

March 30, 2021

Mr. Judd Dayton, P.E. Snyder, Secary & Associates, LLC 2000 Linglestown Road, Suite 304 Harrisburg, PA 17110

#### RE: Stormwater Infiltration Summary Letter 7600 Linglestown Road West Hanover Township, Dauphin County, Pennsylvania Kleinfelder Project No.: 01900271.003A

Dear Mr. Dayton,

In accordance with your request, Kleinfelder, Inc. (Kleinfelder), has completed a Stormwater Infiltration Summary Report for the above referenced project site to evaluate the suitability of the subsurface soils for the infiltration of stormwater. This correspondence serves to transmit the results of our evaluation.

# SITE AND PROJECT DESCRIPTION

The project site currently consists of an agricultural and densely wooded parcel of land located at 7600 Linglestown Road in West Hanover Township, Dauphin County, Pennsylvania. The project site is bordered to the north by agricultural fields, to the east and west by wooded areas and to the south by Linglestown Road. Topography across the project site consists of gentle to steep rolling terrain sloping generally down-gradient towards the southeast with approximately 150 feet of grade variation across the project site. The approximate location of the site in relation to the surrounding area is depicted on the *Topographic Map* (Figure 1) presented within the Appendix.

According to the *Infiltration Test Exhibit* (Plan), prepared by Snyder-Secary & Associates, LLC, the project will consist of constructing a new warehouse/distribution facility. The structure is anticipated to measure 1,117,200 square feet in plan area, comprised of conventional steel-frame construction with exterior, tilt-up cast-in-place (or pre-fabricated) concrete walls with the ground floor slab supported on grade. Development of this project will also include constructing parking areas, truck aprons, drive lanes, subsurface utilities and stormwater management facilities. Based on existing and proposed grades, maximum cuts of approximately 19 feet are anticipated to be required to reach the proposed stormwater invert elevations.

# SCOPE OF WORK

The objective of our work was to determine the permeability of the invert soils, identify any limiting zones (i.e. bedrock, groundwater, or seasonal high-water table) and address PADEP requirements as they relate to stormwater management. This objective was accomplished through a scope of work which included a subsurface exploration, and preparation of this report. This report presents a summary of the work completed, conditions encountered and results of our engineering analysis of subsurface conditions.

# **GEOLOGY**

According to the Pennsylvania Geologic Survey's <u>Atlas of Preliminary Geologic Quadrangles</u>, Fourth Series, 1981, the project site is underlain by the Hamburg Sequence and the Limestone of Hamburg Sequence (geologic symbols Oh and Ohl, respectively). The property within its geologic settings is presented on the Geologic Map (Figure 2) found within the Appendix.

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The *Engineering Characteristics of the Rocks of Pennsylvania*, second edition, 1982, published by the Pennsylvania State Geologic Survey, describes the rock in these formations as transported rocks of the Hamburg overthrust; gray, greenish-gray and maroon shale, silty and siliceous in many places, dark-gray impure sandstone; medium to light gray, finely crystalline limestone and shaley limestone.

The shale in the formations is moderately well bedded and thin, while the sandstone is well bedded and thick. The limestone is also well bedded, but flaggy. Shale fractures form a seamy to platy pattern, are well developed, highly abundant; variably spaced, open and steeply dipping. Sandstone fractures form a blocky pattern, are well developed, moderately abundant; evenly spaced, open and steeply dipping. Limestone fractures form a platy pattern, are poorly developed, moderately abundant; open and steeply dipping.

The shale in the formations is moderately resistant to weathering and is moderately to highly weathered to a deep depth, resulting in loose rubble of pencil-like fragments to rectangular plates. The sandstone is moderately resistant to weathering and is moderately weathered to a shallow depth, resulting in medium to large, irregular.

The Limestone of Hamburg Sequence is comprised of carbonate lithology which is subject to dissolution and the development of sinkholes and other karst-related features. The *Sinkhole Map of Pennsylvania*, prepared by William Kochonov of the Pennsylvania Geologic Survey, does not show any mapped karst features within or surrounding the site. No karst features (i.e. sinkholes, closed depressions and/or bedrock outcrops) were observed at the time of the fieldwork.

# SUBSURFACE EXPLORATION PROGRAM

To characterize the subsurface conditions across the footprints of the proposed stormwater management facilities, 22 test pits were excavated on March 11 through 15, 2021. Supervision and monitoring of the subsurface exploration were provided by a representative of Kleinfelder who field located the test locations based on the previously referenced Plan. The approximate test pit locations are shown on the *Exploration Plan* (Figure 3) presented within the Appendix.

The test pits were excavated utilizing a John Deere 310G tracked excavator. A detailed account of the material encountered during the excavation of each test pit as well as the infiltration test depths and infiltration rates (where applicable) are presented on the *Test Pit Logs* within the Appendix.

# LABORATORY TESTING

Soil samples retrieved from the site were visually reviewed and classified by Kleinfelder. Representative soil samples were subjected to laboratory analyses to verify visual classifications and aid in establishing preliminary engineering parameters for foundation design analysis in accordance with the following schedule:

- Natural Moisture Content (ASTM D2216)
- Sieve Analysis (ASTM D422)
- Atterberg Limits Determination (ASTM D4318)

Unified Soil Classification System (USCS) Group Symbols and ASTM Group Names have been assigned to the soils analyzed. The results of the laboratory analyses are presented within the table below and graphical depictions of the particle size analyses are presented in the Appendix.

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	STANDARD CLASSIFICATION RESULTS													
Location	Depth (feet)	Soil Type	% Gravel	% Sand	% Fines	LL	PL	Ы	Natural Moisture Content	USCS Group Symbol	ASTM Group Name			
IT-2	7 – 10	Stratum	50.6	44.5	4.8	37	34	3	18.6%	GW	Well-graded GRAVEL with Sand			
IT-18	1 – 3	Stratum I	66.4	28.4	5.3	36	26	10	9.4%	GW	Well-graded GRAVEL with Silt and Sand			
IT-22	1.5 – 2	Stratum II	12.4	39.6	48.0	31	25	6	22.0%	SM	Silty SAND			
LL-Liquid L	LL-Liquid Limit; PL-Plastic Limit; PI-Plasticity Index													

# SUBSURFACE CONDITIONS

SOIL

### **Surficial Materials**

The test pits were covered by approximately 6 to 24 inches of topsoil, with exception of test pits IT-14 through IT-16, IT-18 and IT-19, where no topsoil was present. Topsoil thicknesses may vary in unexplored areas of the project site.

# Stratum I – Reddish brown to brown to yellow to tan to gray Well-Graded GRAVEL with varying amounts of Silt and Sand

Stratum II was encountered within each test pit completed, with the exception of test pits IT-12, IT-13, IT-20 and IT-21, and extended to their termination depths ranging from approximately 1 to 12 feet below existing site grades. Laboratory testing conducted on representative samples of Stratum II show this soil to be poorly graded overall and plastic, with natural moisture contents ranging of 9.4% and 18.6%. Stratum II is described under the USCS as Well-graded GRAVEL with Sand (GW) and Well-graded GRAVEL with Silt and Sand (GW).

### Stratum II – Reddish brown to brown Silty SAND

Stratum I was encountered within test pits IT-1, IT-12, IT-13, IT-20, IT-21 and IT-22, and extended to depths ranging from approximately 4 to 13 feet below existing site grades. Laboratory testing conducted on a representative sample of Stratum I shows this soil to be poorly graded and plastic, with a natural moisture content of 22.0%. Stratum I is described under the USCS as Silty SAND (SM).

### BEDROCK

The bedrock surface was encountered within 9 of the 22 test pits completed, at depths ranging from approximately 1 to 10 feet below existing site grades. The bedrock surface was defined as the depth at which the bucket of the excavator could no longer advance. Different equipment may yield different results.

Based on published geologic data, the bedrock surface beneath the project site is pinnacled with significant variation in the elevation of the bedrock surface over short lateral distances. The erratic surface profile of the underlying bedrock is typical of carbonate geologic formations, such as the one which underlies the site. Therefore, the potential exists for the bedrock surface to be encountered at elevations which vary significantly from the elevations encountered during our exploration. The Contractor(s) may utilize this data to understand the bedrock surface, however, should use caution when interpolating bedrock elevation information between test locations due to the underlying geologic formation.

