

November 13, 2015 103IP3406

Mr. Uriah Sowell Rooney Engineering 115 Inverness Drive East, Suite 300, Englewood, CO 80112

Subject: Infiltration Testing – Montello Valve Site Sunoco Pennsylvania Pipeline Project Berks County, Spring Township

Dear Mr. Sowell:

Tetra Tech, Inc. (Tetra Tech) performed infiltration testing within proposed stormwater management feature areas at the proposed Montello Valve Site in Spring Township, Commonwealth of Pennsylvania. This letter report summarizes results of the infiltration testing.

The intended method for infiltration testing was the double-ring constant head test, and was performed at three of the five test locations (IT-03, IT-04, and IT-05). At two of the locations (IT-01, IT-02), the field water truck used to supply water for testing could not access test locations within reasonable distance because of their locations within a corn field (water truck would have destroyed crop along access route). Therefore, the method used to conduct infiltration testing at IT-01 and IT-02 was the single-ring falling head test. This commonly accepted test method utilizes a considerable amount less water, and the water was reasonably hauled to the test locations with buckets. Five infiltration tests were performed at the site on September 18 and 21, 2015, in accordance with ASTM International (ASTM) D5126 for single ring testing, and ASTM D3385 for double ring testing; locations of the tests are shown in Attachment 1.

Prior to infiltration testing, a hand auger soil boring was advanced adjacent to each test location to log lithology, inspect for evidence of seasonal high water table, and collect representative soil samples. Soil borings were advanced by use of a hand-held auger, and subsurface conditions were logged. Boring logs (Attachment 2) include soil data obtained from the explorations. Bedrock and groundwater were not encountered within 2 feet of the infiltration test depth. The underlying geology is the Millbach Formation, a pinkish gray and medium gray Cambrian laminated limestone with interbeds of dolomite.

A soil sample was collected at each of the five infiltration test depths. The samples were inspected and described visually in Tetra Tech's geotechnical laboratory. A Percent Finer than a No. 200 Sieve Test (ASTM D1140) was performed to measure the amount of silt and clay particulate in the soil samples. An Atterberg Limit Test (ASTM D4318) was conducted to aid in classification of the soils. Results of the grain-size analysis and Atterberg Limits testing were referenced to determine the Unified Soil Classification System (USCS) designation for the soils encountered at the infiltration test depth. A summary of the laboratory testing results is in Attachment 3.

As discussed above, infiltration testing via a single-ring falling head test method and double-ring constant head test method were conducted; procedures for these test methods are described in Attachment 4. Results from the infiltration testing are summarized in the attached Infiltration Testing Tables (Attachment 5). Table 1 summarizes investigation and testing depths, results of the infiltration testing, and USCS classifications and descriptions of soils at the infiltration test depths.



	SUMMAR	RT OF RESUL			STIGATION
Infiltration Test Location	Infiltration Test Depth (inches)	Off-Set Soil Boring Depth (inches)	Infiltration Testing Results (inches/hour)	USCS Class. at Test Depth	Generalized Description of Soils at Test Depth
IT-01	9.75	48	4.35 (4.08 last hour)	ML/SM	Brown silt and fine sand
IT-02	9.1	48	6.78 (5.76 last hour)	ML/SM	Yellowish brown silt and fine sand
IT-03	9	48	1.14 ML		Yellowish brown sandy silt, trace fine to coarse gravel
IT-04	9	48	0.13	ML	Brown silt with some fine sand, trace fine gravel
IT-05	6	48	0.03	ML	Brown silt with a little fine sand, trace fine gravel

TABLE 1 SUMMARY OF RESULTS FROM INFILTRATION INVESTIGATION

Tetra Tech's services accorded with generally accepted engineering practice. No warranty, expressed or implied, is given. We appreciate the opportunity to provide our professional services to you. If you have any questions regarding the testing we performed, please contact me at (302) 283-2274, or via E-mail at ralph.boedeker@tetratech.com.

Sincerely,

Ralph Boedeker

Ralph Boedeker, P.E. Geotechnical Project Manager

cc: Karen Gleason (Tetra Tech - Pittsburgh)

Attachments

- Attachment 1: Infiltration Test Locations
- Attachment 2: Soil Boring Logs
- Attachment 3: Laboratory Testing Summary
- Attachment 4: Falling Head Singe Ring Infiltration Test Procedures
- Attachment 5: Infiltration Testing Tables

Infiltration Testing – Montello Sunoco PPP November 13, 2015



Attachment 1

Infiltration Test Locations



LEGEND:

(infiltration Test Locations (IT)



