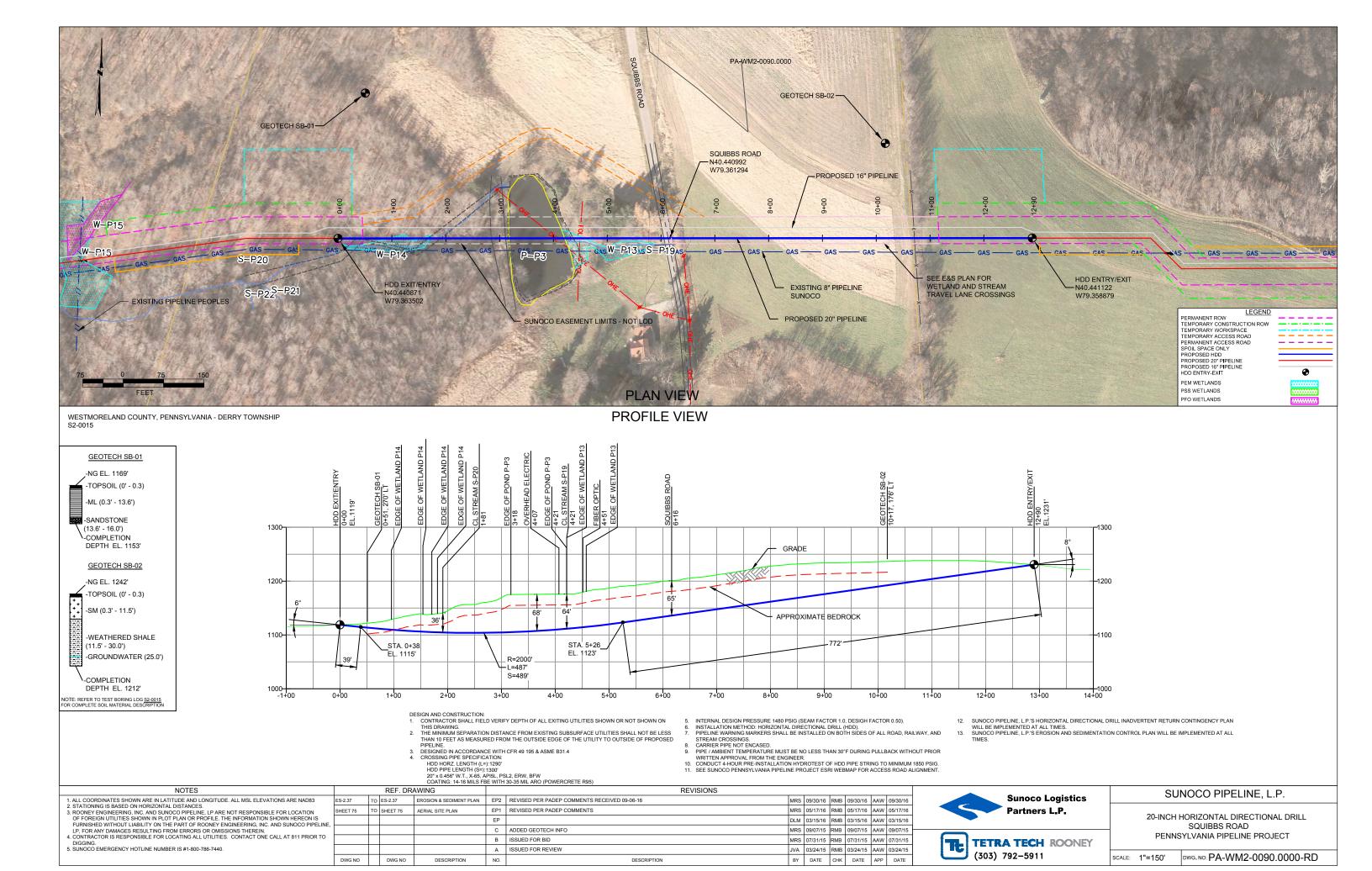
#### HDD PA-WM2-0090.0000-RD (S-P20, P-P3 and S-P19)