## **GROUNDWATER/SOIL MOTTLING**

Perched groundwater was encountered within test pits IT-9 through IT-13 at depths ranging from approximately 2 to 3 feet below existing grades. Groundwater was encountered within test pits IT-11, IT-12, IT-13, IT-20, IT-21, and IT-22 at depths ranging from 5.5 to 12.5 feet below existing site grades. Soil mottling (indicating a seasonal high water table and/or poorly draining soils) was not observed within the test pits completed. These observations were made at the time of the field operation and the groundwater table elevation will vary with daily, seasonal, climatological variations and anthropogenic activities.

# **CONSIDERATION OF KARST GEOLOGY**

The following construction considerations are provided to minimize the potential for development of sinkholes at the site both during and following construction.

- Surface water should not be allowed to collect or pool in low lying areas of the site and should be directed to appropriate stormwater channels. Expeditious backfilling or grading of low-lying areas will also help minimize the potential for the development of sinkholes.
- The bases of all foundation excavations should be reviewed for unusually soft or wet soil conditions. Any
  unstable areas encountered should be further excavated and reviewed by the geotechnical engineer to
  determine the extent of any solution activity so that remedial measures can be designed and
  implemented.
- The extent of excavations should be kept to a minimum and the influx of surface water into excavations should be minimized.
- Positive drainage away from the proposed structure should always be maintained. Roof drains should also be directed away from the structure and into designated storm sewer connections.
- Storm sewer conveyance lines should be constructed with watertight joints
- Unpaved areas, swales, and/or surface/subsurface stormwater management facilities should be avoided adjacent to building/foundation areas.
- Exterior backfill around foundations and utilities should consist of fine-grained, low permeable soils (i.e. silt and clay) in an effort to limit concentrated stormwater infiltration.

The above recommendations constitute best management practices for construction and development in areas underlain by karst geologic formations. The site Owner must recognize the risks associated with development in areas underlain by karst geologic formations. Contingencies should be made in the construction schedule and budget for the repair of sinkholes and unstable soil conditions encountered during development of the site.

# **INFILTRATION TESTING**

To evaluate the infiltration of stormwater, infiltration testing was anticipated to be completed within each test pit excavated. Where infiltration testing was completed, it was completed in accordance with the <u>Pennsylvania</u> <u>Stormwater Best Management Practices Manual</u>, latest Edition. Each test pit was anticipated to extend a minimum of 2 feet below the proposed invert elevation to review for the presence of limiting zones (i.e. bedrock, groundwater and/or soil mottling). Infiltration testing was not completed within 7 of the test pits due to limiting zones encountered. The results of the infiltration testing are presented in the table below.

INFILTRATION TEST RESULTS												
Test Location	Existing Elevation (feet)	Proposed Infiltration Test Elevation (feet)	Actual Infiltration Test Elevation (feet)	Limiting Zone Elevation (feet)	Infiltration Rate (inches/hour)*							
IT-1	565	562	565	Not Encountered at 556	21.0							
IT-2	578	568	568	Not Encountered at 566	0.2							
IT-3	578	568	570	Bedrock at 568	9.0							
IT-4	580	568	573	Not Encountered at 571	4.5							
IT-5	580	570	570	Not Encountered at 568	12.0							
IT-6	580	570	570	Not Encountered at 568	12.0							
IT-7	580	570	570	Not Encountered at 568	12.0							
IT-8	580	570	570	Not Encountered at 568	12.0							
IT-9	542	532	No Test	Perched Water at 539 Groundwater at 535.5 Bedrock at 532	No Test							
IT-10	542	532	538	Perched Water at 539 Groundwater at 536	0.4							
IT-11	538	532	No Test	Perched Water at 536 Groundwater at 531	No Test							
IT-12	537	532	No Test	Perched Water at 534.5 Groundwater at 530.5	No Test							
IT-13	536	532	No Test	Perched water at 533.5 Groundwater at 530	No Test							
IT-14	558	558	No Test	Bedrock at 555	No Test							
IT-15	559	558	No Test	Bedrock at 557.5	No Test							
IT-16	560	558	No Test	Bedrock at 558	No Test							
IT-17	557	555	556	Bedrock at 554	3.6							
IT-18	557	555	555	Not Encountered at 553	7.2							
IT-19	553	555	552	Bedrock at 549.5	6.0							
IT-20	540	535	535	Groundwater at 533	7.2							
IT-21	545	535	535	Groundwater at 532.5	16.8							
IT-22	563	562	561.8	Groundwater at 557.5	12.0							

\*Infiltration rates are field rates and not factored -Shaded cells indicate where testing was completed above proposed invert elevation due to limiting zones -Bold text indicates infiltration testing completed immediately below topsoil layer

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# **SUMMARY OF DATA & CONCLUSIONS**

Based on the results of our field exploration and data obtained, we offer the following comments regarding the infiltration of stormwater at the project site.

- Infiltration testing was conducted within the naturally occurring soils of Stratum I and Stratum II.
- The bedrock surface was encountered within 9 of the 22 test pits completed, at depths ranging from approximately 1 to 10 feet below existing site grades.
- Perched groundwater was encountered within test pits IT-9 through IT-13 at depths ranging from approximately 2 to 3 feet below existing grades. Groundwater was encountered within test pits IT-11, IT-12, IT-13, IT-20, IT-21, and IT-22 at depths ranging from 5.5 to 12.5 feet below existing site grades.
- Soil mottling was not observed within the test pits completed.
- The unfactored field infiltration rates achieved ranged from 0.2 to 21.0 inches per hour. The PADEP recommended rate for infiltration of stormwater is 0.1 to 10 inches per hour.

# LIMITATIONS

This work was performed in a manner consistent with that level of care and skill ordinarily exercised by other members of Kleinfelder's profession practicing in the same locality, under similar conditions and at the date the services are provided. Our conclusions are based on a limited number of observations and data. It is possible that conditions could vary between or beyond the data evaluated. Further, Kleinfelder assumes no liability for interpolation of data between the specific testing locations discussed herein. Kleinfelder makes no other representation, guarantee, or warranty, express or implied, regarding the services, communication (oral or written), report, opinion, or instrument of service provided.

The varied nature of carbonate geology precludes absolute certainty in assessing sinkhole formation. Therefore, the Owner should be aware that conditions could be encountered during construction that would require modifications to our recommendations. Kleinfelder makes no warranty or guarantee with regard to the development of sinkholes on the project site.

This report may be used only by the Client and the registered design professional in responsible charge and only for the purposes stated for this specific engagement within a reasonable time from its issuance, but in no event later than 2 years from the date of the report.

Our scope of services for this exploration and report did not include environmental assessments or evaluations regarding the presence or absence of wetlands or hazardous substances in the soil, surface water, or groundwater at this site.