TETRA TECH

INFILTRATION TEST LOCATIONS MONTELLO VALVE SITE BERKS COUNTY, SPRING TOWNSHIP, PA SUNOCO PENNSYLVANIA PIPELINE PROJECT



Attachment 2

Soil Boring Logs



INFILTRATION TESTING SOIL LOG

Project: Sunoco PPP - Montello Valve Site

Equipment Used: Infiltration Testing Equipment/Hand Auger

Project No.: 103IP3406

Boring/Pit No.: IT-01

Tested by: <u>Tetra Tech/Hynes</u>

Weather: Sunny

Geology:

Horizon	Upper Boundary		Soil Textural Class	Description	Soil Color	Color Patterns	Pores, Roots, Rock Structure		Depth to Water	Comments
	0''	9"	topsoil	topsoil	brown	Solid				
	9"	48"	ML/SM	brown silt and fine sand	brown	solid		Not Encountered	Not Encountered	%<200: 51.5



INFILTRATION TESTING SOIL LOG

Project: Sunoco PPP - Montello Valve Site

Equipment Used: Infiltration Testing Equipment/Hand Auger

Project No.: 103IP3406

Boring/Pit No.: IT-02

Tested by: <u>Tetra Tech/Hynes</u>

Weather: Sunny

Geology:

Horizon	Upper Boundary	Lower Boundary	Soil Textural Class	Description	Soil Color	Color Patterns	Pores, Roots, Rock Structure	Depth to Bedrock	Depth to Water	Comments
	0''	8"	topsoil	topsoil	brown	Solid				
	8"	48"	ML/SM	silt and fine sand	yellowish brown	solid		Not Encountered	Not Encountered	%<200: 50.2



Tested by: Tetra Tech/Hynes

INFILTRATION TESTING SOIL LOG

Project: Sunoco PPP - Montello Valve Site

Equipment Used: Infiltration Testing Equipment/Hand Auger

Project No.: 103IP3406

Boring/Pit No.: IT-03

Weather: Sunny

Geology:

Horizon	Upper Boundary	Lower Boundary	Soil Textural Class	Description	Soil Color	Color Patterns	Pores, Roots, Rock Structure	Depth to Bedrock	Depth to Water	Comments
	0''	9"	topsoil	topsoil	brown	Solid				
	9"	48"	ML	sandy silt, trace f-c gravel.	yellowish brown	solid		Not Encountered	Not Encountered	%<200: 56.3



Tested by: Tetra Tech/Hynes

INFILTRATION TESTING SOIL LOG

Project: Sunoco PPP - Montello Valve Site

Equipment Used: Infiltration Testing Equipment/Hand Auger

Project No.: 103IP3406

Boring/Pit No.: IT-04

Weather: Sunny

Geology:

Horizon	Upper Boundary	Lower Boundary	Soil Textural Class	Description	Soil Color	Color Patterns	Pores, Roots, Rock Structure	Depth to Bedrock	Depth to Water	Comments
	0"	9"		topsoil	brown	Solid				
	9"	48"	ML	silt with some fine sand, trace fine gravel	brown	solid		Not Encountered	Not Encountered	%<200: 74.3



Tested by: Tetra Tech/Hynes

INFILTRATION TESTING SOIL LOG

Project: Sunoco PPP - Montello Valve Site

Equipment Used: Infiltration Testing Equipment/Hand Auger

Project No.: 103IP3406

Boring/Pit No.: IT-05

Weather: Sunny

Geology:

Horizon	Upper Boundary	Lower Boundary	Soil Textural Class	Description	Soil Color	Color Patterns	Pores, Roots, Rock Structure	Depth to Bedrock	Depth to Water	Comments
	0''	6"	topsoil	topsoil	brown	Solid				
	6"	48"	ML	silt with a little fine sand, trace fine gravel	brown	solid		Not Encountered	Not Encountered	%<200: 82.2



Attachment 3

Laboratory Testing Summary

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT MONTELLO VALVE SITE

	Soil		Water Percent		Atterburg	Atterburg Limits (ASTM D4318)			
Valve	Boring	Sample	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.	
Site	No.	No.	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)	
	IT-01	IT-01	20.4	51.5	34	25	9	ML/SM	
	IT-02	IT-02	19.8	50.2	33	25	8	ML/SM	
Montello	IT-03	IT-03	16.7	56.3	34	26	8	ML	
	IT-04	IT-04	18.3	74.3	38	26	12	ML	
	IT-05	IT-05	2.3	82.2	39	27	12	ML	

Notes:

1) Sample depths based on feet below grade at time of exploration.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

