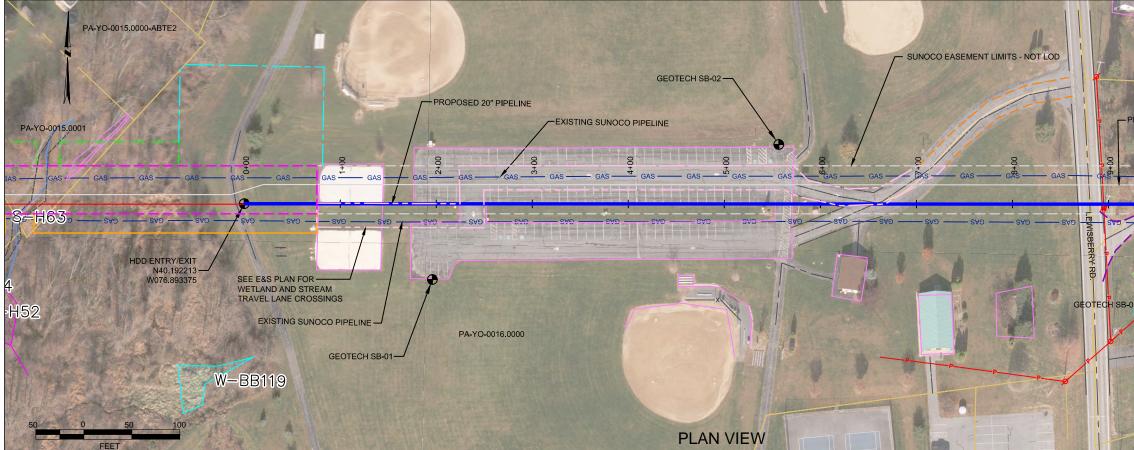
HDD PA-YO-0016.0000-RD

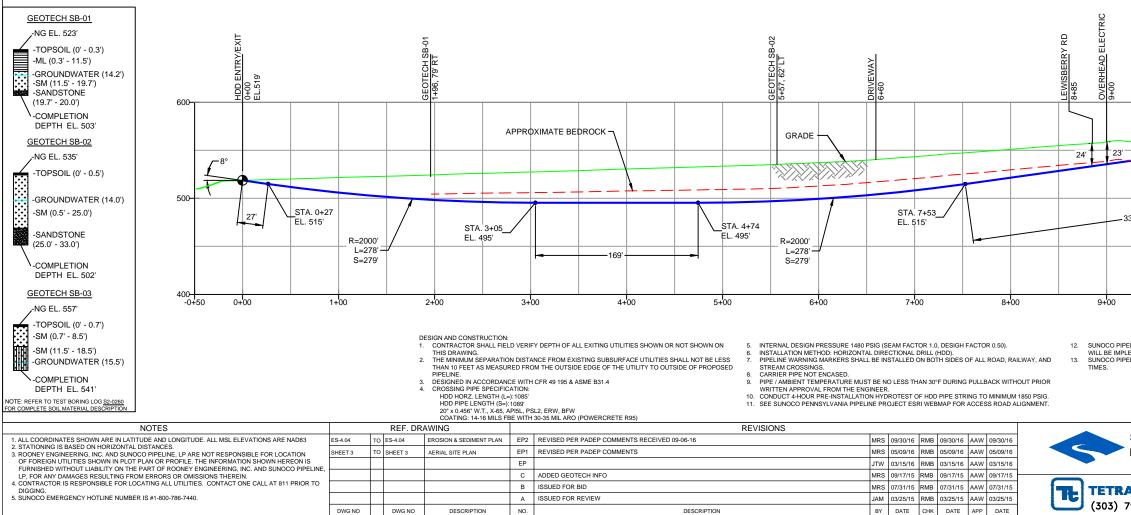
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