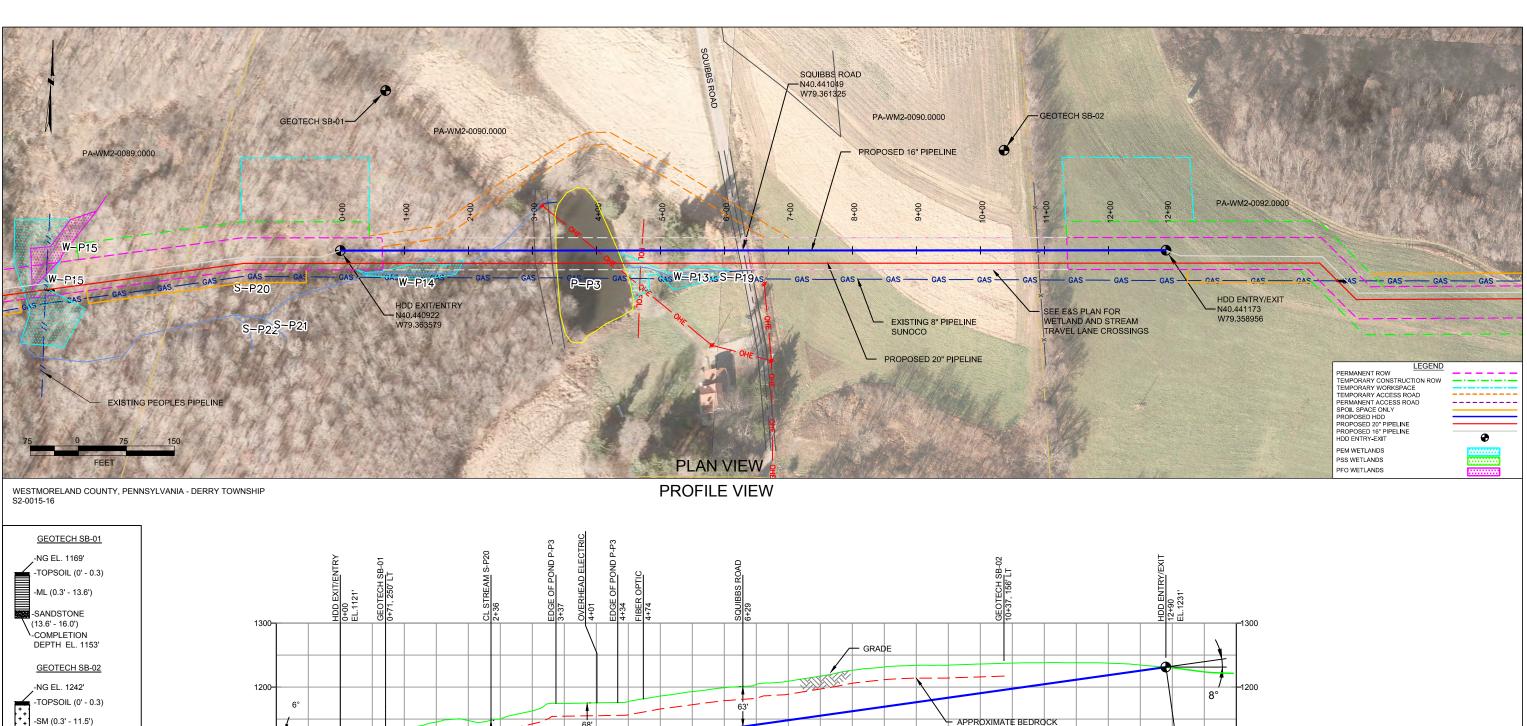
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

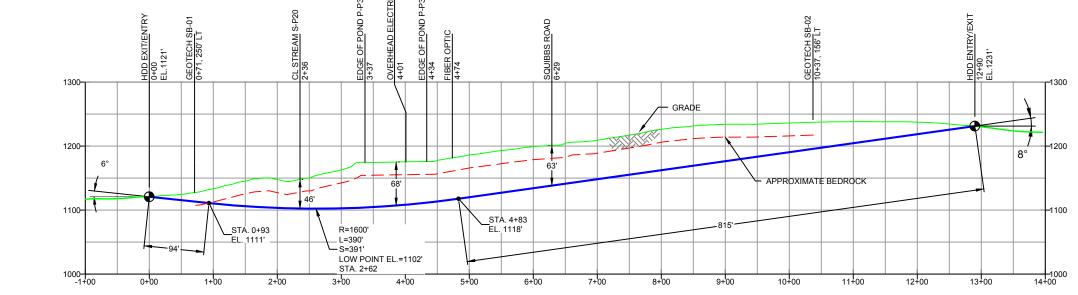
The drill will enter/exit 181 feet west of stream P20. The drill will pass 36 feet under this stream. The east entry/exit point is 1108 feet east of this stream. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silt, sand, and sandstone.