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# **CLOSING**

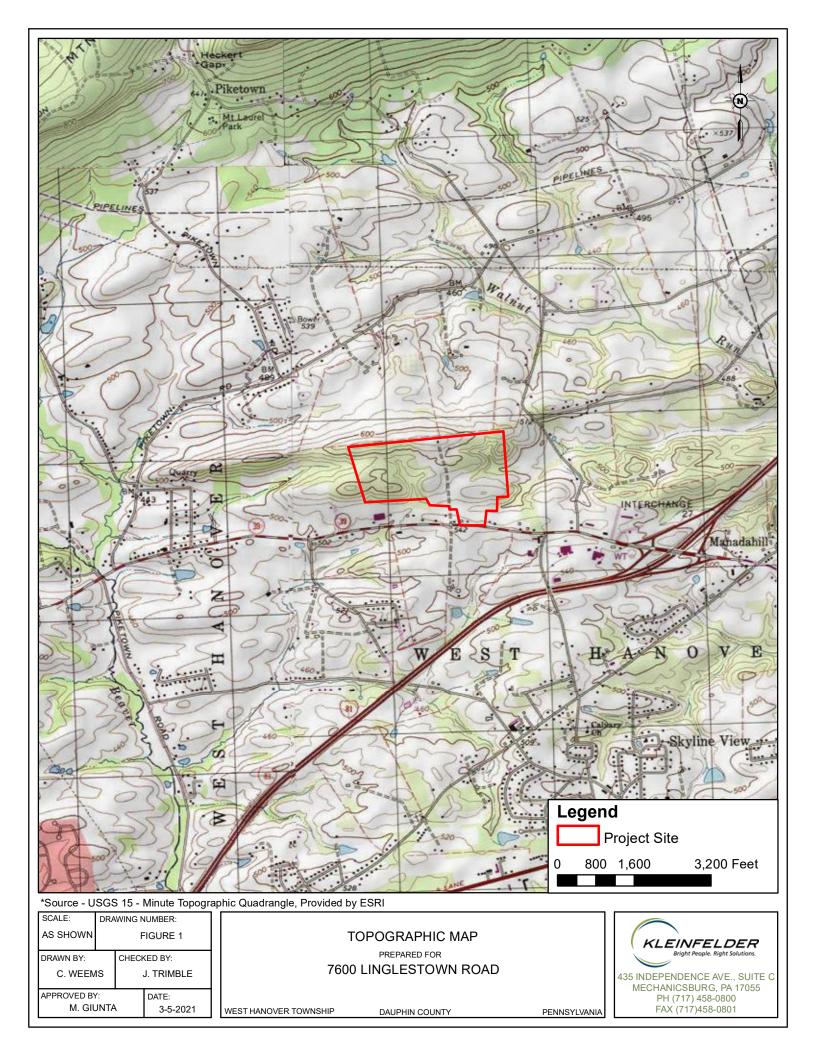
We thank you for the opportunity to work on this project with you. Should you have any questions or require any additional information, please do not hesitate to contact us.

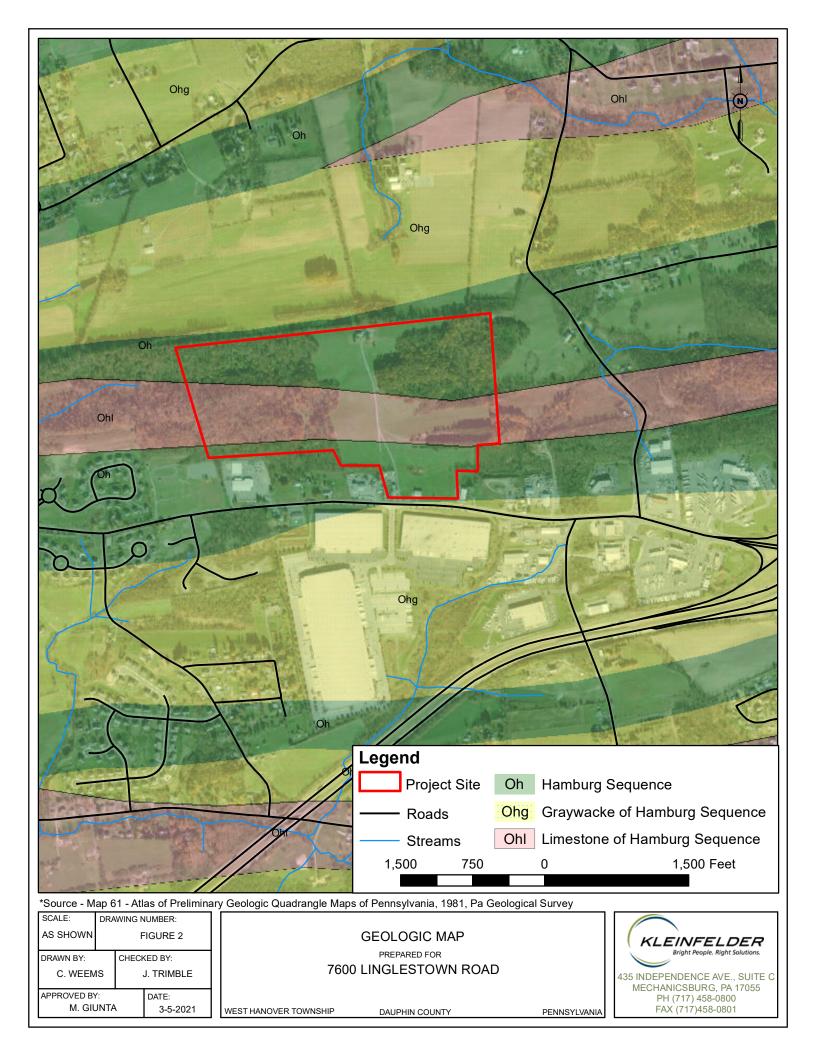
Respectfully Submitted, **KLEINFELDER, INC.** 

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Jason E. Trimble Project Manager

Trevor L. Dombach Program Manager

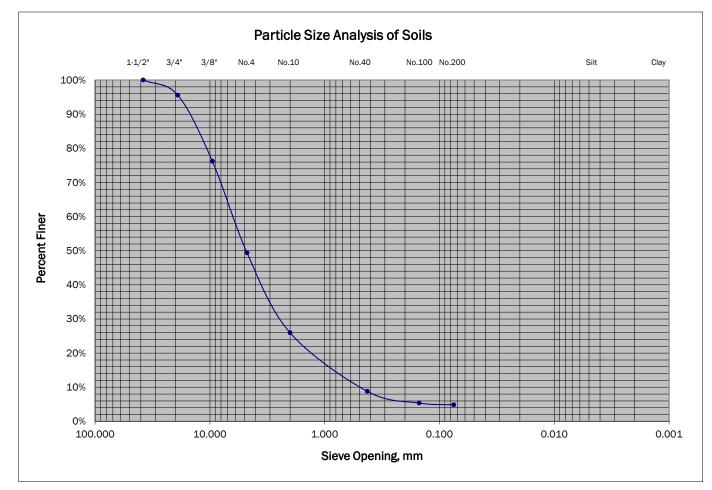








Per ASTM Designations D 2487 and D 2488



As-Received Moisture 18.6%		Particle Size Distribution									
USCS Classification: Well-grade	ed GRAVEL with Sand (GW)	US Standard	Sieve Size	Opening (mm)	%Finer						
Gravel: 50.6% Coarse:	4.4% <b>Fine:</b> 46.2%	Coarse	1-1/2"	38.0	100.0%						
Sand: 44.5% Coarse: 2	3.4% Medium: 17.2% Fine: 4.0%	GRAVEL	3/4"	19.0	95.6%						
Fines: 4.8% Silt:	Clay:	Fine	3/8"	9.50	76.2%						
Gravel Description: Subangula	ar		No. 4	4.75	49.4%						
		Coarse	No. 10	2.00	26.0%						
Sand Description: Subangula	ar	Medium	No. 40	0.425	8.8%						
		SAND	No. 100 0.150		5.4%						
Consistency: N/A	Dry Strength: N/A	Fine	No. 200	0.075	4.8%						
Dilatancy: N/A	Toughness: N/A	Hydrometer	Silt Size	0.005							
Structure: Blocky	Cementation: Weak	Analysis	Clay Size	0.001							
		D <sub>60</sub> : 6.5	D <sub>30</sub> : 2.6	D <sub>10</sub> : 0.5	Cu: 13 Cc: 2.08						
Test Pit: IT-2		Atterberg Limits	<b>LL:</b> 37	<b>PL:</b> 34	<b>PI:</b> 3						
Sample: S-1 D	epth: 7' - 10'	Description:	Yellow to reddish brown Well-graded GRAVEL with								
Project: 7600 Linglestown Roa	d Warehouse	Ι	Sand								
		Remarks:	Stratum I								
Client: Snyder, Secary & Asso	ciates, LLC										
Kleinfelder Project Number:	01900271.003A	Report Date:	March 19, 202	1							



