DEPARTMENT OF ENVIRONMENTAL PROTECTION

COMMONWEALTH OF PENNSYLVANIA DEPARTMENT OF ENVIRONMENTAL PROTECTION BUREAU OF CLEAN WATER

NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) DISCHARGES OF STORMWATER ASSOCIATED WITH CONSTRUCTION ACTIVITIES EROSION AND SEDIMENT CONTROL (E&S) MODULE 1

Applicant: Orchard BJK Company, LLC.

Project Site Name: PMCC North Warehouse

EV/HQ-CWF,MF

Surface Water Name(s): EV Wetlands to Duckpuddle Run Surface Water Use(s):

E&S PLAN INFORMATION

1. Describe the existing topographic features of the project site and the immediate surrounding area.

The existing site is comprised of woodlands. Runoff from the project area flows generally in a westerly direction towards the Duckpuddle Run which flows to the south through the center of the project tract. There is no offsite contributing area which passes through the project area.

2. Complete the following table for soils present at the project site.

Map Unit Symbol	Map Unit Name	Acres	HSG	% of Disturbed Area	Depth (ft)	Hydric
CnB	Chippewa & Norwich Soils, 0 To 8 Percent Slopes, Extremely Stony	1.40	D	6%	0.5	
МоВ	Morris Channery Silt Loam, 0 To 8 Percent Slopes, Extremely Stony	6.02	D	24%	0.5	
WpB	Wellsboro Channery Loam, 0 To 8 Percent Slopes, Extremely Stony	17.58	D	70%	0.5	

Discuss any soil limitations and how the E&S Plan was designed to address those limitations.

Cutbacks cave – Caution should be taken to limit the amount of trench that is open at any given time. Shoring should be used as recommended by the respective governmental safety agencies.

Corrosive to Concrete– Suitable precautions should be taken to protect the proposed storm sewer structures and water storage tanks from corrosion due to the soils.

Corrosive to Steel – Exposed steel shall be provided with a protective coating and pipe materials will be resistant to corrosion.

Easily erodible – Earth disturbance will be limited to the immediate construction area and disturbed surfaces will be stabilized immediately. Erosion Control Blanket will be provided on slopes 3:1. Rip-Rap Aprons shall be provided at all pipe outfalls. Fill slopes will be tracked cross-slope to deter erosion. All disturbed areas will be monitored on a regular basis and after all storm events.

Depth to Saturated/ Seasonal High-Water Table - If groundwater is encountered during construction, the area will be de-watered by pumping the groundwater into a "Pumped Water Filter Bag" silt control system.

Hydric/ hydric inclusions – The existing wetlands have been delineated within the project boundary. Wetlands located outside of the proposed limit of disturbance will not be disturbed during the construction of the project.

Low strength / landslide prone – All slopes will be stabilized by permanent vegetative or impervious cover and/or by turf reinforcement or erosion control mats and blankets.

Slow percolation – Infiltration testing was performed to confirm the suitability of the soils for the proposed BMP's. Additionally, the installation of the Infiltration BMP's shall be limited to towards the end of construction.

Piping - The proposed basin berms have been limited in height and compacted to 98% modified proctor.

Poor Source of Topsoil (Rock Fragments) – Topsoil shall be used from areas relatively free of rock fragments. Should a suitable source of topsoil not be found, existing topsoil shall be screened to remove rock fragments or imported.

Poor Source of Topsoil (Too Acid) – A representative soil test shall be performed to determine the necessary amount of lime to be used.

Shrink-Swell/Frost Action - If winter grading is unavoidable, excavation shall take place when the temperature is above freezing and the soil is dry.

Wetness – If soils are not suitable for the specific onsite application, as determined by inspection by the contractor, then suitable soil shall be derived from suitable onsite material or imported.

If Hydric soils are present, is a wetland determination attached to this module? Xes No N/A

If soils are known to be contaminated, 1) identify the pollutants exceeding Act 2 standards in the space provided below, 2) identify the extent of soil contamination on an E&S Plan Drawing that is attached to this module, and 3) describe the methods that will be used to avoid or minimize disturbance of the contaminated soils in the space provided below.

There are no known soil contaminents present within the project area.

3. Describe the characteristics of the earth disturbance activity, including the past, present and proposed land uses and the proposed alteration to the project site.

The proposed alteration to the project site, as described in Project Description section of this Narrative, will occur primarily in areas consisting of wooded cover conditions with slopes ranging from 0 - 8 percent according to the soil survey. The proposed earthmoving activities include cut and fill operations to construct the proposed improvements, however, it is anticipated that the project site will be balanced (no import or export of material). It is anticipated that excess topsoil, if applicable, will remain onsite to the extent practicable. Site design and layout employ an environmentally sensitive approach that minimizes the effect of land development on surrounding vegetation to the maximum extent practicable. Surface runoff patterns are not anticipated to be adversely impacted by the proposed alteration to the project site.

The past or historical land use(s) for the past 50 years or longer based on a review of historic aerial photographs of Pennsylvania obtained from www.pennpilot.psu.edu appear to be an undeveloped use as of 1971. The present or existing land use for the past five (5) years preceding the planned project is undeveloped. The proposed land use is commercial.

4. Describe the volume and rate of runoff from the project site and its upstream watershed area.

Runoff from within the limit of disturbance shall be directed to either the sediment basin or sediment trap. There is no offsite contributing area which passes through the project area.

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5. C	5. Check boxes to indicate all BMPs that will be installed or implemented, identify plan numbers for the BMPs, and describe any deviations from the E&S Manual.				
	E&S BMPs	Plan No(s). Identified	Plan No(s). for O&M	Deviation(s) from E&S Manual	
	Rock Construction Entrance	8	10	RCE lengthened to 100' for ABACT	
	Rock Construction Entrance with Wash Rack				
	Rumble Pad				
	Wheel Wash				
	Temporary and Permanent Access Roads				
	Waterbar				
	Broad-based Dip				
	Open-top Culvert				
	Water Deflector				
	Roadside Ditch				
	Ditch Relief Culvert				
	Turnout				
	Compost Sock Sediment Trap				
	Temporary Stream Crossing				
	Temporary Wetland Crossing				
	Turbidity Barrier (Silt Curtain)				
	Dewatering Work Areas				
\boxtimes	Pumped Water Filter Bag		11		
	Sump Pit				
	Waste Management				
\square	Concrete Washout	8	11		
\boxtimes	Compost Filter Sock	8	11		
	Compost Filter Berm				
	Weighted Sediment Filter Tube				
	Rock Filter Outlet				
	Silt Fence (Filter Fabric Fence)				
	Reinforced Silt Fence				
	Super Silt Fence (Super Filter Fabric Fence)				

E&S BMPs	Plan No(s). Identified	Plan No(s). for O&M	Deviation(s) from E&S Manual
Sediment Filter Log (Fiber Log)			
Wood Chip Filter Berm			
Straw Bale Barrier			
Rock Filter			
Vegetative Filter Strip			
Inlet Filter Bag			
Stone Inlet Protection			
Runoff Conveyance (Channel)	8	11	Basin outfalls have been with rip-rap lined channels to convey runoff to wetlands
Bench			
Top-of-Slope Berm			
Temporary Slope Pipe			
🛛 Sediment Basin	8	11	Basins have been provided with skimmers
Sediment Trap			
🛛 Riprap Apron	8	16	
Flow Transition Mat			
Stilling Basin (Plunge Pool)			
Stilling Well			
Energy Dissipater			
Drop Structure			
Earthen Level Spreader			
Structural Level Spreader			
Surface Roughening			
☑ Vegetative Stabilization	8	11-12	
Erosion Control Blanket	8	11-12	
Soil Binders			
Sodding			
Cellular Confinement Systems			
Alternative:			
Alternative:			

Table 1 – For PAG-01 applicants, complete the requested information for each selected E&S BMP, where applicable.