	Major Divisi	ons	Group Symbols	Typical Descriptions		Laboratory Classification	ons
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	пbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	$(D_{30})^2_{D_{10} \times D_{60}}$ between 1 and 3
(6	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	d gravel from grain size curve. d gravel from grain size curve. classified as follows: GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C_u or C_c requiren	nents for GW
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size grain size ithan No. 2 ilows: /, SP , SC ases requiri	Atterberg limits below A Line or I $_{\rm P}$ less than 4	Limits plotting in hatched zone with I p between 4 and 7 are
d Soils ger than Ne	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from gravel from tion smaller assified as fr W, GP, SW M. GC, SM orderline c	Atterberg limits above A line with I _p greater than 7	borderline cases requiring use of dual symbols
Coarse Grained Soils if material is larger tha	maller than	sands to fines)	sw	Well graded sands, gravely sands, little or no fines	of fines (fract of fines (fract ed soils are cla percent C percent B cont C percent C	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{10}}$	$(D_{30})2$ $D_{10} \times D_{60}$ between 1 and 3
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM GC, SM, SC 5 to 12 percent Borderline cases requiring dual s)	Not meeting C_u or C_c require	ments for SW
(We	S half of coa No.	t fines able fines)	SM	Silty sands, sand- silt mixtures	Determ bepending	Atterberg limits below A Line or I _p less than 4	Limits Plotting in hatched
	(More than I	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures		Atterberg limits above A line with I _p greater than 7	zone with I _p between 4 and 7 are borderline cases requiring use of dual symbols
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils plotting nea When w _L is near 50	rly on A line use dual symbols i.e ., l _p use CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
	ys han 50)	ML	sands, rock f	s and very fine lour, silty or clayey r clayey silts with ly	60[1111]	e:	
200 sieve)	silts and clays d limit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50 U Lii	1	ON I
ls r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	40 (Id) ×		N ^o O ^N
Fine-grained soils (More than half of material is smaller than No. 200	iquid limit 50)	МН		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %	NUR A	MH or OH
Fir half of mat	e than half of material is sm Silts and Clays (Liquid limit greater than 50)	СН	Inorganic cla fat clays	ys of high plasticity,			
More than	Silts ar 9	ОН	Organic clays plasticity, org	s of medium to high anic silts		CL-ML ML or OL	
)	Highly organic soils	Pt	Peat and oth soils	er highly organic		0 20 30 40 50 6 Liquid Limit (LL	0 70 80 90 100),%

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

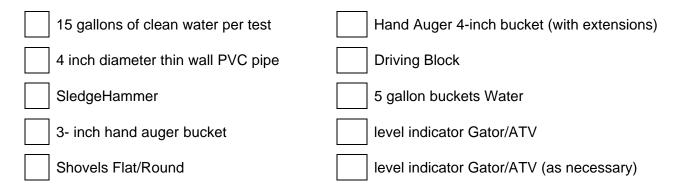


Attachment 4

Falling Head Singe Ring Infiltration Test and Double Ring Constant Head Test Procedures



Falling Head Singe Ring Infiltration Test Procedure



Procedure

- A. Unless directed otherwise, advance one soil boring at each test location. The boring should extend to groundwater. Accurately measure depth to groundwater and depth of each soil change. Pay close attention to soils for mottling. Contact office to determine test depth. Note: This step can be omitted if test borings were advanced during a previous site visit.
- B. Advance a 4-inch diameter soil boring to the specified test depth. Check boring log to ensure that soil at bottom of excavation is soil type to be tested.
- C. Cut thin wall PVC to length (approximately 1 to 2' longer than desired test depth).
- D. Push/drive PVC to bottom of soil boring.
- E. Using 3-inch auger, clean out bottom of test hole to remove any soils that caved in during PVC placement. Drive PVC casing an additional 2 inches to ensure that bottom of test hole does not extend beyond the bottom of the PVC pipe.
- F. Collect initial test information using water level indicator
 - 1. Determine the total depth to the bottom of the hole from top of pipe and record.
 - 2. Determine riser height above ground and .record.
 - 3. Subtract 2 feet from total depth (See F.1.) and record.
- G. Start Test\
 - 1. Set up water level indicator at depth determined in F.3.
 - 2. Fill tube with water until water level indicator alarms. To minimize soil scouring, slowly pour water down the inside of the casing wall.
 - 3. Record exact depth to water with level indicator
- H. Run Test:
 - 1. Pre-soak (1 hour or less).
 - a. Record depth to water every 15 minutes for first hour (pre-soak).
 - b. At the end of first hour refill pipe with water to level determined Step F.3.