Per ASTM Designations D 2487 and D 2488

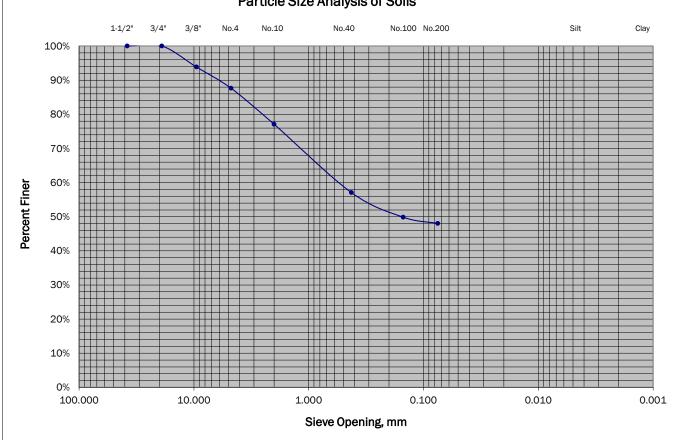
Particle Size Analysis of Soils No.10 No.40 No.100 No.200 1-1/2" 3/4" 3/8" No.4 Silt Clay 100% 90% 80% 70% 60% Percent Finer 50% 40% 30% 20% 10% 0% 100.000 10.000 1.000 0.100 0.010 0.001 Sieve Opening, mm

As-Receiv	ved Moisture	9.4%			Particle Size Distribution								
USCS Classification: Well-graded GRAVEL with Silt and Sand (GW-GM)							US Standard	Sieve Size	Opening (mm)	%	Finer		
Gravel:	66.4%	Coarse: 22.2%			Fine:	44.2%	Coarse	1-1/2"	38.0	9	1.2%		
Sand:	28.4%	Coarse: 14.1%	Medium:	10.7%	Fine:	3.5%	GRAVEL	3/4"	19.0	7	7.8%		
Fines:	5.3%	Silt:		Clay:			Fine	3/8"	9.50	5	3.6%		
Gravel De	escription:	Subangular						No. 4	4.75	3	3.6%		
							Coarse	No. 10	2.00	1	9.5%		
Sand Description: Subangular							Medium	No. 40	0.425	8	8.7%		
						SAND	No. 100	0.150	6.0%				
Consister	ncy: N/A		Dry Strengt	th:	N/A		Fine	No. 200	0.075	į	5.3%		
Dilatancy	/: N/A		Toughness	•	N/A		Hydrometer	Silt Size	0.005				
Structure	: Blocky		Cementatio	on:	Weak		Analysis	Clay Size	0.001				
							D <sub>60</sub> : 12	D <sub>30</sub> : 3	D <sub>10</sub> : 0.58	Cu: 21	Cc: 1.29		
Test Pit:	IT-18						Atterberg Limits	LL: 36	<b>PL:</b> 26	P	기: 10		
Sample:	S-1	Depth:	1' - 3'				Description:		Well-graded GR	AVEL with	Silt and		
Project: 7600 Linglestown Road Warehouse				Sand									
							Remarks:	Stratum I					
Client:	Snyder, Seo	cary & Associates	s, LLC										
Kleinfeld	er Project Nu	umber:	01900271	.003A			Report Date:	March 19, 202	1				