Site Access BMPs											
BMP Name	No.	Length (ft)	Width (ft)	% Slope	Spacing (ft)	Le U Dra	ength of Ipslope ainage (ft)	Culvert Diameter (in)	Soil Ty	pe in Ditch	E&S Manual Figure/Detail No.
Rock Construction Entrance (RCE)											
RCE with Wash Rack											
Temporary and Permanent Access Roads – Crowned Roadway											
Temporary and Permanent Access Roads – Insloped Roadway											
Waterbar											
Broad-based Dip											
Open-top Culvert											
Water Deflector											
Roadside Ditch											
Ditch Relief Culvert											
Sediment Barriers / Filters											
BMP Name	DA (a	c) Dian	neter (in)	Storage Capacity (cf)	Trap Heiថ (in)	ght	% Slope	Slope L Above Ba	ength arrier (ft)	Barrier Height (in)	E&S Manual Figure/Detail No.
Compost Sock Sediment Trap											
Compost Filter Sock											
Compost Filter Berm											
Silt Fence (Filter Fabric Fence)											
Super Silt Fence											
Sediment Filter Log											
Weighted Sediment Filter Tube											
Straw Bale Barrier											
Wood Chip Filter Berm											
Toe-of-Slope Berm											

Table 1 – For PAG-01 applicants, complete the requested information for each selected E&S BMP, where applicable.

Runoff Conveyance	e BMPs																
BMP Name	Temporary	Design Storm	DA (a	c) Multipli	er Q	Qr (cfs)	Q (cfs)	Man	ning's n	Va (fps	1 5)	V (fps)	D (fl	t) d (ft) Flov Dep Rati	w th o	E&S Manual Figure/Detail No.
Vegetated Channel																	
Sodded Channel																	
Riprap Channel																	
Energy Reduction	BMPs		•		•			•						·			
BMP Name	Downstream to Drainage	Distance Course (ft)	Dowr	istream % Slope	D)A (ac)	Disch (cf	iarge s)	Man Dept	hole th (ft)		Inflow Diamet	Pipe er (in)	Outl Diam	et Pipe eter (in)	F	E&S Manual Figure/Detail No.
Level Spreader																	
Drop Structure																	
Stilling Basins / We	Stilling Basins / Wells																
BMP Name	Pipe Diameter (in)	Discharg	je (cfs)	Well Diame (in)	eter	Depth Below I	of Well nvert (ft)	Basi	n Depth	(ft)	Me	dian Rip Size (in)	orap	Distan Dischar to Basir (f	ce from ge Pipe n Center t)	F	E&S Manual Figure/Detail No.
Stilling Basin																	
Stilling Well																	
Other BMPs																	
BMP Name	DA (ac)	Pipe Diameter (in)	Berm Height (in)	Length (ft)	% Slope	Verti Spac (ft	cal C ing De	hannel epth (ft)	Rip Si	orap ize	TI	Riprap hickness (in)	s w	Initial 'idth (ft)	Termir Width	nal (ft)	E&S Manual Figure/Detail No.
Temporary Slope Pipe																	
Bench																	
Rock Filter																	
Riprap Apron																	

For will	selected BMPs not identified in T be used for design and implement	Table 1, report the name of th	ne BMP and the Figure or Detail No.	from the E&S Manual that		
	BMP Name	E&S Manual Figure/Detail No.	BMP Name	E&S Manual Figure/Detail No.		
6.	All applicable Standard E&S	Worksheets from Appendix I	B of the E&S Manual have been com	pleted and are attached.		
7.	Other worksheets or calculat	ions equivalent to Appendix	B of the E&S Manual have been com	pleted and are attached.		
8.	Identify the E&S Plan Drawing scheduling of earth disturbance functioning of all BMPs.	number(s) that describes the activities, prior to, during	e sequence of BMP installation and and after earth disturbance activitie	removal in relation to the es that ensure the proper		
	12					
9.	Supporting E&S calculations	have been completed and a	re available upon request (PAG-01 o	only).		
10.	Supporting E&S calculations	are attached to the NOI/app	lication.			
11.	Plan drawings consist of star	ndard Figures/Construction D	etails in E&S Manual (PAG-01 only)			
12.	Plan drawings have been de	veloped for the project and a	re attached to the NOI/application.			
13.	BMPs will be inspected on a	weekly basis and after meas	surable storm events (i.e., at least 0.2	25 inch).		
14.	Identify the following informatio Drawing No. below: 1) vegetat application rate, 6) mulch type, 7	n relating to temporary stab ive species, 2) % pure live) mulching rate, and 8) liming	ilization measures on an E&S Plan seed, 3) seed application rate, 4) g rate.	Drawing and identify the fertilizer type, 5) fertilizer		
	E&S Plan Drawing No(s).: 12					
15.	Identify the following information Drawing No. below: 1) vegetat application rate, 6) mulch type, 7 material application, 12) topsoil	n relating to permanent stab ive species, 2) % pure live) mulching rate, 8) liming rate placement depth, and 13) see	pilization measures on an E&S Plar seed, 3) seed application rate, 4) e, 9) anchor material, 10) anchoring eding season dates.	Drawing and identify the fertilizer type, 5) fertilizer method, 11) rate of anchor		
	E&S Plan Drawing No(s).: 12					
16.	Describe the procedures that w project site will be conducted pro	rill be taken to ensure that r operly.	ecycling or disposal of materials as	ssociated with or from the		
	1. Anticipated construction limited to excess soil materials impact water quality. Addition construction sequence. Mea housekeeping, materials mana	wastes: building materia s, building materials, conci nal waste material due to d asures should be planned agement, and litter control.	Is and other construction site ware rete wash water, sanitary wastes, e emolition of existing improvemen a and implemented by the perm	astes, including but not etc. That could adversely ts is specified in the site littee or co-permitte for		
	2. Construction waste mater potential for pollution to surf recycling of materials, proper construction site wastes to be	rials shall be properly man ace and ground waters as r materials handling, and mobilized by stormwater r	naged per note 3 and recycled o per 25 pa. Code § 102.4(b)(5)(xi spill prevention and clean-up i runoff and conveyed to surface wa	r disposed of to reduce). Proper trash disposal, reduce the potential for ters.		
	3. All building materials and with the department's solid w No building materials or wast the site.	wastes shall be removed aste management regulation es or unused building mains	from the site and recycled or dis ons at 25 pa. Code 260.1 et seq., terials shall be burned, buried, du	sposed of in accordance 271.1, and 287.1 et. Seq. umped, or discharged at		
	4. Wherever possible, recycl	ing of excess materials is p	preferred, rather than disposal.			
	5. All applicable federal, stat of potentially hazardous mate	e, and local laws and regul rials.	ations must be followed in the use	e, nandling, and disposal		
	6. Under no circumstances construction wastes.	may erosion control bmps	be used for temporary storage of	f demolition materials or		
	7. All sediment removed from from bmps shall be disposed	n bmps shall be handled in of in landscaped areas o ~	the manner described in these ne utside of steep slopes, wetlands,	otes. Sediment removed floodplains or drainage		

swales and immediately stabilized, placed on topsoil stockpiles, or hauled off-site to a permitted construction or storage site.