The drill will enter/exit 318 feet west of the western most edge of pond P3. The drill will pass 68 feet under this pond. The east entry/exit point is 869 feet east of the eastern most edge of this pond. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silt, sand, sandstone, and shale.

The drill will enter/exit 421 feet west of stream P19. The drill will pass 64 feet under this stream. The east entry/exit point is 869 feet east of this stream. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silt, sand, sandstone, and shale.







-WEATHERED SHALE (11.5' - 30.0')

-GROUNDWATER (25.0')

\-COMPLETION

DEPTH EL. 1212'

NOTE: REFER TO TEST BORING LOG <u>\$2-0015</u> OR COMPLETE SOIL MATERIAL DESCRIPTION

- DESIGN AND CONSTRUCTION:

  1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.

  2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED DIDELING.
- PIPELINE.
  DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- CROSSING PIPE SPECIFICATION: HDD HORZ. LENGTH (L=):1290' HDD PIPE LENGTH (S=):1300'
  - 16" x 0.438" W.T., X-70, API5L, PSL2, ERW, BFW

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50).
  INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM GROSSINGS.
  CARRIER PIPE NOT ENCASED.
  DIREC AMBIENT TEMBERATURE MIST BE NOTESS THAN 30°E DUBING BUILD ROCK WITHOUT DRIPED.
- PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR
- WRITTEN APPROVAL FROM THE ENGINEER

  10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

  11. SEE SUNDOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
   SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

				COATING: 14-16 MILS FE	3E WITH	1 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)						
NOTES	REF. DRAWING			AWING	REVISIONS							
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-2.37	то	ES-2.37	EROSION & SEDIMENT PLAN								
STATIONING IS BASED ON HORIZONTAL DISTANCES.     ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 75	то	SHEET 75	AERIAL SITE PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	DLM	10/07/16	RMB 1	10/07/16	AAW	10/07/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING. INC. AND SUNOCO PIPELINE					EP1	REVISED PER PADEP COMMENTS	MRS	05/17/16	RMB (	J5/17/16	AAW	05/17/16
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					EP		DLM	03/15/16	RMB (	J3/15/16	AAW	03/15/16
CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.					В	ADDED GEOTECH INFO	MRS	09/07/15	RMB (	J9/07/15	AAW	09/07/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR BID	MRS	08/31/15	RMB (	J8/31/15	AAW	08/31/15
	DWG NO	Т	DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE

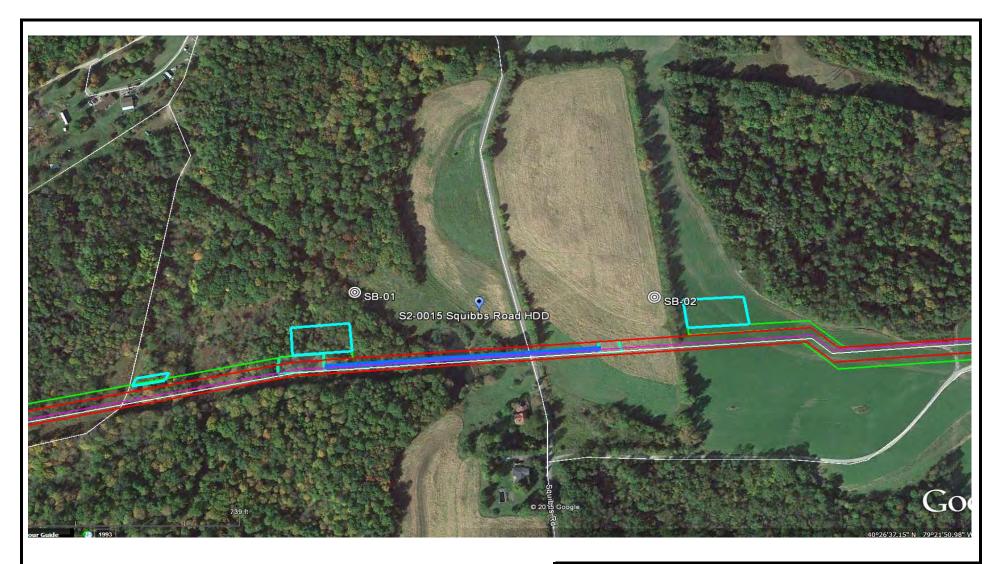


SUNOCO PIPELINE, L.P.