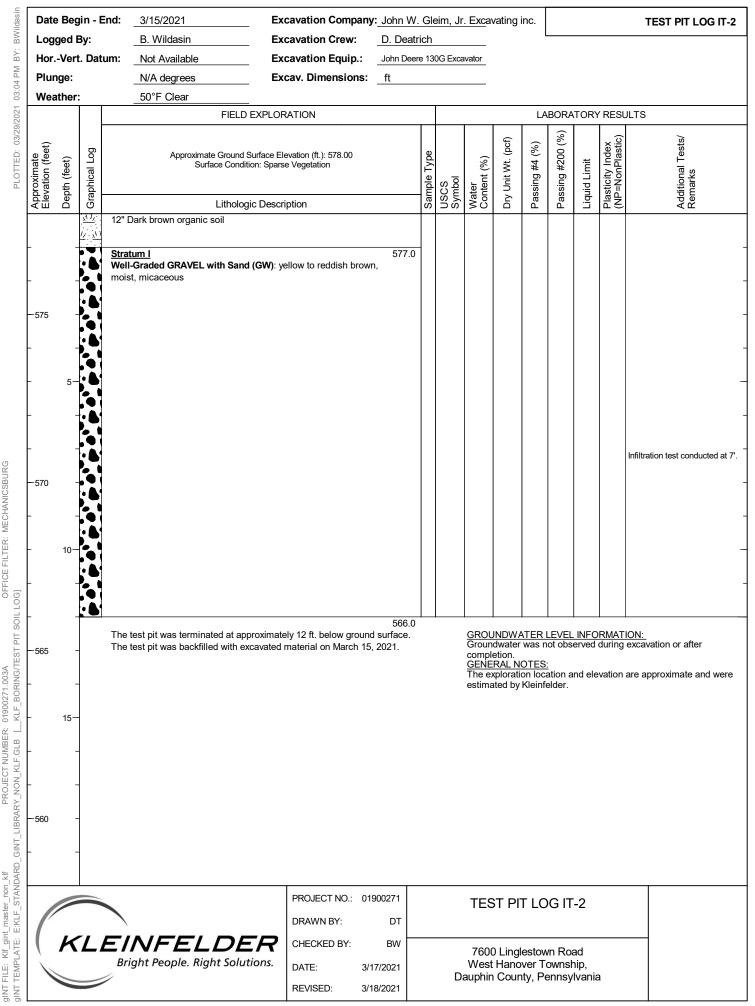
Per ASTM Designations D 2487 and D 2488

Particle Size Analysis of Soils

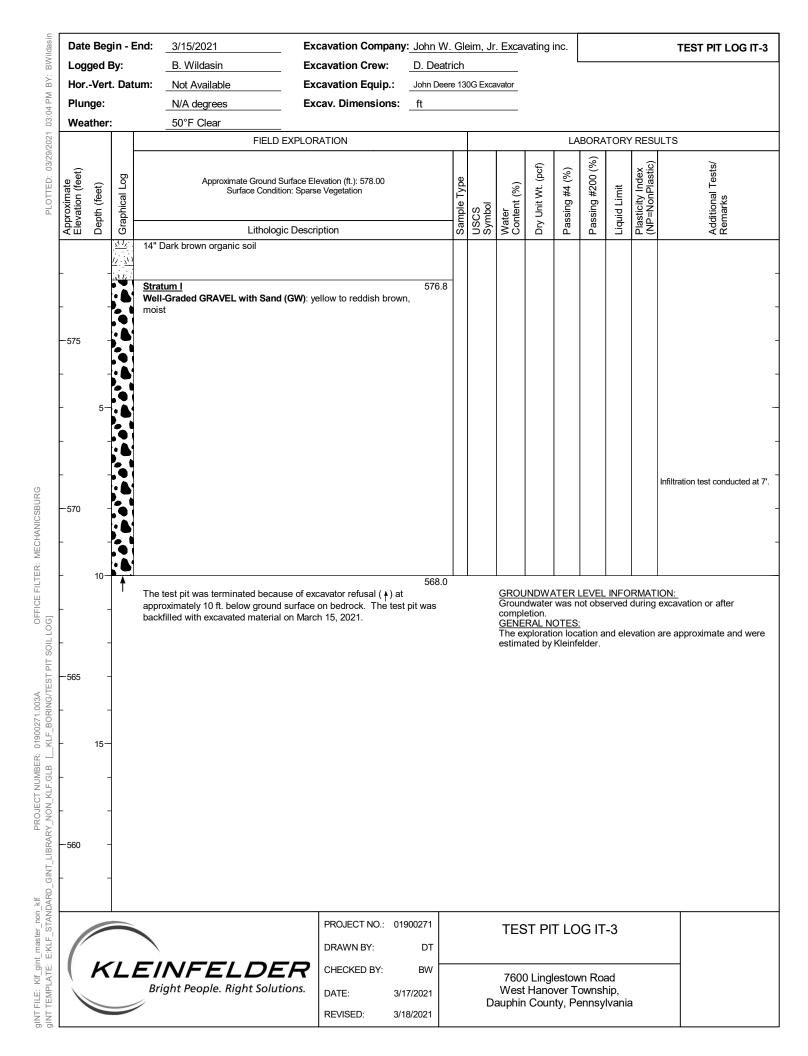


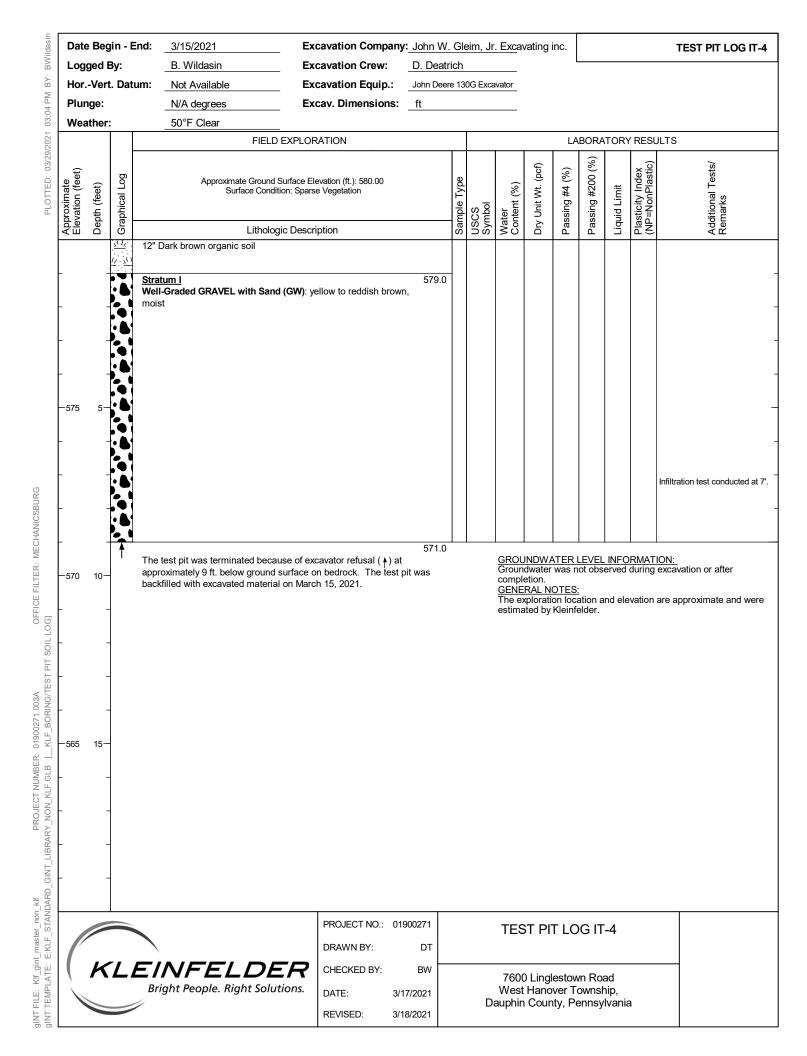
As-Received Moisture 22.0%		Particle Size Distribution									
USCS Classification: Silty SAND (SM	1)	US Standard	Opening (mm)	%Finer							
Gravel: 12.4% Coarse: 0.09	6	Fine:	12.4%	Coarse	1-1/2"	38.0	100.0%				
Sand: 39.6% Coarse: 10.6%	6 <b>Medium:</b> 20.0%	Fine:	9.1%	GRAVEL	3/4"	19.0	100.0%				
Fines: 48.0% Silt:	Clay:			Fine	3/8"	9.50	93.8%				
Gravel Description: Subrounded					No. 4	4.75	87.6%				
				Coarse	No. 10	2.00	77.1%				
Sand Description: Subrounded				Medium	No. 40 0.425		57.1%				
					No. 100	0.150	49.9%				
Consistency: N/A	Dry Strength:	N/A		Fine	No. 200	0.075	48.0%				
Dilatancy: N/A	Toughness:	N/A		Hydrometer	Silt Size	0.005					
Structure: Homogeneous	Cementation:	Weak		Analysis	Clay Size	0.001					
				D <sub>60</sub> :	D <sub>30</sub> :	D <sub>10</sub> :	Cu: Cc:				
Test Pit: IT-22				Atterberg Limits	LL: 31	<b>PL:</b> 25	<b>PI:</b> 6				
Sample: S-1 Deptr	<b>1</b> .5' - 2'			Description:	Reddish brown	Silty SAND					
Project: 7600 Linglestown Road											
				Remarks:	Stratum II						
Client: Snyder, Secary & Associate	es, LLC										
Kleinfelder Project Number:	01900271.003A			Report Date:	March 19, 2021						

Date	e Beç	gin - E	End:	3/15/2021	Excavation Company	Excavation Company: John W. Gleim, Jr. Excavating inc							TEST PIT LOG IT-1				
Log	ged I	By:		B. Wildasin	Excavation Crew:	Excavation Equip.: John Deere 130G Excavator											
Hor.	-Ver	t. Dat	um:	Not Available	Excavation Equip.:												
Plur	nge:			N/A degrees	Excav. Dimensions:												
Wea	ather	:		50°F Clear													
				FIELD E	XPLORATION						LA	BORA	TOR	/ RESI	JLTS		
Approximate Elevation (feet)	Approximate Ground Surface Ele Surface Condition: Sparse				Approximate Ground Surface Elevation (ft.): 565.00 Surface Condition: Sparse Vegetation				Water Content (%)	Dry Unit Wt. (pcf)	Passing #4 (%)	Passing #200 (%)	Liquid Limit	Plasticity Index (NP=NonPlastic)	Additional Tests/ Remarks		
Appr Elev	Dep	Grap		Lithologic	Description		Sample Type	USCS Symbol	Wate	Dry I	Pas	Pas	Liqui	Plas (NP:	Addi		
		<u>x, 1/</u>	24" [	Dark brown organic soil	•					_	-	-					
	- - - 5 -		Silty Strat	tum II SAND (SM): reddish brown, m tum I rey GRAVEL with Sand (GW): fragments, highly weathered re	tan to gray, moist, with Cobbl	563.0 559.5 e-sized									Infiltration test conducted at a		
- 555 - -	- 10- - -			test pit was terminated at appr test pit was backfilled with exc					compl GENE	dwater etion. <u>RAL N</u> kplorati	was r <u>OTES</u> on loc	iot obs <u>:</u> ation a	erved	during	I <u>ON:</u> excavation or after are approximate and were		
-550		-															
(					PROJECT NO.: 019 DRAWN BY:	900271 DT			TES	ST PI	T LC	)G IT	-1				
	K			<b>NFELDE</b> ight People. Right Solution	DATE: 3/1	BW 7/2021 8/2021	_	Da		) Ling Hanc Cour	ver T	ownsł	nip,	a			