8. Concrete wash water shall be handled in the manner described in these notes. In no case shall it be allowed to enter any surface waters, groundwater systems or storm sewer systems.

9. All off-site waste and borrow areas must have an e&s plan approved by the local conservation district or dep fully implemented prior to being activated.

17. Identify the presence of any naturally occurring geologic formations or soil conditions that may have the potential to cause pollution during earth disturbance activities. If such formations or conditions exist, identify BMPs that will be implemented to avoid or minimize potential pollution.

N/A

18. Identify whether the potential exists for thermal impacts to surface waters from the earth disturbance activity. If such potential exists, identify BMPs that will be implemented to avoid, minimize, or mitigate potential thermal impacts.

Potential thermal impacts to surface waters of the Commonwealth include the following activities:

Exposure of stormwater to surfaces, specifically pavement and rooftops, with elevated temperatures prior to reaching surface waters. During construction, BMPs and/or construction practices shall be implemented to avoid, minimize or mitigate the potential to cause pollution from thermal impacts are as follows:

- Collected Runoff shall flow through deep storm sewer prior to discharge from the site.

- The majority of the site runoff shall flow through a sediment control facility where it will be contained and discharged slowly though a vegetated area prior to discharge from the site.

19. 🛛 The E&S	19. 🛛 The E&S Plan has been planned, designed, and will be implemented to be consistent with the PCSM Plan.					
20. If applicable, Drawing No(s	identify existings) below (select	g and proposed riparian N/A if not applicable).	forest buffers on E&	S and PCSM Plan Drawings and identify the		
E&S Plan Dra	awing No(s):	Plan 8 & 9	□ N/A			
PCSM Plan	Drawing No(s):	Plan 13 & 14				
		E&S PL	AN DEVELOPER			
I am trained a	and experienced	in E&S control methods.	🛛 I am a licen	sed professional.		
Name:	Michael Hartr	nan	Title:	Project Engineer		
Company:	Schlouch, Inc	corporated	Phone No.:	484-663-3487		
Address:	P.O. Box 69	1	Email:	mhartman@schlouch.com		
City, State, ZIP:	Blandon, PA	19510	License No.:	PE 059897		
License Type:	Professional	Engineer /	Exp. Date:	9/23		
E&S Plan Developer Signature Date						
	1					



WORKSHEET #12 Sediment Basin Capacity Requirements

PROJECT NAME:	PMCC North Warehouse			
LOCATION:	Coolbaugh Township, Monroe County			
PREPARED BY:	MDH	DATE:	12/8/2022	2
CHECKED BY:		DATE:		
BASIN NUMBER			1	
PERMANENT OR T	EMPORARY BASIN?	(P or T)	Р	
SPECIAL PROTEC	TION WATERSHED?	(YES OR NO)	N	

SPECIAL PROTECTION WATERSHED?	(YES OR NO)	N	
Karst soils?	(YES OR NO)	N	
(A) MAXIMUM TOTAL DRAINAGE AREA	(AC)	14.6	
IS DRAINAGE AREA (A) MORE THAN 10% LARGER THAN THE		N	
PRECONSTRUCTION CONDITION?	(YES OR NO)	14	
(A1) DISTURBED ACRES IN DRAINAGE AREA	(AC)	14.6	
(I) INITIAL REQ'D DEWATERING ZONE (5,000 X A)	(CF)	72973	
(T) REDUCTION FOR TOP DEWATERING (-700 X A)	(CF)	0	
(P) REDUCTION FOR PERMANENT POOL (-700 X A)	(CF)	0	
(L) REDUCTION FOR 4:1 FLOW LENGTH:WIDTH (-350 X A)	(CF)	0	
(D) REDUCTION FOR 4 TO 7 DAY DEWATERING (- 350 X A)	(CF)	0	
(Sv) REQUIRED DEWATERING ZONE [I - (T+P+L+D)] ¹	(CF)	72973	
(Sd) REQUIRED SEDIMENT STORAGE VOLUME (1000 X A1) (CF)	(CF)	14595	
(St) TOTAL REQUIRED STORAGE VOLUME (Sv + Sd)	(CF)	87567	
TOTAL STORAGE VOLUME PROVIDED (@ ELEV 3) ²	(CF)	102853	
DEWATERING TIME FOR DEWATERING ZONE (DAYS)	(DAYS)	5.7	
REQUIRED DISCHARGE CAPACITY (2 X A)	(CFS) ³	29.19	
PRINCIPAL SPILLWAY TYPE (PERFORATED RISER, SKIMMER, etc.)		Skimmer	
PEAK FLOW FROM 10 YR/24 HR STORM FOR DRAINAGE AREA	(A)	N/A	
PRINCIPAL SPILLWAY CAPACITY (@ ELEV 5)	(CFS) ⁴	32.03	
EMERGENCY SPILLWAY CAPACITY (@ ELEV 5)	(CFS) ⁴	0.00	
TOTAL BASIN DISCHARGE CAPACITY (@ ELEV 5)	(CFS)	32.03	
EMERGENCY SPILLWAY PROTECTIVE LINING ⁵		SC 250	
OUTLET TO A SURFACE WATER?	(YES OR NO) ⁶	Y	
PEAK FLOW FROM A 100 YR/24 HR STORM FOR DRG. AREA	(CFS)	N/A	

- 1 The minimum dewatering zone capacity for sediment basins is (3,600 X A). No reduction is permitted in Special Protection (HQ and EV) Watersheds.
- 2 Total Storage Volume provided at riser crest.
- 3 Or provide calculations to show peak flow from 25 yr./24 hr. storm for area (A) is routed through the basin.
- 4 Provide supporting computations.
- 5 If grass lining is proposed, spillway should be constructed in original ground unless a suitable TRM lining is used. Wherever a TRM is used, riprap should be placed at the bottom of the embankment to prevent scour.
- 6 If no, and basin is permanent or drainage area is more than 10% larger than pre-construction, provide supporting calculations to show accelerated erosion will not result from the proposed discharge. For discharges increasing volume or rate of flow onto a neighboring property prior to entering a surface water, an easement should be obtained prior to plan submittal.