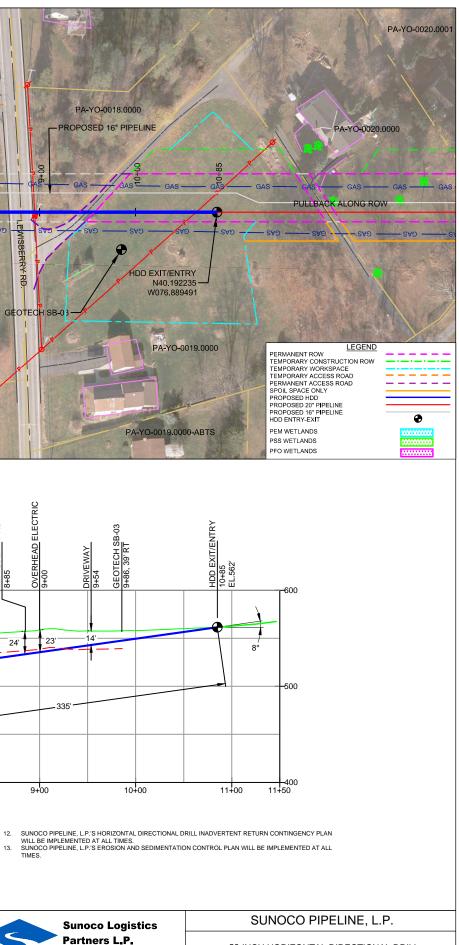
The drill will enter/exit 870 feet from the western edge of Lewisberry Road and enter/exit 210 feet from the eastern edge. The drill will pass below the road about 24 feet The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report primary substrates being drilled through are sandstone below layers of silts and fine sands. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.



PROFILE VIEW

YORK COUNTY, PENNSYLVANIA - FAIRVIEW TOWNSHIP S2-0260



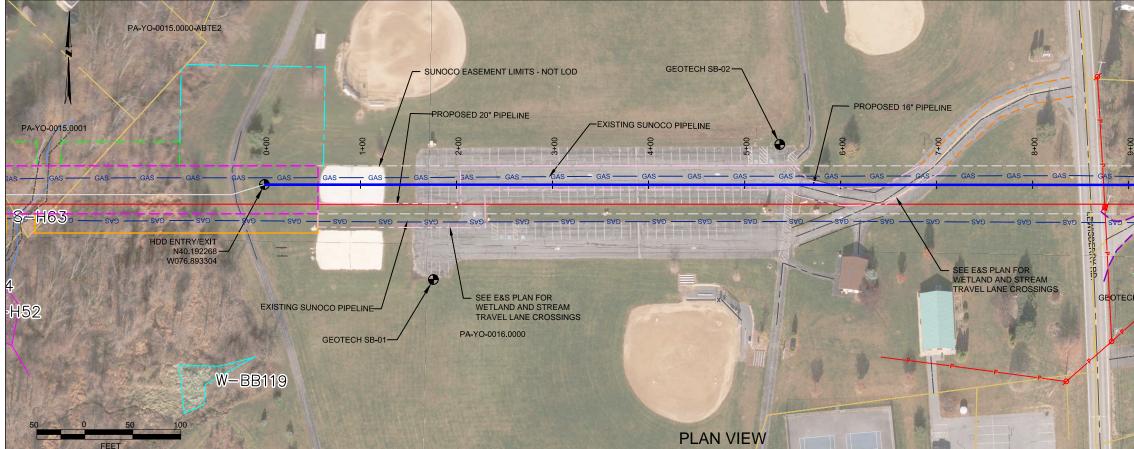


TECH	ROONEY		PEI
92-5911		SCALE:	1"=100'