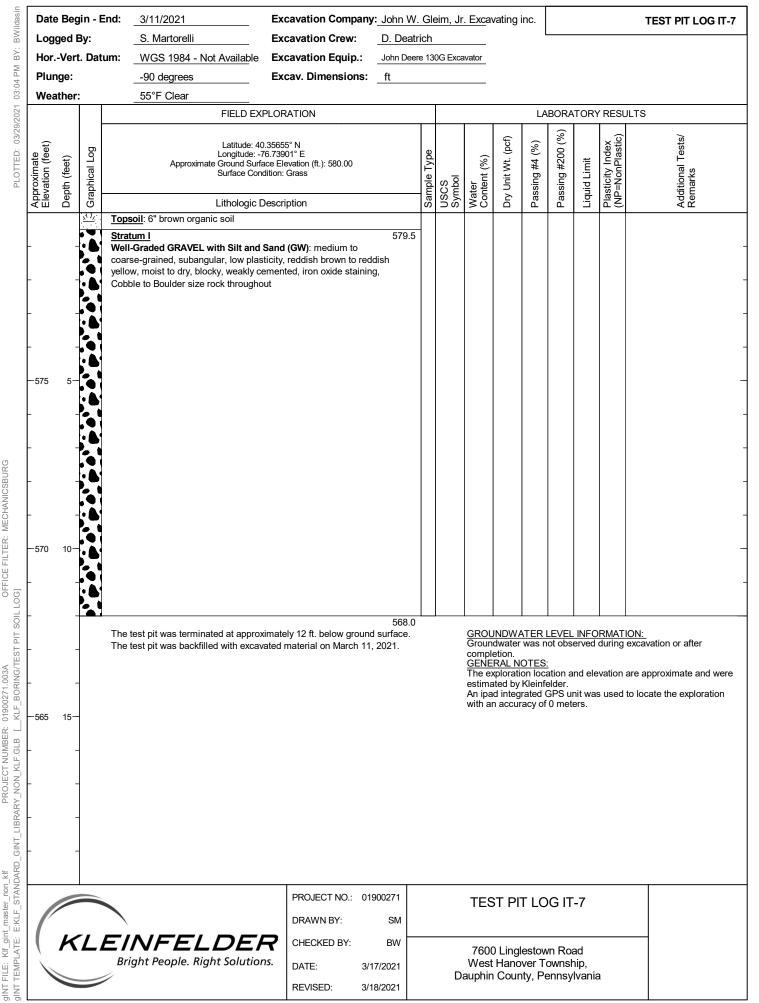
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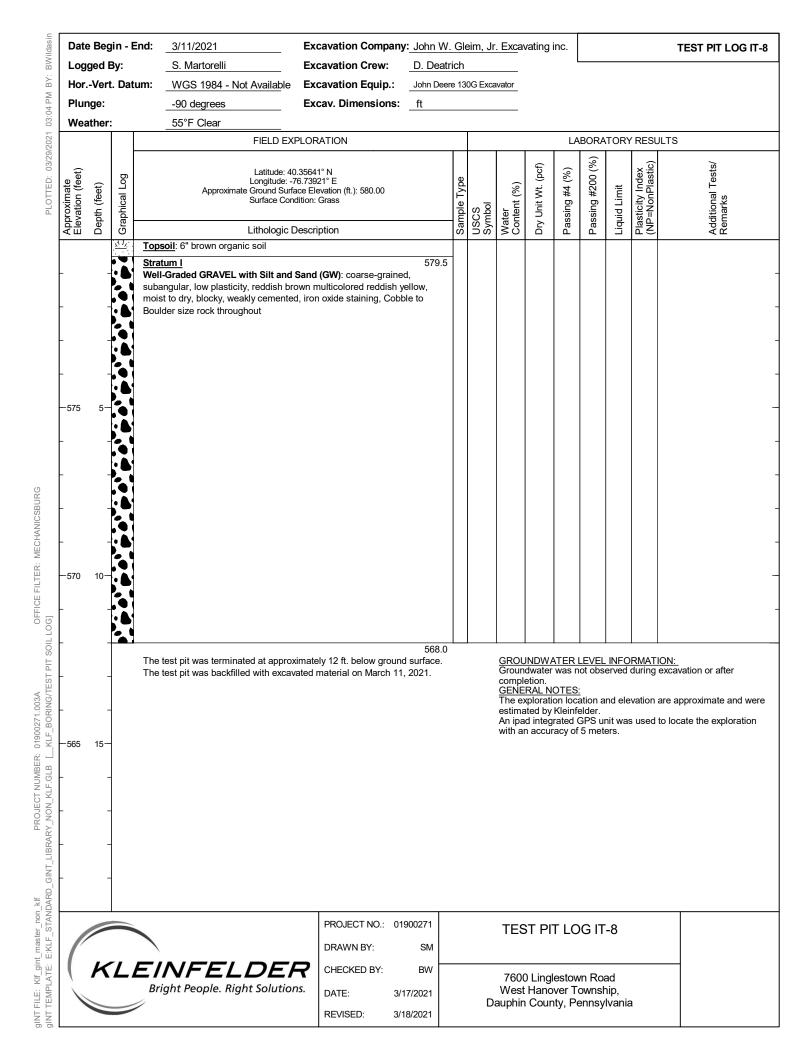