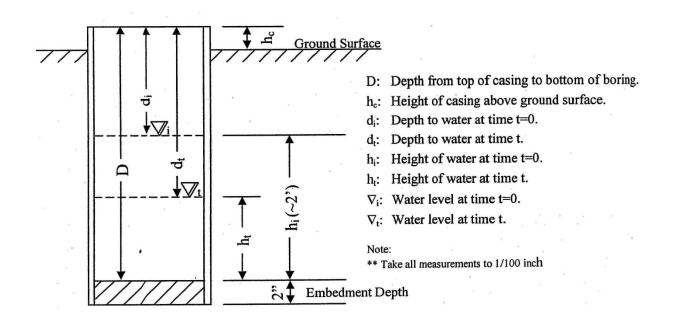
Falling Head Singe Ring Infiltration Test Procedure

- 2. Infiltration testing (four, one Hour tests)
 - a. Tests starts after completing Step H.1.b
 - b. Record depth of water every 15 minutes (or more frequently) for one hour or until water drains from pipe.
 - c. Refill pipe with water to level determined in Step F.3
 - d. Repeat Step H.2.b. and c. three additional times (four test runs)
 - e. Testing concludes after pre-soak and four test runs are completed
- I. Calculations

Infiltration rate is calculated as inches per hour.

Determine the water level drop recorded during each one hour test (note that the water level indicator is marked in tenths of a foot. A conversion to inches is required). Multiply the water level drop recorded in tenths of a foot by 1.2 to get water level drop in inches.

All data should be recorded on pre-made forms.





Falling Head Singe Ring Infiltration Test Procedure

Date: Test Location:

___ Test Location:

Depth from top of Casing to Bottom of Boring (D):

Height of Casing above Ground Surface (h_c):

Tester/ Technician Performing Test:

		Depth to Water (d _t)	
Time (t)	Time Elapsed	Measured from top of casing to water to nearest 1/100 foot	Change



Constant Head Double Ring Infiltration Test

Tools and Supplies:

- \square 125 gallons of clean water per test □ Driving Block and Cap
- □ Two infiltrometers per test □ Purge Pump and tubing
- \Box One 12" ring per test
- \Box One 24" ring per test
- □ Splash Guard
- □ Shovels Flat/Round
- □ Hand Rake
- □ Sledge Hammer

- □ Battery
- - □ Backhoe (for tests greater than 2 feet)
 - □ Gator/ATV (as necessary
 - □ Hand Auger (with extensions)
- □ Thermometer
- \Box Supply bucket (1/2 inch PVC, tubing,
 - funnel extra valves....)

Procedure:

- A. Unless directed otherwise, advance one soil boring at each test location. The boring should extend to groundwater. Accurately measure depth to groundwater and depth of each soil change. Pay close attention to soils for mottling. Contact office to determine test depth. Note this step can be omitted if test borings were advanced during a previous site visit.
- B. Excavate test pit to specified test depth. Test pit should be sloped or benched in accordance with OSHA standards. (For safety two people will be onsite for tests deeper than 4 feet).
- **C.** Use Flat point shovel to grade bottom of test pit. Bottom of excavation should be flat but not compacted. Check boring log to ensure that soil at bottom of excavation is soil type to be tested. (Collect and bag sample of soil at the bottom of the excavation) Include soil description and classification on worksheet.
- **D.** Set up infiltrometer:
 - 1. Set 24" ring at bottom of excavation.
 - 2. Using driving block drive (ring) 3 to 4 inches into the ground (Record penetration depth).
 - 3. Set 12" ring at bottom of excavation centered in 24" ring.
 - 4. Using driving block drive (ring) 2 to 3 inches into the ground (Leave 12" ring approximately 1 inch higher than 24" ring.) (Record penetration depth)
 - 5. Lightly tamp disturbed soil along inside and outside edges of rings. Do not compact soil at the bottom of the hole.
 - 6. Use hand rake to scarify soils within the test rings.
 - 7. Install drop tubes on infiltrometers.
 - 8. Measure distance from bottom of drop tube to soil. Should be 5 to 6 inches. Record on attached form. Should be $\pm \frac{1}{4}$ inch between two rings