20-INCH HORIZONTAL DIRECTIONAL DRILL LEWISBERRY ROAD

PENNSYLVANIA PIPELINE PROJECT

DWG. NO: PA-YO-0016.0000-RD



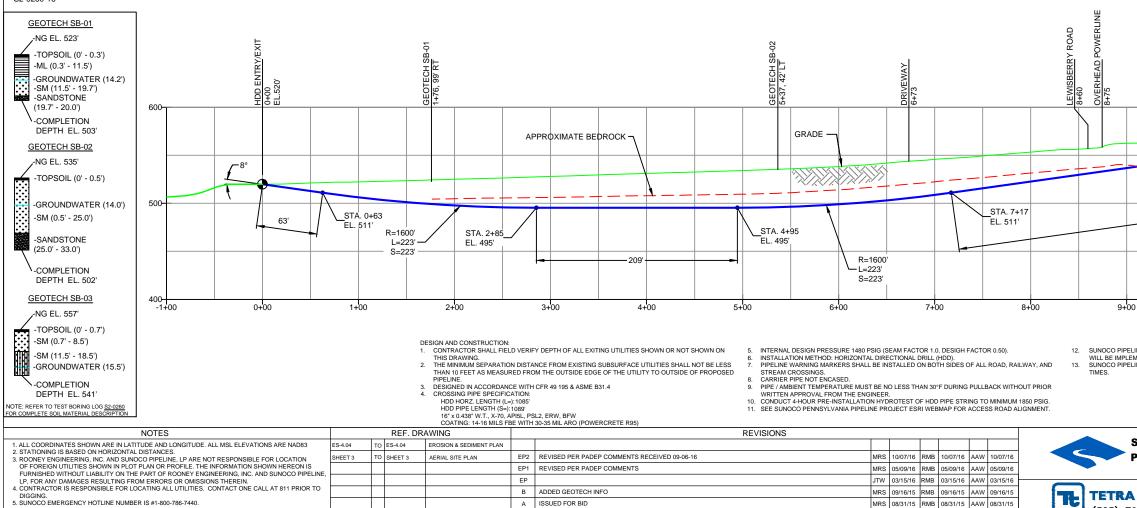
PROFILE VIEW

DESCRIPTION

MRS 08/31/15 RMB 08/31/15 AAW 08/31/15

BY DATE CHK DATE APP DATE

YORK COUNTY, PENNSYLVANIA - FAIRVIEW TOWNSHIP S2-0260-16



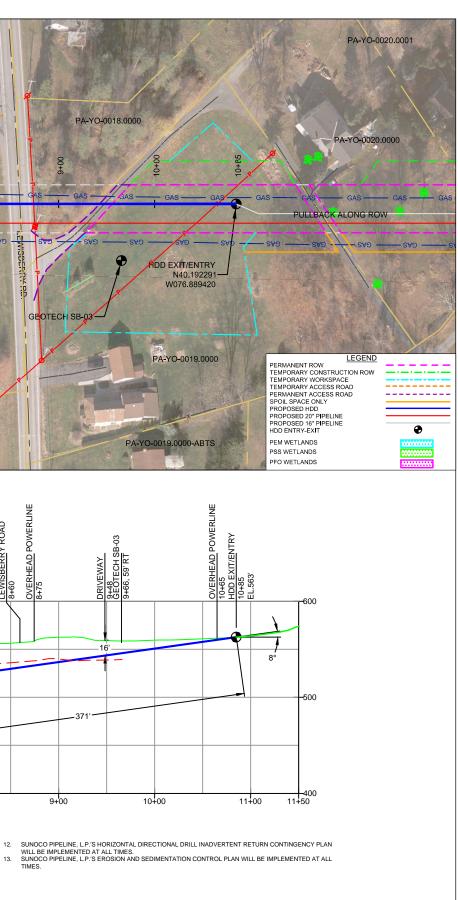
A ISSUED FOR BID

NO.