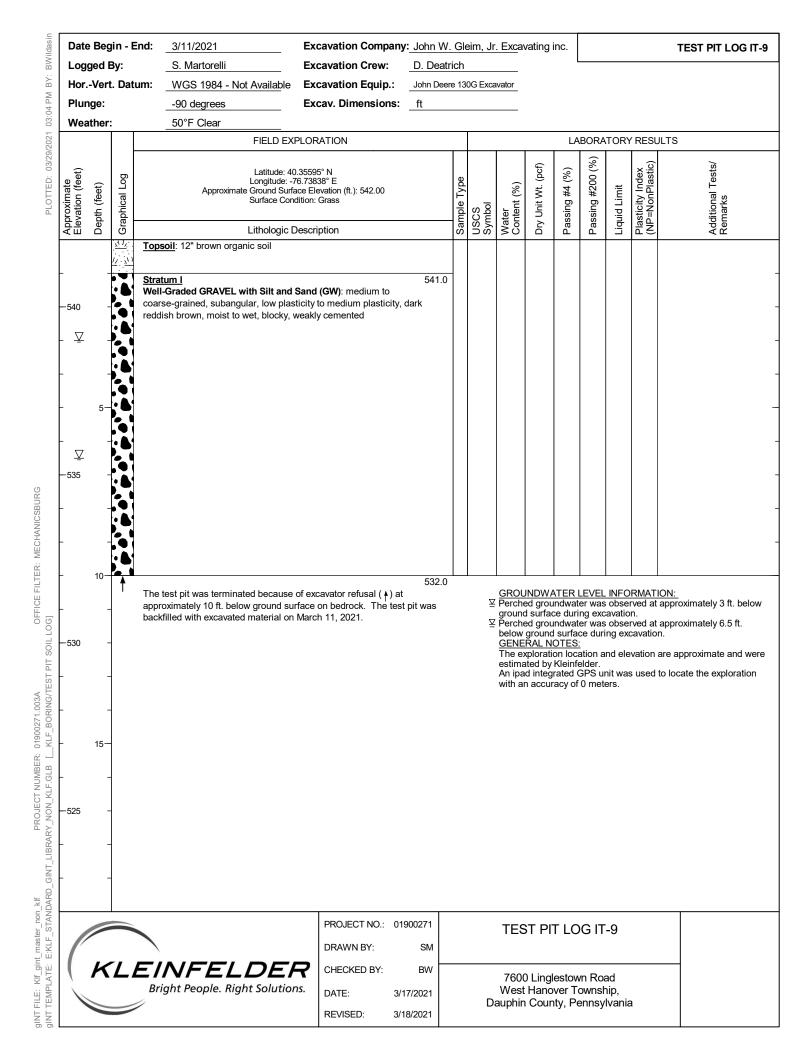


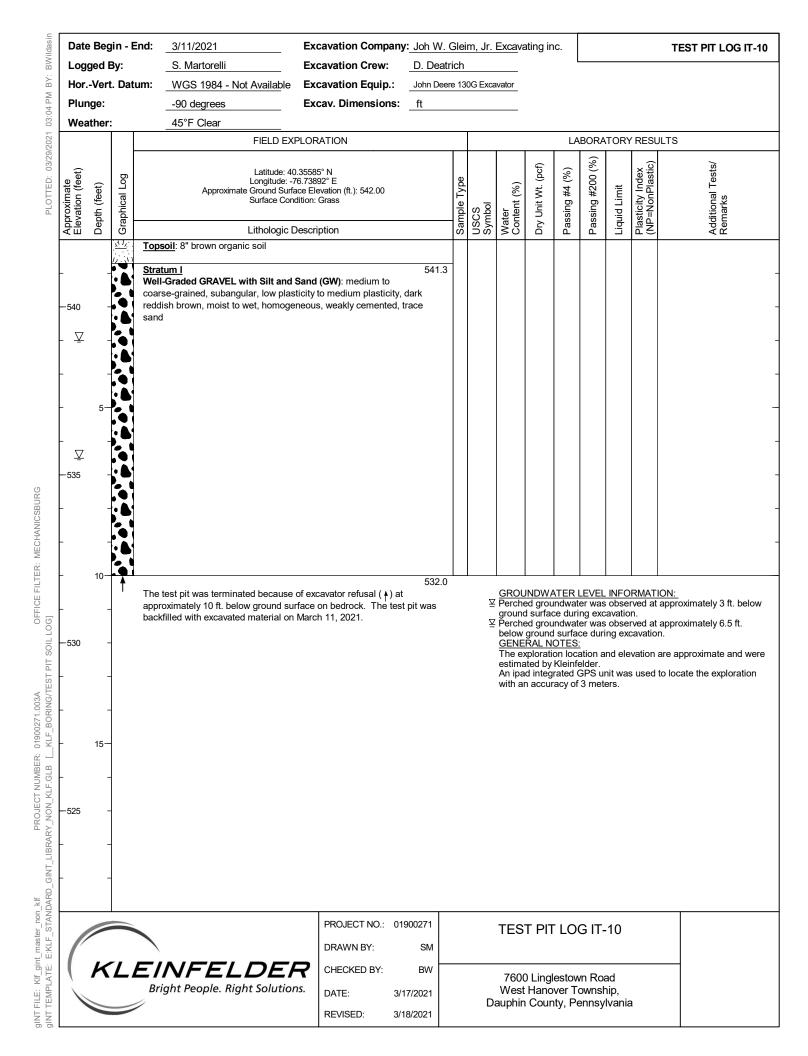


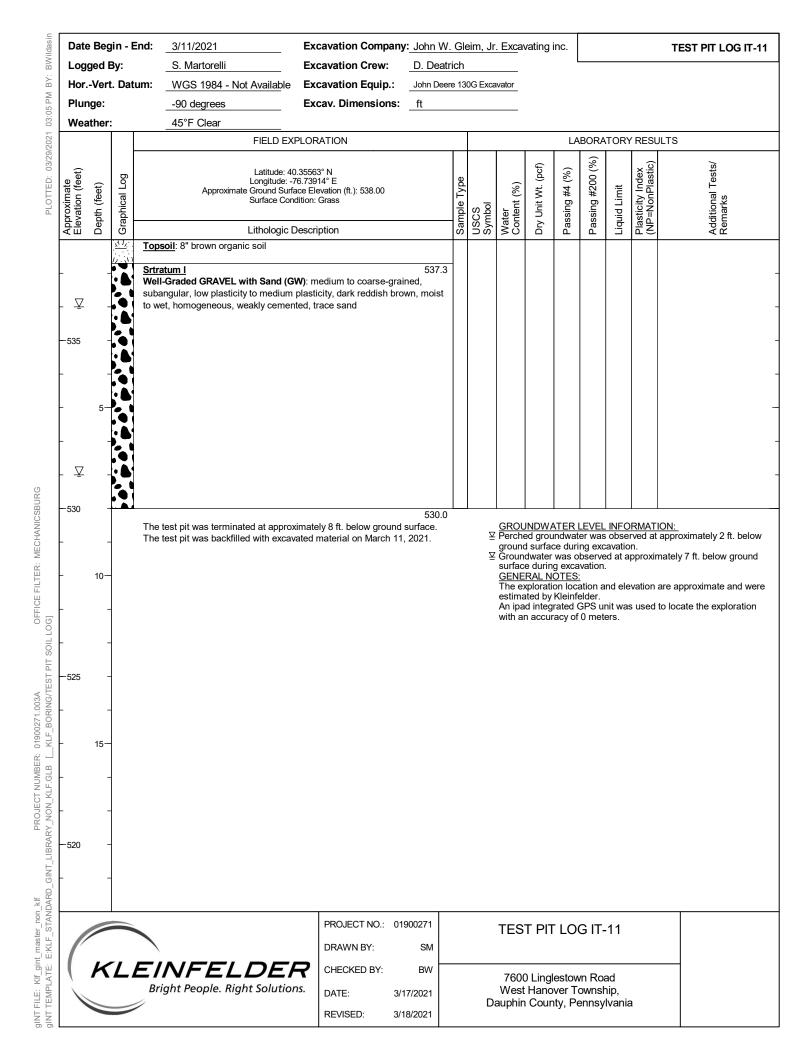


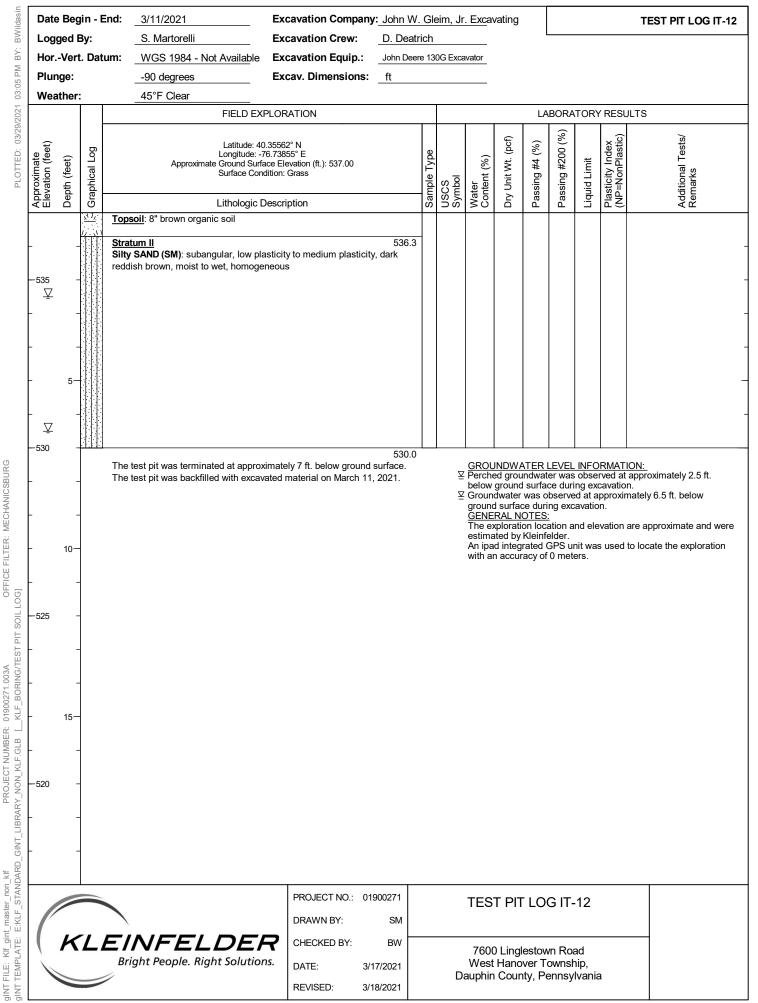




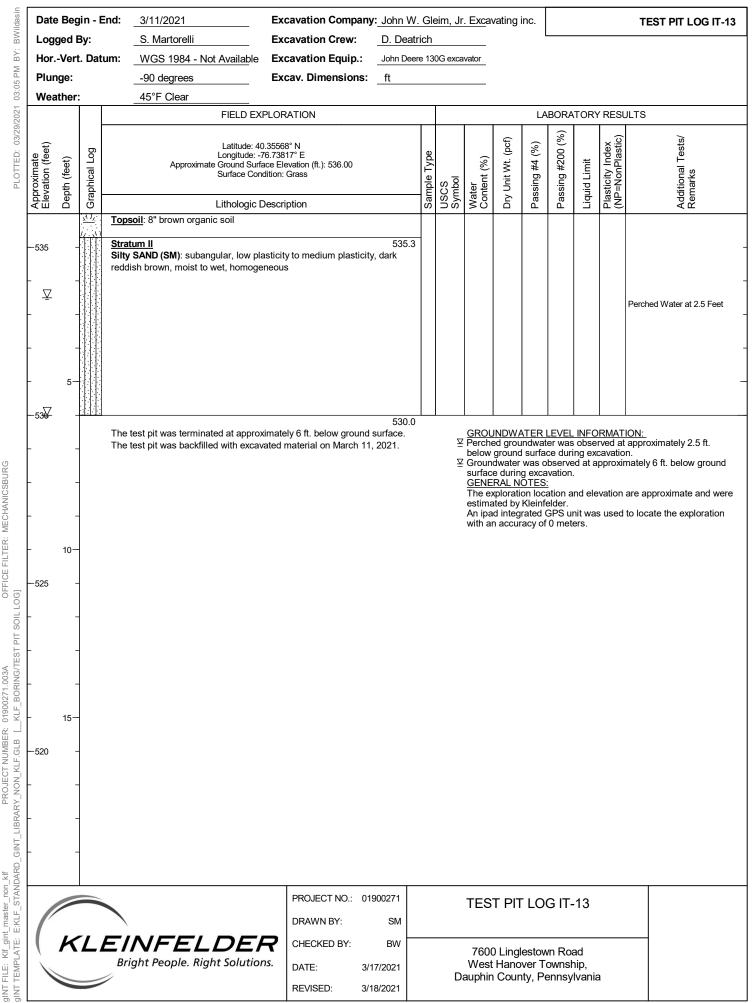