Constant Head Double Ring Infiltration Test

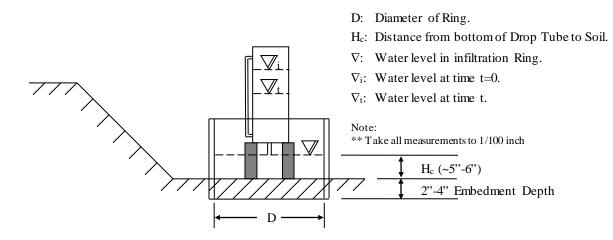
- 9. Set stand and infiltrometer on each ring.
 - a. Make sure infiltrometers are oriented so that bottom valve is easy to reach.
 - b. Measure distance from bottom of drop tube to soil. Should be 5 to 6 inches. Record as Hc on attached form.
- E. Fill infiltrometer and ring.
 - 1. Use pump, battery to transfer water to test set up as necessary.
 - 2. Be sure bottom valves are closed and all top valves are open.
 - 3. Fill infiltrometers through top valve to 0 L mark.
 - 4. Place splash guard within rings to prevent soil scouring.
 - 5. Fill rings until water reaches the bottom of the drop tube.
 - 6. Water level should be $+ \frac{1}{4}$ inch between two rings.
 - 7. Remove splash guard.
- **F.** Start Test
 - 1. Close all upper valves.
 - 2. Open bottom valves.
 - 3. Record water level (in milliliters) in sight windows on attached form. Observe water from vantage point that is approximately level with the water in the sight glass. Please note that markings on PVC casing are for reference only. Do not use these marks when observing water level.
 - 4. Record time on attached form.
- G. Monitor Test
 - 1. Record water level in sight window and time on attached form.
 - 2. Readings should be taken every 15 minutes for the 1st hour, 30 minutes for the second hour and every hour there after.
 - 3. Do not allow water to drop below sight window at any time during testing.
 - 4. Test duration is a minimum of 5 hours.
 - 5. Refill test set up as necessary (When the water level reaches a point which will not allow another reading without running out.)
 - a. Close bottom valves.
 - b. Open top valves.
 - c. Fill infiltrometers through top valve to 0 L mark.
 - d. Record time and water level before and after filling.
 - e. Close top two valves.
 - f. Open bottom valve.
 - 6. Test can be terminated when two successive permeability rates do not vary by more than 10%.



Constant Head Double Ring Infiltration Test

H. Calculations

Inner Ring: $V_{IR} = \Delta V_{IR} / (A_{IR} * \Delta T)$ Annular Space: $V_A = \Delta V_A / (A_A * \Delta T)$ V= infiltration velocity (cm/h) ΔV = volume of liquid used during time interval (cm³) ΔT = time interval in h A= internal area of ring or annular space (cm²)





Attachment 5

Infiltration Testing Tables

INFILTRATION TEST DATA SHEET

JOB NAME:

PPP - Montello Station Valve Sites

PROJECT NUMBER:

103IP3406 September 18, 2015

TEST LOCATION: IT-01 0.81 ft

TEST DATE:		September 18, 2015	TEST DEPTH:	0.81 ft			
	TIME	DEPTH TO WATER BELOW GROUND SURFACE	HYDRAULIC HEAD	Δ HYDRAULIC HEAD	PERMEA	BILITY (K _{m)}	COMMENTS
	9:00	-1.22 ft	2.03 ft				
ur 1	9:15	-1.09 ft	1.90 ft	0.13 ft			
Test Hour	9:30	-0.93 ft	1.74 ft	0.16 ft			
Test	9:45	-0.77 ft	1.58 ft	0.16 ft			
	10:00	-0.64 ft	1.45 ft	0.13 ft	0.58 ft/hr	6.96 in/hr	
N	10:15	-0.55 ft	1.36 ft	0.09 ft			
Test Hour	10:30	-0.40 ft	1.21 ft	0.15 ft			
sth	10:45	-0.30 ft	1.11 ft	0.10 ft			
ΤΨ	11:00	-0.20 ft	1.01 ft	0.10 ft	0.44 ft/hr	5.28 in/hr	
33	11:15	-0.08 ft	0.89 ft	0.12 ft			
Inop	11:30	0.00 ft	0.81 ft	0.08 ft			
Test Hour 3	11:45	0.09 ft	0.72 ft	0.09 ft			
Ĕ	12:00	0.19 ft	0.62 ft	0.10 ft	0.39 ft/hr	4.68 in/hr	
4	12:15	0.26 ft	0.55 ft	0.07 ft			
Test Hour 4	12:30	0.33 ft	0.48 ft	0.07 ft			Refilled to 1.20 feet AGS
sst F	12:45	-1.20 ft	2.01 ft	0.00 ft			
Ĕ	13:00	-1.06 ft	1.87 ft	0.14 ft	0.28 ft/hr	3.36 in/hr	
۲.5 ۲	13:15	-0.92 ft	1.73 ft	0.14 ft			
Pon	13:30	-0.87 ft	1.68 ft	0.05 ft			
Test Hour	13:45	-0.79 ft	1.60 ft	0.08 ft			
Ĕ	14:00	-0.72 ft	1.53 ft	0.07 ft	0.34 ft/hr	4.08 in/hr	

There are generally two acceptable methods to calculate steady state infiltration rates:

1. Time Weighted Average: 4.35 in/hr

2. Final Test Hour Reading: 4.08 in/hr

INFILTRATION TEST DATA SHEET

JOB NAME:

PPP - Montello Station Valve Sites

PROJECT NUMBER:

103IP3406

TEST LOCATION: IT-02 0.76 ft

TEST DATE:		September 18, 2015	TEST DEPTH:	0.76 ft			
	TIME	DEPTH TO WATER BELOW GROUND SURFACE	HYDRAULIC HEAD	Δ HYDRAULIC HEAD	PERMEA	BILITY (K _{m)}	COMMENTS
	9:00	-1.26 ft	2.02 ft				
ur 1	9:15	-0.98 ft	1.74 ft	0.28 ft			
Test Hour 1	9:30	-0.61 ft	1.37 ft	0.37 ft			
Les	9:45	-0.48 ft	1.24 ft	0.13 ft			
	10:00	-0.31 ft	1.07 ft	0.17 ft	0.95 ft/hr	11.40 in/hr	
N	10:15	-0.18 ft	0.94 ft	0.13 ft			
Test Hour	10:30	0.01 ft	0.75 ft	0.19 ft			
sst H	10:45	0.13 ft	0.63 ft	0.12 ft			
Τe	11:00	0.26 ft	0.50 ft	0.13 ft	0.57 ft/hr	6.84 in/hr	
33	11:15	0.38 ft	0.38 ft	0.12 ft			Refilled to 1.36 feet AGS
Test Hour 3	11:30	-1.36 ft	2.12 ft	0.00 ft			
sst F	11:45	-1.14 ft	1.90 ft	0.22 ft			
Ψ	12:00	-0.91 ft	1.67 ft	0.23 ft	0.57 ft/hr	6.84 in/hr	
4	12:15	-0.77 ft	1.53 ft	0.14 ft			
Test Hour 4	12:30	-0.61 ft	1.37 ft	0.16 ft			
sst H	12:45	-0.45 ft	1.21 ft	0.16 ft			
Τe	13:00	-0.27 ft	1.03 ft	0.18 ft	0.64 ft/hr	7.68 in/hr	
5	13:15	-0.09 ft	0.85 ft	0.18 ft			
Inol	13:30	0.01 ft	0.75 ft	0.10 ft			
Test Hour	13:45	0.12 ft	0.64 ft	0.11 ft			
Te	14:00	0.21 ft	0.55 ft	0.09 ft	0.48 ft/hr	5.76 in/hr	

There are generally two acceptable methods to calculate steady state infiltration rates: 1. Time Weighted Average: 6.78 in/hr

2. Final Test Hour Reading: 5.76 in/hr

					<u>Tt D</u>	ouble Ring	Infiltratio	on Test				
Location:		IT-03		Project Name	e:	PPP - Mon	tello Statior	Valve Sites	Date:	Septembe	r 18, 2015	
Inner Ring I	Diameter:	12	inch	Soil Tempera			77	degrees	Outer Ring I		3.00	inches
Inner Ring		729.7	cm^2	Water Temp			64	degrees	Inner Ring F		2.00	inches
Outer Ring		24	inch	Inner Ring Li			4.00	inches	Test Depth:		0.75	feet
Annular Spa		2189	cm^2	Annular Spa		pth:	3.00	inches	Technician:		JR/AB	
				-					-			
Trial	Start/End	Date	Time	delta-Time	Elapsed			Readings		Incrementa	I Infiltration	Notes
					Time	Inner	Ring		r Space	Inner	Outer	
						Reading	Flow	Reading	Flow	Ring	Ring	
				(min)	(min)	(ml)	(ml)	(ml)	(ml)	(in/hr)	(in/hr)	
1	S	9/18/15	10:10	15	15	0	1000	0	500	0.00	0.00	
•	E	9/18/15	10:25	10	10	1,000	1000	500	000	0.00	0.00	
2	S	9/18/15	10:25	15	30	1,000	500	500	0	1.0791	0.0000	
_	E	9/18/15	10:40			1,500		500	-			
3	S	9/18/15	10:40	15	45	1,500	1,000	500	750	2.1583	0.5396	
	E	9/18/15	10:55	-	-	2,500	,	1,250				
4	S	9/18/15	10:55	15	60	2,500	500	1,250	250	1.0791	0.1799	
	E	9/18/15	11:10			3,000		1,500				
5	S E	9/18/15 9/18/15	11:10	15	75	3,000 3,750	750	1,500 2,500	1,000	1.6187	0.7194	
	S	9/18/15	11:25 11:25			3,750		2,500	-			-
6	E	9/18/15	11:40	15	90	5,000	1,250	4,000	1,500	2.6978	1.0791	
	S	9/18/15	11:40			5,000		4,000				
7	E	9/18/15	11:55	15	105	5,750	750	4,500	500	1.6187	0.3597	
	S	9/18/15	11:55			5,750		4,500				
8	E	9/18/15	12:10	- 15	120	6,250	500	6,000	1,500	1.0791	1.0791	
	S	9/18/15	12:10			6,250		6,000				
9	E	9/18/15	12:25	15	135	7,000	750	6,750	750	1.6187	0.5396	
40	S	9/18/15	12:25	45	450	7,000	500	6,750	4.050	4.0704	0.0000	
10	E	9/18/15	12:40	15	150	7,500	500	8,000	1,250	1.0791	0.8993	
11	S	9/18/15	12:40	15	165	7,500	500	8,000	1,000	1.0791	0.7194	
1.1	E	9/18/15	12:55	15	105	8,000	500	9,000	1,000	1.0791	0.7194	
12	S	9/18/15	12:55	15	180	8,000	250	9,000	1,000	0.5396	0.7194	
14	E	9/18/15	13:10	10	100	8,250	200	10,000	1,000	0.0000	0.1104	
13	S	9/18/15	13:10	15	195	8,250	500	10,000	500	1.0791	0.3597	
	E	9/18/15	13:25			8,750		10,500				
14	S	9/18/15	13:25	15	210	8,750	250	10,500	500	0.5396	0.3597	
	E	9/18/15	13:40			9,000		11,000				
15	S E	9/18/15	13:40	15	225	9,000	500	11,000 12,000	1,000	1.0791	0.7194	
	S	9/18/15 9/18/15	13:55			9,500						
16	E	9/18/15	13:55 14:10	16	241	9,500 10,000	500	12,000 13,250	1,250	1.0117	0.8431	
	S	9/18/15	14:10			10,000		13,250				
17	E	9/18/15	14:10	15	256	10,000	0	13,250	500	0.0000	0.3597	
	S	9/18/15	14:25			10,000		13,750	1.			
18	E	9/18/15	14:40	- 15	271	10,000	250	14,750	1,000	0.5396	0.7194	
	S	9/18/15	14:40		a.c	10,250		14,750	1			
19	E	9/18/15	14:55	15	286	11,000	750	15,750	1,000	1.6187	0.7194	
	S	9/18/15	14:55		06.1	11,000	0.7.5	15,750		0.5000		
20	Ē	9/18/15	15:10	15	301	11,250	250	16,250	500	0.5396	0.3597	
						,		,	1			1