WORKSHEET #13 Sediment Basin Dimensions And Elevations

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	
	2		



BASI	N NUMBER		1	
1.	DISCHARGE PIPE ELEVATION	(FT)	1949.37	
2.	TOP OF SEDIMENT STORAGE ZONE (@ Sd)	/ F T)	1052 40	
	(MIN. 1.0' ABOVE ELEVATION 7)	(FI)	1953.40	
3.	ELEVATION AT TOP OF DEWATERING ZONE (St)	(FT)	4054.40	
	(CREST OF PRINCIPAL SPILLWAY)	(FI)	1954.48	
4.	EMERGENCY SPILLWAY CREST ELEVATION	(57)	4050.05	
	(MIN. 0.5' ABOVE ELEVATION 3)	(+1)	1956.95	
5.	2 CFS/ACRE OR 25-YR/24-HR FLOW ELEVATION		1955.30	
6.	TOP OF EMBANKMENT ELEVATION			
	(MIN. 24" ABOVE ELEVATION 5	(FT)	1958.45	
	OR 12" WITH ROUTED 100-YR/24-HR STORM)			
7.	BOTTOM ELEVATION	(FT)	1952.33	
	AVERAGE BOTTOM WIDTH (FT)	(FT)	135	
	AVERAGE BOTTOM LENGTH (FT)	(FT)	330	
	(SAmin) REQUIRED SURFACE AREA AT ELEVATION 2	(SQ. FT.)	47100	
	SURFACE AREA PROVIDED AT ELEVATION 2	(SQ. FT.)	47100	
	AVERAGE BASIN WIDTH (W) AT ELEVATION 3	(FT)	155	
	FLOW LENGTH (L) AT ELEVATION 3	(FT)	314	
	FLOW LENGTH: WIDTH RATIO AT ELEVATION 3	(L/W)	2.0	
	SILT CURTAIN OR FOREBAY? (IF YES, INDICATE WHICH)	(FT)	N/A	
	EMBANKMENT TOP WIDTH (FT, 8' MIN.)	(FT)	8	
	EMBANKMENT SOIL TYPE(S)		Loam	
	KEY TRENCH DEPTH (FT, 2' MIN.)		2	
	KEY TRENCH WIDTH (FT, 4' MIN.)		4	
	RISER DIAMETER/TYPE (15" MIN.)	(FT)	N/A	
	BARREL DIAMETER/TYPE (12" MIN.)	(FT)	30	
	Lb (BARREL LENGTH (FT)	(FT)	18	
	EMERGENCY SPILLWAY WIDTH (FT)	(FT)	150	
	EMERGENCY SPILLWAY SIDE SLOPES (H:V)	(FT)	3	
	EMERGENCY SPILLWAY DEPTH (FT)		N/A	

For irregular shaped traps, provide stage storage data

WORKSHEET #14 Sediment Basin Storage Data

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	

WATER				STORAGE VOLUME	
SURFACE	AREA	AVERAGE	DIFFERENCE	(CUBIC FEET)	
ELEVATION		AREA	IN ELEVATION		
(FEET)	(SQ. FT.)	(SQ.FT.)	(FEET)	INCREMENTAL	TOTAL
1952.33	43457	0	0	0	0
1953	45717	44587	0.67	29873	29873
1954	49192	47454.5	1	47455	77328
1956	56431	52811.5	2	105623	182951
1958	63888	60159.5	2	120319	303270



Schlouch Incorporated

PO Box 69 Blandon, PA 19510 Phone: 610-926-7070

Sediment Basin Skimmer Desgin				
2-cfs/acre Elevation:	1955.30			
Cleanout Elevation:	1953.40			
Dewatering Storage Volume:				
Volume provided at 1,955.30	145983 cu. ft.			
Volume provided at 1,953.40	45533 cu. ft.			
	100449 cu. ft.			
	2.75			
Orifice Size:	3./5 in.			
Downstoring Time	57 dava			
	J./ days			
Tube Invert:	1953 25			
Tube mivert.	1755.25			
Elevation Difference - (2-cfs/acre Elevation: - Tube Invert:)	2.0 ft			
	2.0 10			
Arm Length:	2.9 ft.			

WORKSHEET #17 Sediment Basin Discharge Capacity

PROJECT NAME:	PMCC North Warehouse				
LOCATION:	Coolbaugh Township, Monroe County				
PREPARED BY:	MDH	DATE:	12/8/2022		
CHECKED BY:		DATE:			

PRINCIPAL SPILLWAY DISCHARGE CAPACITY

BASIN NO:

1

WATER
SURFACE
ELEVATION ⁴
(FT)
1955.30

Flow into Top of			BAR	REL	
PERMANENT RISER		PIPE FLOW		PRINCIPAL	
	ORIFICE	WEIR			SPILLWAY
HEAD	FLOW ¹	FLOW	HEAD ²	Q	CAPACITY
(FT)	Q (CFS)*	Q (CFS)	(FT)	(CFS)	(CFS) ³
0.82	52.22	32.03	4.68	53.98	32.03

EMERGENCY SPILLWAY DISCHARGE CAPACITY

	EMERGENCY				TOTAL
WATER	SPILLWAY	TABLE OR	EMERGENCY	REQUIRED	DISCHARGE
SURFACE	BOTTOM	C VALUE	SPILLWAY	DISCHARGE	CAPACITY
ELEVATION ⁴	WIDTH⁵	USED ⁶	CAPACITY	CAPACITY	PROVIDED
(FT)	(FT)		(CFS)	(CFS)	(CFS)
1955.30	150.00	2.8	0	29.19	32.03

1. Flow into top of riser only (Flow through perforations not included)

2. Water surface elevation minus elevation at centerline of pipe outlet

- 3. Least of orifice, weir, or pipe flow (Peak flow from 10 yr/24 hr storm Min.)
- 4. 24" below top of embankment (12" if 100-year storm routed through basin)
- 5. 8 Ft. minimum
- 6. Use Tables 7.5 through 7.8 or equation for broad-crested weir [Q = CLH1. 5, where C < 2.8 (MAX)]; for
- 7. Principal Spillway Capacity + Emergency Spillway Capacity

WORKSHEET #12 Sediment Basin Capacity Requirements

PROJECT NAME:	PMCC North Warehouse			
LOCATION:	Coolbaugh Township, Monroe County			
PREPARED BY:	MDH	DATE:	12/8/2022	
CHECKED BY:		DATE:		
BASIN NUMBER			2	