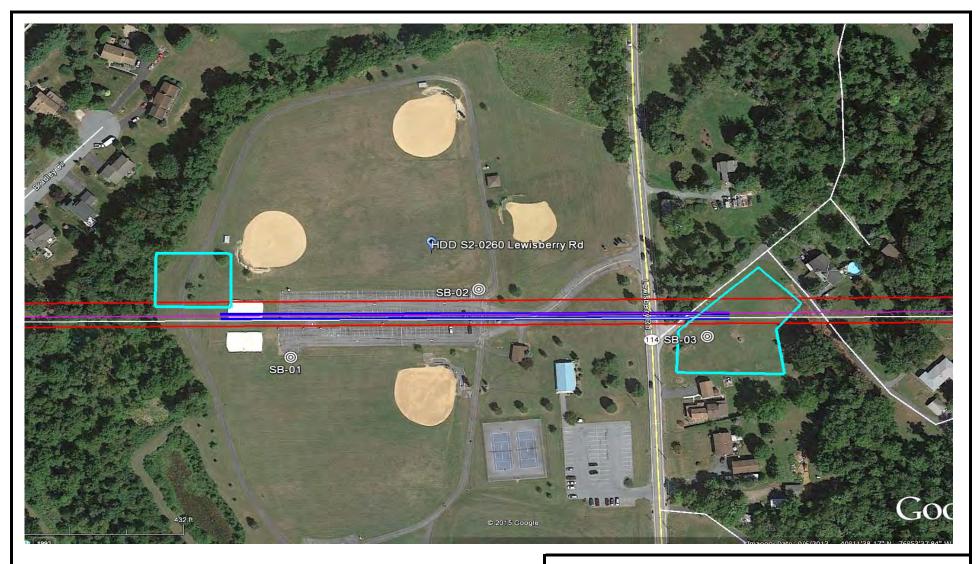
DWG NO

DWG NO

DESCRIPTION



Sunoco Logistics		SU	NOCO PIPELINE, L.P.
Partners L.P.			HORIZONTAL DIRECTIONAL DRILL LEWISBERRY ROAD
TETRA TECH ROONEY		PENNS	SYLVANIA PIPELINE PROJECT
(303) 792-5911	SCALE:	1"=100'	DWG.NO: PA-YO-0016.0000-RD-16



LEGEND:

(6) Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS HDD S2-0260 YORK COUNTY, FAIRVIEW TOWNSHIP, PA SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

			fax: 302.454	4.5988									
Projec	t Name:		SUNOC	O PENN	SYLVA	NIA PI	PELINE PROJECT		Project No.:	103IP34	106		
Projec	t Locatio	n:	ROOF P	ARK, LE	WISB	ERRY	ROAD, NEW CUMBERLAND, PA		Page 1 of 1				
HDD N	lo.:		S2-0260				Dates(s) Drilled: 10-27-14	Inspector:	E. WATT				
Boring	No.:		SB-01				Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER				
Drilling	g Contrac	tor:	HAD DR	ILLING			Groundwater Depth (ft): 14.2	Total Depth (ft):	28.0				
Sample	Sample	Depth (ft)	Strata D	epth (ft)	Recov. (in)	Strata	Description of Materia	als	6"	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Re	(USCS)			-	1		-	
			0.0	0.3			TOPSOIL (4")						
			0.3	3.5			GRAY SILT WITH A LITTLE FINE SAND.						
1	3.0	5.0	3.5				MOTTLED BROWN AND ORANGE BROWN SIL	T AND FINE SAND	2	4	6	7	10
						ML	(USCS: ML)						
2	8.0	10.0		11.5		-	MOTTLED BROWN TO GREENISH BROWN SIL	T AND FINE SAND	. 3	10	11	10	21
3	13.0	15.0	11.5				DR WEATHERED TO A VARI-COLORED FINE S	AND WITH SOME	SILT 3	6	8	16	14
							AND TRACE OF UNWEATHERED FINE SAND		- v		•		
	40.0	40.0				SM			NIT 40	50/4			50
4	18.0	18.6					DR WEATHERED TO A VARI-COLORED F-M SA		SILT 18	50/1"			>50
				19.7			AND TRACE OF UNWEATHERED FINE SAND	STONE GRAVEL.					
5	19.7	20.0	19.7	20.0			PARTIALLY WEATHERED SANDSTONE.		50/4				>50
							AUGER REFUSAL AT 19.7'.						
							ROCK CORING						
RUN 1	20.0	22.0	20.0		24	~	GRAY HIGHLY FRACTURED AND WEATHERED	SANDSTONE.	TCR:	100%, SC	CR: 0%,	RQD: 0	%
RUN 2	22.0	25.0			36	ROCK	GRAY HIGHLY FRACTURED AND WEATHERED	SANDSTONE.	TCR:	100%, SC	CR: 0%,	RQD: 0	%
RUN 2	25.0	28.0		28.0	33	Ř	GRAY HIGHLY FRACTURED AND WEATHERE	SANDSTONE.	TCR:	92%, SCI	R: 7%, F	RQD: 0%	/ 0
-													
							WATER LEVEL THROUGH AUGERS AT 14.2'.						
							CAVED AT 19.5'.						
										-			
	S1: 2 TS S2: 2.5	Pentrome SF TSF	eter Testir		<u> </u>	<u>I</u>	DR: DECOMPOSED ROCK			<u> </u>		<u> </u>	1