Note: mL is equal to cm³.

Average Hourly infiltration rate (inches per hour) = 1.1424

					<u>Tt D</u>	ouble Ring	Infiltratio	on Test				
Location: IT-03			Project Name: PPP -			tello Statior	Valve Sites	Date: September 21, 2015				
Inner Ring Diameter: 12		inch	Soil Temperature:			79	degrees	Outer Ring I		1.00	inches	
Inner Ring Area:		729.7	cm^2	Water Temperature:			65	degrees	Inner Ring F		1.00	inches
Outer Ring Diameter:		24	inch	Inner Ring Liquid Depth:			7.75	inches	Test Depth:		0.75	feet
Annular Space Area:		2189	cm^2	Annular Space Liquid Dep		pth:	7.75	inches	Technician:		JR/AB	
				-					-			
Trial	Start/End	Date	Time	delta-Time	Elapsed	Flow Readings			Incremental		I Infiltration	Notes
1					Time	Inner	-		r Space	Inner	Outer	
						Reading	Flow	Reading	Flow	Ring	Ring	
				(min)	(min)	(ml)	(ml)	(ml)	(ml)	(in/hr)	(in/hr)	
1	S	9/21/15	10:45	15	15	0	0	0	3,500	0.00	0.00	
	E	9/21/15	11:00			0		3,500	2,000			
2	S	9/21/15	11:00	- 15	30	0	500	3,500	3,000	1.0791	2.1583	
	E	9/21/15	11:15			500		6,500				
3	S	9/21/15	11:15	15	45	500	250	6,500	2,250	0.5396	1.6187	
	E	9/21/15	11:30			750		8,750				
4	S E	9/21/15 9/21/15	11:30 11:45	15	60 75	750 750	0	8,750 11,250	2,500	0.0000	1.7986	
	S	9/21/15	11:45			750		11,250		┟────┤		
5	E	9/21/15	12:00	15		750	0	13,250	2,000	0.0000	1.4388	
	S	9/21/15	12:00			750		13,250				
6	E	9/21/15	12:00	15	90	750	0	15,500	2,250	0.0000	1.6187	
	S	9/21/15	12:15	15	105	750		15,500			-	
7	E	9/21/15	12:30			750	0	17,250	1,750	0.0000	1.2590	
8	S	9/21/15	12:30	15	100	750		17,250	0.050		4.0407	
	E	9/21/15	12:45	15	120	750	0	19,500	2,250	0.0000	1.6187	
9	S	9/21/15	12:45	45	405	750	0	0	4.050	0.0000	0.0000	Defilled to 0 ml
	E	9/21/15	13:00	15	135	750	0	1,250	1,250 0.000	0.0000	0.8993	Refilled to 0 mL
10	S	9/21/15	13:00	- 15	150	750	0	1,250	2,000	0.0000	1.4388	
	E	9/21/15	13:15			750		3,250	2,000	0.0000	1.4300	
11	S	9/21/15	13:15	- 15	165	750	0	3,250	1,250	0.0000 0.8993	0 8003	
	E	9/21/15	13:30			750		4,500		0.0000	0.0393	
12 13	S	9/21/15	13:30	- 15 - 15	180 195	750	250 0	4,500	2,000	0.5396	1.4388	
	E	9/21/15	13:45			1,000		6,500	2,000			
	S	9/21/15	13:45			1,000		6,500	1,750	0.0000	0 1.2590	
	E	9/21/15	14:00			1,000		8,250	.,			
14	S	9/21/15	14:00	- 15 - 15	210	1,000	250	8,250	1,750	0.5396	1.2590	
	E S	9/21/15 9/21/15	14:15			1,250		10,000	,	<u>├</u>		
15	E	9/21/15	14:15 14:30		225	1,250 1,250	0	10,000 11,500	1,500	0.0000	1.0791	
16	S	9/21/15	14:30	- 16	241	1,250	250	11,500	1	-		
	E	9/21/15	14:30			1,250		13,250	1,750	0.5058	1.1803	
17	S	9/21/15	14:45	- 15	256	1,500	0	13,250				
	E	9/21/15	15:00			1,500		14,500	1,250	0.0000	0.8993	
18	S	9/21/15	15:00	- 15	271	1,500	0	14,500	1			
	E	9/21/15	15:00			1,500		16,500	2,000	0.0000	1.4388	
19	S	9/21/15	15:15	- 15	286	1,500	250	16,500	1			1
	E	9/21/15	15:30			1,750		18,250	1,750	0.5396	1.2590	
	S	9/21/15	15:30	- 15	301	1,750	0	18,250	4			
20	Ē	9/21/15	15:45			1,750		19,500	1,250	0.0000	0.8993	
	-	5, _ 1, 10				.,. 00		. 0,000		1		1