PERMANENT OR TEMPORARY BASIN?	(P or T)	Р	
SPECIAL PROTECTION WATERSHED?	(YES OR NO)	N	
Karst soils?	(YES OR NO)	N	
(A) MAXIMUM TOTAL DRAINAGE AREA	(AC)	7.3	
IS DRAINAGE AREA (A) MORE THAN 10% LARGER THAN THE		N	
PRECONSTRUCTION CONDITION?	(YES OR NO)	IN	
(A1) DISTURBED ACRES IN DRAINAGE AREA	(AC)	7.3	
(I) INITIAL REQ'D DEWATERING ZONE (5,000 X A)	(CF)	36700	
(T) REDUCTION FOR TOP DEWATERING (-700 X A)	(CF)	0	
(P) REDUCTION FOR PERMANENT POOL (-700 X A)	(CF)	0	
(L) REDUCTION FOR 4:1 FLOW LENGTH:WIDTH (-350 X A)	(CF)	0	
(D) REDUCTION FOR 4 TO 7 DAY DEWATERING (- 350 X A)	(CF)	0	
(Sv) REQUIRED DEWATERING ZONE [I - (T+P+L+D)] ¹	(CF)	36700	
(Sd) REQUIRED SEDIMENT STORAGE VOLUME (1000 X A1) (CF)	(CF)	7340	
(St) TOTAL REQUIRED STORAGE VOLUME (Sv + Sd)	(CF)	44040	
TOTAL STORAGE VOLUME PROVIDED (@ ELEV 3) ²	(CF)	51991	
DEWATERING TIME FOR DEWATERING ZONE (DAYS)	(DAYS)	4.6	
REQUIRED DISCHARGE CAPACITY (2 X A)	(CFS) ³	14.68	
PRINCIPAL SPILLWAY TYPE (PERFORATED RISER, SKIMMER, etc.)		Skimmer	
PEAK FLOW FROM 10 YR/24 HR STORM FOR DRAINAGE AREA	(A)	N/A	
PRINCIPAL SPILLWAY CAPACITY (@ ELEV 5)	(CFS) ⁴	15.17	
EMERGENCY SPILLWAY CAPACITY (@ ELEV 5)	(CFS) ⁴	0.00	
TOTAL BASIN DISCHARGE CAPACITY (@ ELEV 5)	(CFS)	15.17	
EMERGENCY SPILLWAY PROTECTIVE LINING ⁵		SC 250	
OUTLET TO A SURFACE WATER?	(YES OR NO) ⁶	Y	
PEAK FLOW FROM A 100 YR/24 HR STORM FOR DRG. AREA	(CFS)	N/A	

- 1 The minimum dewatering zone capacity for sediment basins is (3,600 X A). No reduction is permitted in Special Protection (HQ and EV) Watersheds.
- 2 Total Storage Volume provided at riser crest.
- 3 Or provide calculations to show peak flow from 25 yr./24 hr. storm for area (A) is routed through the basin.
- 4 Provide supporting computations.
- 5 If grass lining is proposed, spillway should be constructed in original ground unless a suitable TRM lining is used. Wherever a TRM is used, riprap should be placed at the bottom of the embankment to prevent scour.
- 6 If no, and basin is permanent or drainage area is more than 10% larger than pre-construction, provide supporting calculations to show accelerated erosion will not result from the proposed discharge. For discharges increasing volume or rate of flow onto a neighboring property prior to entering a surface water, an easement should be obtained prior to plan submittal.

WORKSHEET #13 Sediment Basin Dimensions And Elevations

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	
	2		



BASI	N NUMBER		2	
1.	DISCHARGE PIPE ELEVATION	(FT)	1945.83	
2.	TOP OF SEDIMENT STORAGE ZONE (@ Sd)	(FT)	1017 70	
	(MIN. 1.0' ABOVE ELEVATION 7)	(F1)	1947.73	
3.	ELEVATION AT TOP OF DEWATERING ZONE (St)	(57)	40.40.00	
	(CREST OF PRINCIPAL SPILLWAY)	(+1)	1949.00	
4.	EMERGENCY SPILLWAY CREST ELEVATION	()	1010 55	
	(MIN. 0.5' ABOVE ELEVATION 3)	(+1)	1949.55	
5.	2 CFS/ACRE OR 25-YR/24-HR FLOW ELEVATION		1949.55	
6.	TOP OF EMBANKMENT ELEVATION			
	(MIN. 24" ABOVE ELEVATION 5	(FT)	1951.55	
	OR 12" WITH ROUTED 100-YR/24-HR STORM)	、 <i>、 、</i>		
7.	BOTTOM ELEVATION	(FT)	1946	
	AVERAGE BOTTOM WIDTH (FT)	(FT)	52	
	AVERAGE BOTTOM LENGTH (FT)	(FT)	220	
	(SAmin) REQUIRED SURFACE AREA AT ELEVATION 2	(SQ. FT.)	17409	
	SURFACE AREA PROVIDED AT ELEVATION 2	(SQ. FT.)	17409	
	AVERAGE BASIN WIDTH (W) AT ELEVATION 3	(FT)	70	
	FLOW LENGTH (L) AT ELEVATION 3	(FT)	244	
	FLOW LENGTH: WIDTH RATIO AT ELEVATION 3	(L/W)	3.5	
	SILT CURTAIN OR FOREBAY? (IF YES, INDICATE WHICH)	(FT)	N/A	
	EMBANKMENT TOP WIDTH (FT, 8' MIN.)	(FT)	8	
	EMBANKMENT SOIL TYPE(S)		Loam	
	KEY TRENCH DEPTH (FT, 2' MIN.)		2	
	KEY TRENCH WIDTH (FT, 4' MIN.)		4	
	RISER DIAMETER/TYPE (15" MIN.)	(FT)	N/A	
	BARREL DIAMETER/TYPE (12" MIN.)	(FT)	24	
	Lb (BARREL LENGTH (FT)	(FT)	18	
	EMERGENCY SPILLWAY WIDTH (FT)	(FT)	80	
	EMERGENCY SPILLWAY SIDE SLOPES (H:V)	(FT)	3	
	EMERGENCY SPILLWAY DEPTH (FT)		N/A	

For irregular shaped traps, provide stage storage data

WORKSHEET #14 Sediment Basin Storage Data

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	

WATER				STORAGE	E VOLUME
SURFACE	AREA	AVERAGE	DIFFERENCE	(CUBIC FEET)	
ELEVATION		AREA	IN ELEVATION		
(FEET)	(SQ. FT.)	(SQ.FT.)	(FEET)	INCREMENTAL	TOTAL
1946	13830	0	0	0	0
1948	17975	15902.5	2	31805	31805
1950	22397	20186	2	40372	72177
1952	27118	24757.5	2	49515	121692
1954	32140	29629	2	59258	180950



Schlouch Incorporated

PO Box 69 Blandon, PA 19510 Phone: 610-926-7070

Sediment Basin Skimmer Desgin				
	4040.55			
2-cts/acre Elevation:	1949.55			
Cleanout Elevation:	1947.73			
Volume provided at 1 040 55	(102(1 m ft			
Volume provided at 1,949.55	15903 cu ft			
volume provided at 1,947.75	45134 cu ft			
	73137 Cu. II.			
Orifice Size:	3 in			
Office Size.	5 111.			
Dewatering Time	4.6 days			
	110 44490			
Tube Invert:	1947.20			
Elevation Difference - (2-cfs/acre Elevation: - Tube Invo	ert:) 2.3 ft.			
	,			
Arm Length:	3.3 ft.			