16-INCH HORIZONTAL DIRECTIONAL DRILL SQUIBBS ROAD PENNSYLVANIA PIPELINE PROJECT

TETRA TECH ROONEY (303) 792-5911

SCALE: 1"=150' DWG. NO: PA-WM2-0090.0000-RD-16



### **LEGEND**:

© Geotechnical Soil Boring (SB) Locations



# **TETRATECH**

**GEOTECHNICAL BORING LOCATIONS** HDD S2-0015 WESTMORELAND COUNTY, DERRY TOWNSHIP, PA SUNOCO PENNSYLVANIA PIPELINE PROJECT



#### TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

## **TEST BORING LOG**

Projec	t Name:		SUNOC	O PENNS	SYLVA	NIA PI	PELINE PROJECT	Proje	ct No.: 1	03IP34	106		
Projec	t Location	n:	SQUIBS	S ROAD	, BLAII	RSVILL	E, PA	Page	1 of 1				
HDD N	<b>1</b> 0.:		S2-0015	,			Dates(s) Drilled: 01-08-15 Inspector:	E. WA	ATT				
Boring	No.:		SB-01				Drilling Method: SPT - ASTM D1586 Driller:	S. HC	FFER				
Drilling	g Contrac	tor:	HAD DR	RILLING			Groundwater Depth (ft): NOT ENCOUNTERED Total Depth	(ft): 16.0					
Sample No.	Sample D	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata (USCS)	Description of Materials		6" I	ncreme	ent Blo	<i>N</i> S *	N
	110		0.0	0.3	<u> </u>	(0000)	TOPSOIL (3")				,		
1	3.0	5.0	0.3		10		LIGHT BROWN SILT AND FINE TO MEDIUM SAND (HIGHLY V	VEATHERE	8	20	10	12	30
							SANDSTONE).						
2	8.0	8.8			8	ML	LIGHT BROWN SILT AND FINE TO MEDIUM SAND WITH SOM	IE FINE TO	8	50/4"			>50
				13.6			COARSE SANDSTONE GRAVEL. (USCS: ML).					ļ	
3	13.0	13.8	13.6	16.0	10		PARTIALLY WEATHERED SANDSTONE (SAND AND GRAVEL	.).	20	50/4"			>50
												I	
							AUGER REFUSAL AT 16'. OFFSET BORING 15' AND CONTIL	NUOUSLY					
							AUGERED TO REFUSAL AT 15.8'.						
							AUGER GRINDING STARTED AT 13'.						
							DRY AND CAVED AT 14'.						
		<u> </u>	<u> </u>										
					<u> </u>								

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

<sup>\*</sup> Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



#### TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

## **TEST BORING LOG**

Project	t Name:		SUNOCO PENNSYLVANIA PIPELINE PROJECT Project No.: 103IP3406											
Project	t Locatio	n:	SQUIBS	S ROAD	, BLAI	RSVILL		Page 1 of 1						
HDD N			S2-0015	i			7.7	. WATT						
Boring			SB-02					. HOFFER						
Drilling	Contrac		HAD DR			1	Groundwater Depth (ft): 25.0 Total Depth (ft): 3	0.0						
Sample No.	Sample I From	Depth (ft) To	Strata D	Pepth (ft)	Recov.	Strata Description of Materials 6" II					NS *	N		
			0.0	0.3			TOPSOIL (4")							
1	3.0	5.0	0.3		10		MOTTLED (SHADES OF BROWN) MICACEOUS FINE SAND AND SILT	Г, 8	11	11	7	22		
							WITH A LITTLE UNWEATERED GRAVEL. (USCS: SM).							
2	8.0	10.0			24	SM	MOTTLED BROWN MICACEOUS FINE SAND AND SILT, WITH SOME	7	28	25	40	53		
				11.5			FINE TO COARSE GRAVEL.							
3	13.0	14.3	11.5		11		LIGHT GRAY AND BROWN WEATHERED FISSILE SHALE,	8	46	50/4"		>50		
						SHALE	SOME OXIDATION.							
4	18.0	19.3			8	SH	LIGHT GRAY AND BROWN WEATHERED FISSILE SHALE,	9	50	50/4"		>50		
						J 5	SOME OXIDATION.							
5	23.0	23.3			6	WEATHERED	LIGHT GRAY AND MAROON WEATHERED SHALE.		50/3"			>50		
						EAT								
6	28.0	28.8		30.0	8	>	LIGHT GRAY AND MAROON WEATHERED SHALE.	20	50/3"			>50		
							AUGURED TO 30'.				<u> </u>			
							CAVED AT 28.5'.				1			
							WATER LEVEL ON CAVE AT 25'.							
											<u> </u>			
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Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