Note: mL is equal to cm³.

Average Hourly infiltration rate (inches per hour) = 0.1328

Location: IT-06 Project Name: PPP - Montello Station Valve Sites Date: September 21, 2015 Inner Ring Diameter: 12 inch Soil Temperature: 77 degrees Outer Ring Penetration: 1.00 inches Outer Ring Diameter: 24 inch Inner Ring Liquid Deph: 7.50 inches Test Deph: 0.50 feat. Annular Space Area: 2180 cmr2 Annular Space Liquid Deph: 7.50 inches Test Deph: 0.50 feat. Trial Stat/End Date Time deta=Time Elapsed Test Deph: noner Outer Ring Ring Ring 1 S. 9/21/15 1215 15 500 500 500 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.000 0.7194 1 5 500 500 500 1.000 0.000 0.7194 1 5 500 500 1.000 0.000 0.7194 <	
Inner Ring Diameter: 12 Inch Soil Temperature: 77 degrees Outer Ring Penetration: 1.00 inches Duter Ring Diameter: 24 inch Inner Ring Penetration: 1.00 inches Quter Ring Diameter: 24 inch Inner Ring Penetration: 0.50 feet Annular Space Area: 2189 cm²2 Annular Space Liquid Depth: 5.00 inches Technician: JRAB Trial Start/End Date Time delta-Time Elapset Flow Reading Flow Reading Flow Reading Flow Reading flow Ring	
Inner Ring Area: 729.7 cm²2 Water Temperature: 64 degrees Inner Ring Penetration: 1.00 inches Annular Space Area: 2189 cm²2 Annular Space Liquid Depth: 5.00 inches TestDepth: 0.50 feet Trial Start/End Date Time delta-Time Elapsed Incer Ring Annular Space Inner Ring	
Outer Ring Diameter: 24 Incher King Liquid Depth: 7.50 inches Test Depth: 0.50 feet Annular Space Area: 2189 cm/2 Annular Space Liquid Depth: 5.00 inches Test Depth: 0.16 chs Test Depth: 0.16 chs Trial Start/End Date Time delta-Time Elapsed Time Flow Reading (min) Flow Reading (min) Annular Space Inceremental Infiltration 1 S 9/21/15 12:00 15 15 0 0 0 0.01 0.000 0.7194 1.00 0.000 0.7194 1.00 1.000 0.0000 0.7194 1.00 0.0000 0.7194 1.01 0.0000 0.7194 1.01 0.0000 0.0000	
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E 9/21/15 16:00 16 241 500 0 13,500 1,000 0.0000 0.6745	
17 E 9/21/15 16:15 15 256 750 250 10,000 500 0.5396 0.3597	
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20 E 9/21/15 17:00 15 301 750 0 16,000 750 0.0000 0.5396	

Note: mL is equal to cm³.

Average Hourly infiltration rate (inches per hour) = 0.0337