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project	Name:		SUNOC) PENN	SYLVA	NIA PI	PELINE PROJECT	Proje	ect No.: 1	03IP34	106		
Project	Locatio	n:	ROOF P	ARK, LE	WISB	ERRY I	ROAD, NEW CUMBERLAND, PA	Page	e 1 of 1				
HDD N	lo.:		S2-0260				Dates(s) Drilled: 10-27 and 11-04-14	Inspector: E. W	ATT				
Boring			SB-02				Drilling Method: SPT - ASTM D1586		OFFER				
Drilling	Contrac		HAD DR			1	Groundwater Depth (ft): 14.0	Total Depth (ft): 33.0					
Sample No.	Sample I From	Depth (ft) To	Strata D From	epth (ft) To	Recov. (in)	Strata (USCS)	Description of Materia	ls	6" I	ncreme	ent Blo	ws *	Ν
			0.0	0.5			TOPSOIL (6")						
1	3.0	5.0	0.5		19		GREENISH BROWN TO GRAYISH BROWN FINE	SAND WITH SOME	3	8	9	12	17
						-	SILT.						
2	8.0	10.0			16	-	YELLOWISH BROWN TO LIGHT BROWN FINE T SOME SILT, TRACE FINE GRAVEL.	O MEDIUM SAND WITH	4	20	39	50	59
3	13.0	13.9			9	_	YELLOWISH BROWN TO LIGHT BROWN FINE 1		7	50/5"			>50
3	13.0	13.9			9	SM		O MEDION SAND WITH	1	50/5			>50
4	18.0	18.9			10	-	SOME SILT, TRACE FINE GRAVEL. BROWN TO YELLOWISH BROWN MEDIUM TO (COARSE SAND WITH	3	50/5"			>50
							SOME SILT, AND A LITTLE FINE GRAVEL.						
5	20.0	20.8			5	-	LIGHT BROWN TO YELLOWISH BROWN F-M S	AND WITH A LITTLE	2	50/4"			>50
						-	SILT.						
6	23.0	23.3			3		PARTIALLY WEATHERED SANDSTONE.		50/4"				>50
U	20.0	20.0			Ŭ				00/1				
							AUGER REFUSAL AT 25'.						
							ROCK CORING						
RUN 1	25.0	28.0	25.0		12		GRAY HIGHLY FRACTURED AND DEGRADED S	SANDSTONE. WITH	TCR: 3	3%, SCF	R: 0%, F	RQD: 0%	/ %
						-	OXIDATION.	,					
RUN 2	29.0	33.0			26	-	GRAY HIGHLY FRACTURED AND DEGRADED S	SANDSTONE, WITH	TCR: 5	4%, SCF	R: 0%, F	RQD: 0%	6
				33.0			OXIDATION.						
							BORING COLLAPSED AFTER REMOVING COE	BAREL AFTER RUN 1.					
							AUGERED BACK DOWN TO 29'. EACH CORE F	UN TOOK SEVERAL					
							ATTEMPTS BECAUSE SANDSTONE FRAGMEN	TS KEPT COLLAPSING					
							INTO BOREHOLE.						
							REFUSAL MATERIAL MAY BE A RESULT OF BC	OULDERY CONDITIONS.					<u> </u>
													+
Note	es/Comm	nents:			I	I				I		I	<u> </u>