WORKSHEET #17 Sediment Basin Discharge Capacity

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	

PRINCIPAL SPILLWAY DISCHARGE CAPACITY

BASIN NO:

2

WATER
SURFACE
ELEVATION ⁴
(FT)
1949.55

Flow into Top of			BAR		
PERMANENT RISER		PIPE FLOW		PRINCIPAL	
	ORIFICE	WEIR			SPILLWAY
HEAD	FLOW ¹	FLOW	HEAD ²	Q	CAPACITY
(FT)	Q (CFS)*	Q (CFS)	(FT)	(CFS)	(CFS) ³
0.55	28.57	15.17	2.72	25.98	15.17

EMERGENCY SPILLWAY DISCHARGE CAPACITY

	EMERGENCY				TOTAL
WATER	SPILLWAY	TABLE OR	EMERGENCY	REQUIRED	DISCHARGE
SURFACE	BOTTOM	C VALUE	SPILLWAY	DISCHARGE	CAPACITY
ELEVATION ⁴	WIDTH⁵	USED ⁶	CAPACITY	CAPACITY	PROVIDED
(FT)	(FT)		(CFS)	(CFS)	(CFS)
1949.55	80.00	2.8	0	14.68	15.17

1. Flow into top of riser only (Flow through perforations not included)

2. Water surface elevation minus elevation at centerline of pipe outlet

- 3. Least of orifice, weir, or pipe flow (Peak flow from 10 yr/24 hr storm Min.)
- 4. 24" below top of embankment (12" if 100-year storm routed through basin)
- 5. 8 Ft. minimum
- 6. Use Tables 7.5 through 7.8 or equation for broad-crested weir [Q = CLH1. 5, where C < 2.8 (MAX)]; for

7. Principal Spillway Capacity + Emergency Spillway Capacity

Sediment Basin Baffle Design

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	12/8/2022
CHECKED BY:		DATE:	

CONSTRUCTION DETAIL



	Ba	ffle	Temporary Riser Bottom		
	Length	Height	Crest ELEV	Bottom ELEV	
Basin or	Bal	Bah	TRCE	BE	
Trap No.	(FT)	(FT)	(FT)	(FT)	
1	132	2.2	1954.48	1952.33	
2	184	3.0	1949.00	1946.00	

STANDARD WORKSHEET #5A Silt Soxx Sediment Barrier

PROJECT NAME:	PMCC North Warehouse		
LOCATION:	Coolbaugh Township, Monroe County		
PREPARED BY:	MDH	DATE:	8/25/2022
CHECKED BY:		DATE:	

CONSTRUCTION DETAIL



				Actual	Allowable
		Soxx	Slope-	Slope	Slope
Barrier No.	Location	Height	Percent	Length	Length
1	North-East side of Basin 1	24	0.026	920	1168
2	East side of Basin 1	24	0.031	860	1053
3	North-East side of Basin 2	24	0.056	486	622
4	South-East side of Basin 2	24	0.084	250	480
5	South of truck parking area	24	0.067	300	567
6	Along S.R. 611 widening	12	0.172	64	150

Schlouch IncorporatedProject:PMCC North WarehousePO Box 69Order No.:20402110Blandon, PA 19510Calc. by:MDHPhone: 610-926-7070Date:12/8/2022

WORKSHEET #11

CHANNEL DESIGN DATA

Channel or Channel Section		1	1	
Temporary or Permanent	(T OR P)	Perm	Perm	
Design Storm	(2,5, 10 YR)	100-year	10-year	
Acres	(AC)	N/A	N/A	
Multiplier	(1.6, 2.25, or 2.75) ¹	N/A	N/A	
Q _r (Required Capacity)	(CFS)	22.43	22.43	
Q (Calculated at flow depth,d)	(CFS)	19.46		
Protective Lining ²		R-4	R-4	
V _a (Allowable Velocity)	(FPS)	N/A	N/A	
V (Calculated At Flow Depth, d)	(FPS)	2.90		
τ_a (Max allowable shear stress)	(LB/FT ²)	2.00	2.00	
τ_d (Calculated At Flow Depth, d)	(LB/FT ²)	1.13	0.14	
Channel Bottom Width	(FT)	10.00	10.00	
Channel Side Slopes	(H:V)	2.00	2.00	
D (Total Depth)	(FT)	1.00		
Channel Top Width (ft)@ D	(FT)	14.00		
d (Calculated Flow depth in ft)	(FT)	0.60	0.07	
Channel Top Width (ft)@ d	(FT)	12.40		
Bottom Width:Depth Ratio	(12:1 Max)	16.67		
d ₅₀ Stone Size (in)	(IN)	6.00	6.00	
A (Area in sq. ft.)	(SQ. FT.)	6.72	0.72	
n (Manning's Coefficient) ²		0.0585		
R (Hydraulic Radius)		0.53		
S (Bed Slope) ³	(FT/FT)	0.03		
S _c (Critical Slope)	(FT/FT)	0.063		
.7S _c	(FT/FT)	0.044		
1.3S _c	(FT/FT)	0.082		
Stable Flow? (Y/N)	(Y/N)	Y		
Freeboard Based On Unstable Flow	(FT)	N/A		
Freeboard Based On Stable Flow	(FT)	0.50		
Minimum Required Freeboard ⁴	(FT)	0.50		
Design Method For Protective Lining ⁵		s	9	
Permissible Velocity (V) Or Shear Stress (S)		0	5	

 1. Use 1.6 for Temporary Channels; 2.25 for Temporary Channels in Special
 Protection (HQ or EV) Watersheds;

 2.75 for Permanent Channels. For Rational Method, enter "N/A" and attach
 E&S Worksheets 9 and 10.

 For TR-55 enter "N/A" and attach appropriate Worksheets.
 E&S Worksheets 9 and 10.

2. Adjust "n" value for changes in channel liner and flow depth. For vegetated channels, provide data for manufactured linings without vegetation and with vegetation in separate columns.

3. Slopes may not be averaged.

4. Minimum Freeboard is 0.5 ft. or 1/4 Total Channel Depth, whichever is greater

5. Permissible velocity lining design method is not acceptable for channels with a bed slope of 10% or greater. Shear stress lining design method is required for channels with a bed slope of 10% or greater. Shear stress lining design method may be used for any channel bed slope.