<sup>\*</sup> Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

# GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0015

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of Sample (ft.)		Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, % Limit, %		Index, %	(ASTM D2487)
		1	3.0	5.0	5.5	52.1	-	-	-	-
	SB-01	2	8.0	8.8	5.7	63.6	31	24	7	ML
		3	13.0	13.8	13.7	58.8	-	-	-	-
S2-0015		1	3.0	5.0	12.6	45.3	31	25	6	SM
32-0013		2	8.0	10.0	8.7	33.2	-	-	-	-
	SB-02	3	13.0	14.3	4.6	44.3	-	-	-	-
		4	18.0	19.3	7.0	27.0	-	-	-	-
		5	23.0	23.3	4.0	65.7	-	-	-	-

### Notes:

1) Sample depths based on feet below grade at time of exploration.

## REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0015

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS	
S2-015	SB-		Casselman Formation - Cyclic sequences of shale, siltstone, sandstone, red beds, thin, impure limestone, and thin,	Rolling hills,	Casselman	Shale-sandstone with limestone-	236-525		Yields range from 5-10 gpm (note: only 3 wells	
S2-015 Squibbs Road		SB-02	nonpersistent coal; red beds are associated with landslides; base is at top of Ames limestone.	moderate relief	Casseman	clastic-coal	230 323		within 1-mile radius)	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

### FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

#### **GRANULAR SOILS**

(Sand, Gravel & Combinations)

<u>Density</u>	N (blows)*	Particle S	ize Identifica	tion
Very Loose	ery Loose 5 or less		8 in. diame	
Loose	6 to 10	Boulders		
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
very bense	31 01 111010	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proporti	ons		Medium	No. 10 to No. 40 sieve
<b>Description Term</b>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20		(. ,	(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	Site, ciay	Less man d	110. 200 3.616 (10.07 411111)

#### **COHESIVE SOILS**

(Silt, Clay & Combinations)

<b>Consistency</b>	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	Plasticity Index
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

### ROCK (Rock Cores)

Rock	Rock					
Quality Designation	Quality Descripti					
(RQD), %	<u>on</u>					
0-25	Very Poor					
25-50	Poor					
50-75	Fair					
75-90	Good					
90-100	Excellent					

\*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

### UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons			
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols <sup>(1)</sup>	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	(D <sub>30</sub> )2 D <sub>10</sub> x D <sub>60</sub> between 1 and 3			
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP Poorly graded gravels, gravelsand mixtures, little or no fines		curve. 00 sieve),	Gw, GP, Sw, SP GM. GC, SM, SC Borderline cases requiring dual symbols <sup>(1)</sup>	Not meeting C <sub>u</sub> or C <sub>c</sub> requiren	nents for GW			
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size ( than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are			
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from tion smaller assified as fo	W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols			
Coarse Grained Soils f material is larger tha	maller than	ands io fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (fraced soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{(D_{30})2}{D_{10} \times D_{60}}}$ between 1 and 3				
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Sands Tree fraction is smaller 4 Sieve) Clean sands	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve.  Depending on Percentage of fines (fraction smaller than No. 200 sieve),  coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting $C_u$ or $C_c$ requirements for SW			
N)		th fines table f fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched			
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols			
Major	Divisions	Group Symbols	Typical Descriptions		For soils p When w <sub>l.</sub>	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w <sub>L</sub> =60 gives CH-MH. ± 2 percent.			
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	O A Line:					
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	5(	U Line:	1 1	Or I			
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or			
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH			
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	Plasi		Character				
(More than	Silts ar 9	ОН	Organic clays	s of medium to high anic silts	7		ML or OL	0 70 80 90 100			
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL				

<sup>(1)</sup> Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.