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project	Name:		SUNOC	O PENN	SYLVA	NIA PI	PELINE PROJECT		Project No	o.: 10)3IP34	06		
Project	Location	n:	ROOF F	PARK, LE	EWISB	ERRY I	ROAD, NEW CUMBERLAND, PA	T	Page 1 of	1				
HDD N	lo.:		S2-0260)			Dates(s) Drilled: 10-26-14	Inspector:	E. WATT					
Boring			SB-03				Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFE	R				
Drilling	Contrac		HAD DR			1	Groundwater Depth (ft): 15.5	Total Depth (ft):	18.5					r
Sample No.	Sample I From	Depth (ft) To	Strata D From	Depth (ft) To	Recov. (in)	Strata (USCS)	Description of Mater	als		6" In	creme	nt Blo	ws *	Ν
			0.0	0.7			TOPSOIL (7")							
1	3.0	5.0	0.7		21		MOTTLED ORANGE BROWN AND LIGHT BRO	WN FINE TO MEDIU	JM	1	6	5	10	11
				8.5		SM	SAND AND SILT (USCS: SM).							
2	8.0	10.0	8.5		24		DR WEATHERED TO A GREENISH BROWN TO	O GRAYISH BROWN	N FINE	2	12	15	20	27
							TO MEDIUM SAND WITH A LITTLE SILTY CL	AY, TRACE F-GRA	/EL.					
3	13.0	13.8			9	_	DR WEATHERED TO A GREENISH BROWN TO	O GRAYISH BROWN	NFINE 2	20	50/3"			>50
						SC/ SM	TO MEDIUM SAND WITH A LITTLE SILTY CL	TO MEDIUM SAND WITH A LITTLE SILTY CLAY, TRACE F-GRAVEL.						
4	18.0	18.2			3	511	DR WEATHERED TO A LIGHT BROWN TO YE	LOWISH BROWN	50)/3"				>50
							MEDIUM TO COARSE SAND WITH A LITTLE	SILTY CLAY.						
5	18.5	18.5		18.5	0		NO RETURN.		50)/0"				
							AUGER REFUSAL AT 18.5'.							
							REFUSAL MATERIAL MAY BE A RESULT OF E	ITHER BOULDERY	OR					
							CONGLOMERATE SUBSURFACE CONDITION	S.						
								-						
Ţ														
					1									

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0260

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of S	Sample (ft.)	Content, %	Silts/Clays, %	,	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	5.0	17.4	53.1	39	37	2	ML
		2	8.0	10.0	32.2	53.8	-	-	-	-
	SB-01	3	13.0	15.0	22.5	26.2	-	-	-	-
		4	18.0	18.6	6.6	21.4	-	-	-	-
		5	19.7	20.0	9.1	22.8	-	-	-	-
		1	3.0	5.0	9.1	27.5	-	-	-	-
S2-0260		2	8.0	10.0	7.0	24.0	-	-	-	-
32-0200	SB-02	3	13.0	13.9	8.5	26.1	-	-	-	-
		4	18.0	18.9	12.9	22.8	-	-	-	-
		6	23.0	23.3	6.1	14.0				
		1	3.0	5.0	15.2	36.8	29	22	7	SM
	SB-03	2	8.0	10.0	12.2	19.3	-	-	-	-
	30-03	3	13.0	13.8	5.3	12.6	-	-	-	-
		4	18.0	18.2	4.4	14.1	-	-	-	-

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0260

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
		30-01	Quartz Fanglomerate - consists of coarse conglomerate containing rounded cobbles and boulders of		Quartz	Conglomerate-		31-64	
			quartzite, sandstone, quartz, and some metarhyolite in a matrix of red sand.	Conthusian	fanglomerate	sandstone		31-04	
S2-0260	Lewisberry Road	SB-03	Gettysburg conglomerate is a coarse quartz conglomerate containing rounded pebbles and cobbles in a matrix of red sand. Diabase - occurs primarily as dikes and sheets and forms a complex igneous network that extensively intrudes sedimentary rocks in the Gettysburg basin.	Gently sloping to level upland (suburban)	Gettysburg Conglomerate with diabase sheets to the east	Quartz conglomerate with sand to occasional diabase dikes and sheets	7,300	15-31	

<u>Note</u> : Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S2-0260

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
S2-260	SB-1	1	20	22	100	0	0							Extremely heavily
S2-260	SB-1	2	22	25	100	0	0	20	28	Moderately to heavily	Coarse sandstone	Massive	grav	fractured, ranging from 0° to 90°; no pieces large or intact enough for
S2-260	SB-1	3	25	28	92	7	0							compression testing
S2-260	SB-2	1	25	28	33	0	0	25	28	Moderate	Sandstone	Massive	Red	Poor recovery, fractures ranging from 0° to 45°
S2-260	SB-2	2	29	33	54	0	0	29	33	Heavily	Sandstone	Massive	Gray	Heavily fractured, ranging from 0° to 90°