Schlouch Incorporated	Project: PMCC North	Warehouse
PO Box 69	Order No.:	20402110
Blandon, PA 19510	Calc. by:	MDH
Phone: 610-926-7070	Date:	12/8/2022
	-	

WORKSHEET #11

CHANNEL DESIGN DATA

Channel or Channel Section		2	2	
Temporary or Permanent	(T OR P)	Perm	Perm	
Design Storm	(2,5, 10 YR)	100-year	10-year	
Acres	(AC)	N/A	N/A	
Multiplier	(1.6, 2.25, or 2.75) ¹	N/A	N/A	
Q _r (Required Capacity)	(CFS)	2.57	2.57	
Q (Calculated at flow depth,d)	(CFS)	2.35		
Protective Lining ²		R-4	R-4	
V _a (Allowable Velocity)	(FPS)	N/A	N/A	
V (Calculated At Flow Depth, d)	(FPS)	1.38		
$ au_a$ (Max allowable shear stress)	(LB/FT ²)	2.00	2.00	
τ_d (Calculated At Flow Depth, d)	(LB/FT ²)	0.39	0.18	
Channel Bottom Width	(FT)	2.00	2.00	
Channel Side Slopes	(H:V)	2.00	2.00	
D (Total Depth)	(FT)	1.00		
Channel Top Width (ft)@ D	(FT)	6.00		
d (Calculated Flow depth in ft)	(FT)	0.55	0.25	
Channel Top Width (ft)@ d	(FT)	4.20		
Bottom Width:Depth Ratio	(12:1 Max)	3.64		
d ₅₀ Stone Size (in)	(IN)	6.00	6.00	
A (Area in sq. ft.)	(SQ. FT.)	1.71	0.51	
n (Manning's Coefficient) ²		0.0608		
R (Hydraulic Radius)		0.38		
S (Bed Slope) ³	(FT/FT)	0.01		
S _c (Critical Slope)	(FT/FT)	0.079		
.7S _c	(FT/FT)	0.055		
1.3S _c	(FT/FT)	0.102		
Stable Flow? (Y/N)	(Y/N)	Y		
Freeboard Based On Unstable Flow	(FT)	N/A		
Freeboard Based On Stable Flow	(FT)	0.50		
Minimum Required Freeboard ⁴	(FT)	0.50		
Design Method For Protective Lining ⁵		9	9	
Permissible Velocity (V) Or Shear Stress (S)		0	5	

1. Use 1.6 for Temporary Channels; 2.25 for Temporary Channels in Special 2.75 for Permanent Channels. For Rational Method, enter "N/A" and attach For TR-55 enter "N/A" and attach appropriate Worksheets.

Protection (HQ or EV) Watersheds; E&S Worksheets 9 and 10.

2. Adjust "n" value for changes in channel liner and flow depth. For vegetated channels, provide data for manufactured linings without vegetation and with vegetation in separate columns.

3. Slopes may not be averaged.

4. Minimum Freeboard is 0.5 ft. or 1/4 Total Channel Depth, whichever is greater

5. Permissible velocity lining design method is not acceptable for channels with a bed slope of 10% or greater. Shear stress lining design method is required for channels with a bed slope of 10% or greater. Shear stress lining design method may be used for any channel bed slope.

Schlouch Incorporated

PO Box 69 Blandon, PA 19510 Phone: 610-926-7070 Project:PMCC North WarehouseOrder No.:20402110Calc. by:MDHDate:1/12/2023

WORKSHEET #11

CHANNEL DESIGN DATA

Channel or Channel Section		Berm	
Temporary or Permanent	(T OR P)	Temp	
Design Storm	(2,5, 10 YR)	5-year	
Acres	(AC)	0.81	
Multiplier	(1.6, 2.25, or 2.75) ¹	2.25	
Q _r (Required Capacity)	(CFS)	1.82	
Q (Calculated at flow depth,d)	(CFS)	1.82	
Protective Lining ²		S75	
V _a (Allowable Velocity)	(FPS)	N/A	
V (Calculated At Flow Depth, d)	(FPS)	0.94	
$ au_a$ (Max allowable shear stress)	(LB/FT ²)	1.55	
τ_d (Calculated At Flow Depth, d)	(LB/FT ²)	0.26	
Channel Bottom Width	(FT)	0.00	
Channel Side Slopes	(H:V)	11.50	
D (Total Depth)	(FT)	1.00	
Channel Top Width (ft)@ D	(FT)	23.00	
d (Calculated Flow depth in ft)	(FT)	0.41	
Channel Top Width (ft)@ d	(FT)	9.45	
Bottom Width:Depth Ratio	(12:1 Max)	0.00	
d ₅₀ Stone Size (in)	(IN)	0.00	
A (Area in sq. ft.)	(SQ. FT.)	1.94	
n (Manning's Coefficient) ²		0.0550	
R (Hydraulic Radius)		0.20	
S (Bed Slope) ³	(FT/FT)	0.01	
S _c (Critical Slope)	(FT/FT)	0.075	
.7S _c	(FT/FT)	0.053	
1.3S _c	(FT/FT)	0.098	
Stable Flow? (Y/N)	(Y/N)	Y	
Freeboard Based On Unstable Flow	(FT)	N/A	
Freeboard Based On Stable Flow	(FT)	0.50	
Minimum Required Freeboard ⁴	(FT)	0.50	
Design Method For Protective Lining ⁵		<u></u>	
Permissible Velocity (V) Or Shear Stress (S)		0	

 1. Use 1.6 for Temporary Channels; 2.25 for Temporary Channels in Special
 Protection

 2.75 for Permanent Channels. For Rational Method, enter "N/A" and attach
 E&S Wor

 For TR-55 enter "N/A" and attach appropriate Worksheets.
 E&S Wor

Protection (HQ or EV) Watersheds; E&S Worksheets 9 and 10.

2. Adjust "n" value for changes in channel liner and flow depth. For vegetated channels, provide data for manufactured linings without vegetation and with vegetation in separate columns.

3. Slopes may not be averaged.

4. Minimum Freeboard is 0.5 ft. or 1/4 Total Channel Depth, whichever is greater

5. Permissible velocity lining design method is not acceptable for channels with a bed slope of 10% or greater. Shear stress lining design method is required for channels with a bed slope of 10% or greater. Shear stress lining design method may be used for any channel bed slope.



CHANNEL ANALYSIS

> > > <u>Topsoil Berm</u>

Name	Topsoil Berm
Discharge	1.82
Channel Slope	0.01
Channel Bottom Width	0
Left Side Slope	3
Right Side Slope	20
Low Flow Liner	
Retardence Class	E <2 in
Vegetation Type	None
Vegetation Density	None
Soil Type	None

S75

Phase	Reach	Discharge	Velocity	Normal Depth	Mannings N	Permissible Shear Stress	Calculated Shear Stress	Safety Factor	Remarks	Staple Pattern
S75 Unvegetated	Straight	1.82 cfs	1.19 ft/s	0.36 ft	0.04	1.6 lbs/ft2	0.23 lbs/ft2	7.03	STABLE	D
Underlying Substrate	Straight	1.82 cfs	1.19 ft/s	0.36 ft	0.04	0.37 lbs/ft2	0.11 lbs/ft2	3.3	STABLE	D

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Outlet	Pipe Dia	Slope	Tail	Mannings	Qa	Vd	Allow	Rip-Rap	3Do	La	W
Number	DO (in.)	(ft/ft)	Water	"n"	(cfs)	(fps)	Veloc.	Size	(ft)	(ft)	(ft)
			Min/Ma	X							
ES-1**	24	0.030	Min	0.0130	25.95	8.32	9.0	R-4	6.00	17.00	23.00
ES-2**	36	0.010	Min	0.0130	54.97	7.78	9.0	R-4	9.00	28.00	37.00
ES-3*	30	0.020	Min	0.0130	22.43	10.94	11.5	R-5	7.50	16.00	23.50
ES-4**	18	0.020	Min	0.0130	14.66	8.30	9.0	R-4	4.50	13.00	17.50
ES-5*	24	0.005	Min	0.0130	2.57	3.73	9.0	R-4	6.00	12.00	18.00

* Denotes 100-year design outflow was used for sizing of Rip-Rap Apron

** Use 25-year Storm Sewer Outflows (Flow and Velocity based on Hydroflow printout)



Spillways

North American Green 5401 St. Wendel-Cynthiana Rd. Poseyville, Indiana 47633 Tel. 800.772.2040 >Fax 812.867.0247 www.nagreen.com ECMDS v7.0

SPILLWAY ANALYSIS

> > > <u>Basin 1 Spillway</u>

Name	Basin 1 Spillway
Discharge	159.95
Peak Flow Period	0.5
Channel Slope	0.33
Channel Bottom Width	150
Low Flow Liner	
Retardence Class	C 6-12 in
Vegetation Type	Mix (Sod and Bunch)
Vegetation Density	Very Good 80-95%
Soil Type	Silt Loam (SM)

SC250 - Class C - Mix (Sod & Bunch) - Very Good 80-95%

Phase	Reach	Discharge	Velocity	Normal Depth	Mannings N	Permissible Shear Stress	Calculated Shear Stress	Safety Factor	Remarks	Staple Pattern
SC250 Unvegetated	Straight	159.95 cfs	7.98 ft/s	0.13 ft	0.028	3 lbs/ft2	2.74 lbs/ft2	1.09	STABLE	E
Underlying Substrate	Straight	159.95 cfs	7.98 ft/s	0.13 ft	0.028	2.2 lbs/ft2	2.74 lbs/ft2	0.8	UNSTABLE	E
SC250 Reinforced Vegetation	Straight	159.95 cfs	7.54 ft/s	0.14 ft	0.031	10 lbs/ft2	2.9 lbs/ft2	3.45	STABLE	E
Underlying Substrate	Straight	159.95 cfs	7.54 ft/s	0.14 ft	0.031	3 lbs/ft2	2.9 lbs/ft2	1.04	STABLE	E



SPILLWAY ANALYSIS

> > > <u>Basin 2 Spillway</u>

Name	Basin 2 Spillway
Discharge	40.06
Peak Flow Period	0.5
Channel Slope	0.33
Channel Bottom Width	80
Low Flow Liner	
Retardence Class	C 6-12 in
Vegetation Type	Mix (Sod and Bunch)
Vegetation Density	Very Good 80-95%
Soil Type	Silt Loam (SM)

SC250 - Class C - Mix (Sod & Bunch) - Very Good 80-95%

Phase	Reach	Discharge	Velocity	Normal Depth	Mannings N	Permissible Shear Stress	Calculated Shear Stress	Safety Factor	Remarks	Staple Pattern
SC250 Unvegetated	Straight	40.06 cfs	5.62 ft/s	0.09 ft	0.03	3 lbs/ft2	1.83 lbs/ft2	1.64	STABLE	E
Underlying Substrate	Straight	40.06 cfs	5.62 ft/s	0.09 ft	0.03	2.2 lbs/ft2	1.83 lbs/ft2	1.2	STABLE	E
SC250 Reinforced Vegetation	Straight	40.06 cfs	5.11 ft/s	0.1 ft	0.035	10 lbs/ft2	2.02 lbs/ft2	4.96	STABLE	E
Underlying Substrate	Straight	40.06 cfs	5.11 ft/s	0.1 ft	0.035	3 lbs/ft2	2.01 lbs/ft2	1.49	STABLE	E



SPILLWAY ANALYSIS

> > > <u>RG 1 Spillway</u>

Name	RG 1 Spillway
Discharge	22.99
Peak Flow Period	0.5
Channel Slope	0.33
Channel Bottom Width	75
Low Flow Liner	
Retardence Class	C 6-12 in
Vegetation Type	Mix (Sod and Bunch)
Vegetation Density	Very Good 80-95%
Soil Type	Silt Loam (SM)

SC250 - Class C - Mix (Sod & Bunch) - Very Good 80-95%

Phase	Reach	Discharge	Velocity	Normal Depth	Mannings N	Permissible Shear Stress	Calculated Shear Stress	Safety Factor	Remarks	Staple Pattern
SC250 Unvegetated	Straight	22.99 cfs	4.48 ft/s	0.07 ft	0.032	3 lbs/ft2	1.41 lbs/ft2	2.13	STABLE	E
Underlying Substrate	Straight	22.99 cfs	4.48 ft/s	0.07 ft	0.032	2.2 lbs/ft2	1.4 lbs/ft2	1.57	STABLE	E
SC250 Reinforced Vegetation	Straight	22.99 cfs	3.97 ft/s	0.08 ft	0.039	10 lbs/ft2	1.59 lbs/ft2	6.29	STABLE	E
Underlying Substrate	Straight	22.99 cfs	3.97 ft/s	0.08 ft	0.039	3 lbs/ft2	1.59 lbs/ft2	1.89	STABLE	E



SPILLWAY ANALYSIS

> > > <u>Rain Garden - Downstream</u>

Name	Rain Garden - Downstream
Discharge	22.99
Peak Flow Period	0.5
Channel Slope	0.039
Channel Bottom Width	75
Low Flow Liner	
Retardence Class	C 6-12 in
Vegetation Type	Bunch Type
Vegetation Density	Fair 50-64%
Soil Type	Sandy Loam (GM)

Unreinforced Vegetation - Class C - Bunch Type - Fair 50-64%

Phase	Reach	Discharge	Velocity	Normal Depth	Mannings N	Permissible Shear Stress	Calculated Shear Stress	Safety Factor	Remarks	Staple Pattern
Unreinforced Vegetation	Straight	22.99 cfs	1.56 ft/s	0.2 ft	0.063	4 lbs/ft2	0.48 lbs/ft2	8.39	STABLE	
Underlying Substrate	Straight	22.99 cfs	1.56 ft/s	0.2 ft	0.063	7.48 lbs/ft2	0.47 lbs/ft2	15.77	STABLE	

C - 12

an Green

North American Green 5401 St. Wendel-Cynthiana Rd. Poseyville, Indiana 47633 Tel. 800.772.2040 >Fax 812.867.0247 www.nagreen.com ECMDS v7.0

SLOPE ANALYSIS

> > > <u><</u>	<u>Slope</u>
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Country	United States
State/Region	Pennsylvania
City	Scranton
Annual R Factor	100.00
Adjusted R Factor	34.00
Total Slope Length	50
Protection Type	Permanent
Protection Period	6
Beginning Month	January
Slope Gradient (H:1)	3
Soil Type	Sandy Loam
K Factor	0.19

Reach 1 Start: Oft End: 50 ft Vegetation Type: 80-95%

Material	ASL bare	ASL mat	MSL bare	MSL mat	Soil Loss Tolerance	SF	Remarks	Staple / App Rate
SC150BN	0.1 in	0.0 in	0.2 in	0.0 in	0.25 in	>10	STABLE	D
Estb. Veg.	0.4 in	0.0 in	N/A in	N/A in	0.03 in	1.895	STABLE	