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	<u>N (blows)*</u>	Particle S	ize Identifica	tion
Very Loose	5 or less		8 in. diamet	
Loose	6 to 10	Boulders	0 a.a	
Medium Dense	11 to 30	Cobbles	3 to 8 in. di	ameter
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
Very Dense	51.01 11016	Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clav	Less Than a	. , , .
And	36 - 50			, , , , , , , , , , , , , , , , , , ,
Little Some	11 - 20 21 - 35	Silt/Clay		No. 40 to No. 200 sieve (0.425 – 0.074mm) No. 200 sieve (<0.074mm)

COHESIVE SOILS

(Silt, Clay & Combinations)

<u>Consistency</u>	<u>N (blows)*</u>	Plasticity	
Very Soft	3 or less	Degree of Plasticity	Plasticity Index
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	6 , 6	

ROCK

(Rock Cores)

Rock	Rock					
Quality Designation	Quality <u>Descripti</u>					
<u>(RQD), %</u>	<u>on</u>					
0-25	Very Poor					
25-50	Poor					
50-75	Fair					
75-90	Good					
90-100	Excellent					

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

	Major Divisi	ons	Group Symbols	Typical Descriptions		Laboratory Classification	ons	
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	пbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	$(D_{30})^2_{D_{10} \times D_{60}}$ between 1 and 3	
(6	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve), ng dual syr	Not meeting C_u or C_c requiren	nents for GW	
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size grain size ithan No. 2 ilows: /, SP , SC ases requiri	Atterberg limits below A Line or I $_{\rm P}$ less than 4	Limits plotting in hatched zone with I p between 4 and 7 are	
d Soils ger than Ne	More tha	Gravel v (Appre amount	GW gravels, gravel-sand mixtures, little or no fines Cu GP Poorly graded gravels, gravel-sand mixtures, little or no fines Output of the sand mixtures, little or no fines Not GP Silty gravels, gravel-sand-silt mixtures Silty gravels, gravel-sand-silt mixtures Output of the sand mixtures Attu or the sand mixtures GM Clayey gravels, gravel-sand-silt mixtures Constant of the sand mixtures Constant of the sand of	Atterberg limits above A line with I _p greater than 7	borderline cases requiring use of dual symbols			
Coarse Grained Soils of material is larger tha mater than More		sands to fines)	sw	Well graded sands, gravely sands, little or no fines	of fines (fract of fines (fract ed soils are cla percent C percent B cont C percent C	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{10}}$	$(D_{30})2$ $D_{10} \times D_{60}$ between 1 and 3	
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM GC, SM, SC 5 to 12 percent Borderline cases requiring dual s)	Not meeting C_u or C_c require	ments for SW	
(We	S half of coa No.	t fines able fines)	SM	Silty sands, sand- silt mixtures	Determ bepending	Atterberg limits below A Line or I _p less than 4	Limits Plotting in hatched	
	(More than I	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures		Atterberg limits above A line with I _p greater than 7	zone with I _P between 4 and 7 are borderline cases requiring use of dual symbols	
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils plotting nea When w _L is near 50	rly on A line use dual symbols i.e ., l _p use CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.	
	ys han 50)	ML	sands, rock f	s and very fine lour, silty or clayey r clayey silts with ly	60[1111]	e:		
200 sieve)	silts and clays d limit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50 U Lii	1	ON I	
ls r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	40 (Id) ×		N ^o O ^N	
Fine-grained soils naterial is smaller t	iquid limit 50)	МН		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %	NUR A	MH or OH	
Fir half of mat	(More than half of material is smaller than No. 200 Silts and Clays (Liquid limit greater than 50) (Liquid limi	СН	Inorganic cla fat clays	ys of high plasticity,				
More than		ОН	Organic clays plasticity, org	s of medium to high anic silts		CL-ML ML or OL		
)	Highly organic soils	Pt	Peat and oth soils	er highly organic		0 20 30 40 50 6 Liquid Limit (LL	0 70 80 90 100),%	

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.