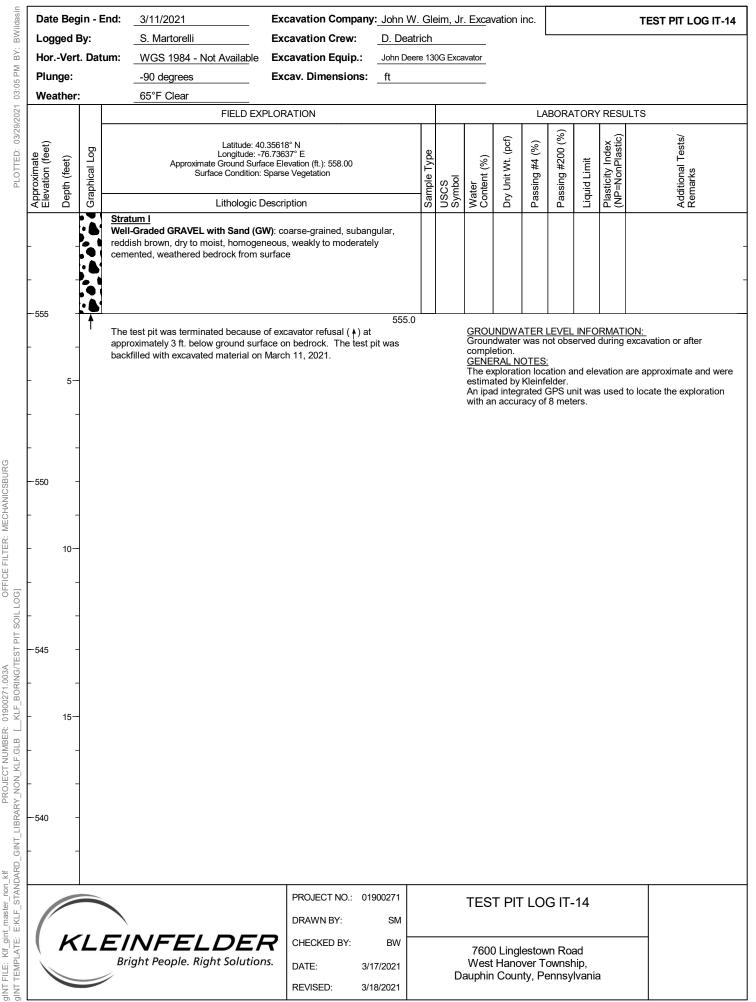




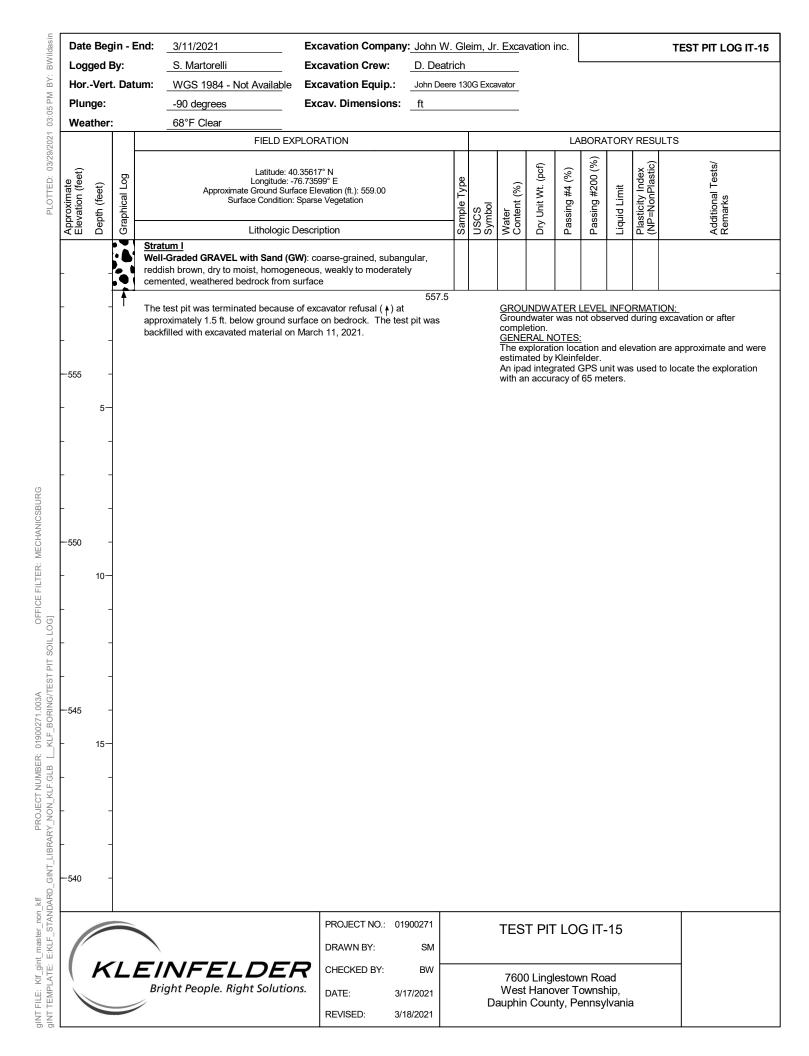
OFFICE FILTER: MECHANICSBURG PROJECT NUMBER: 01900271.003A

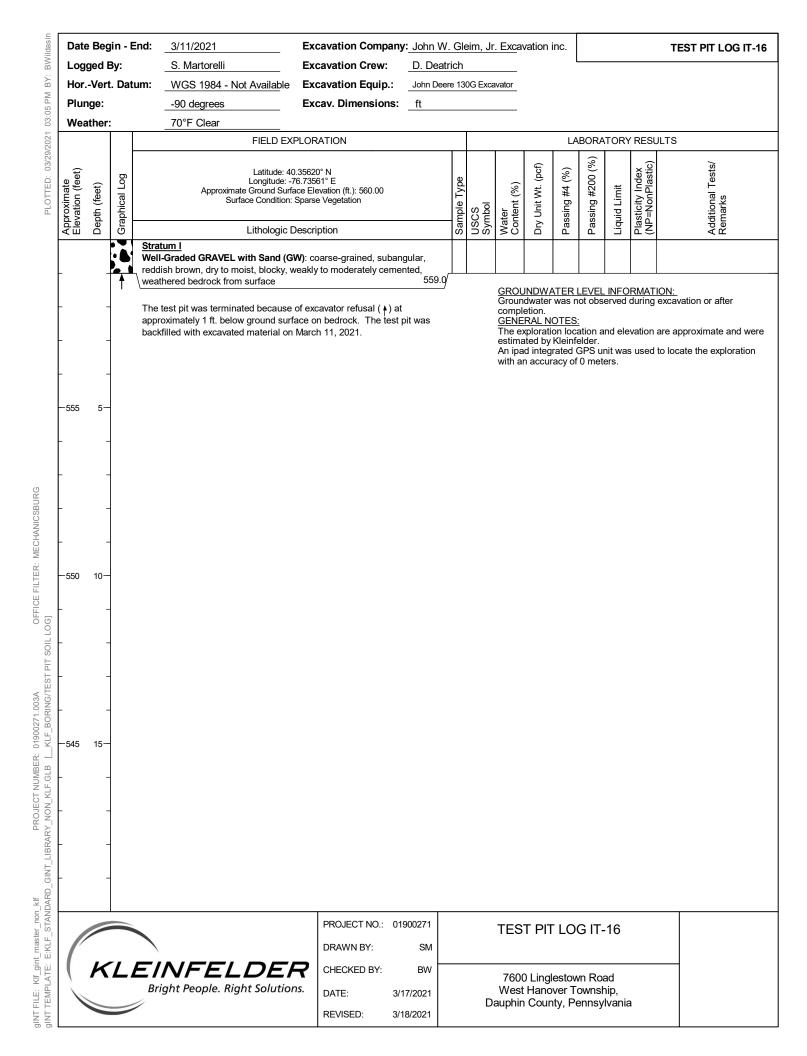


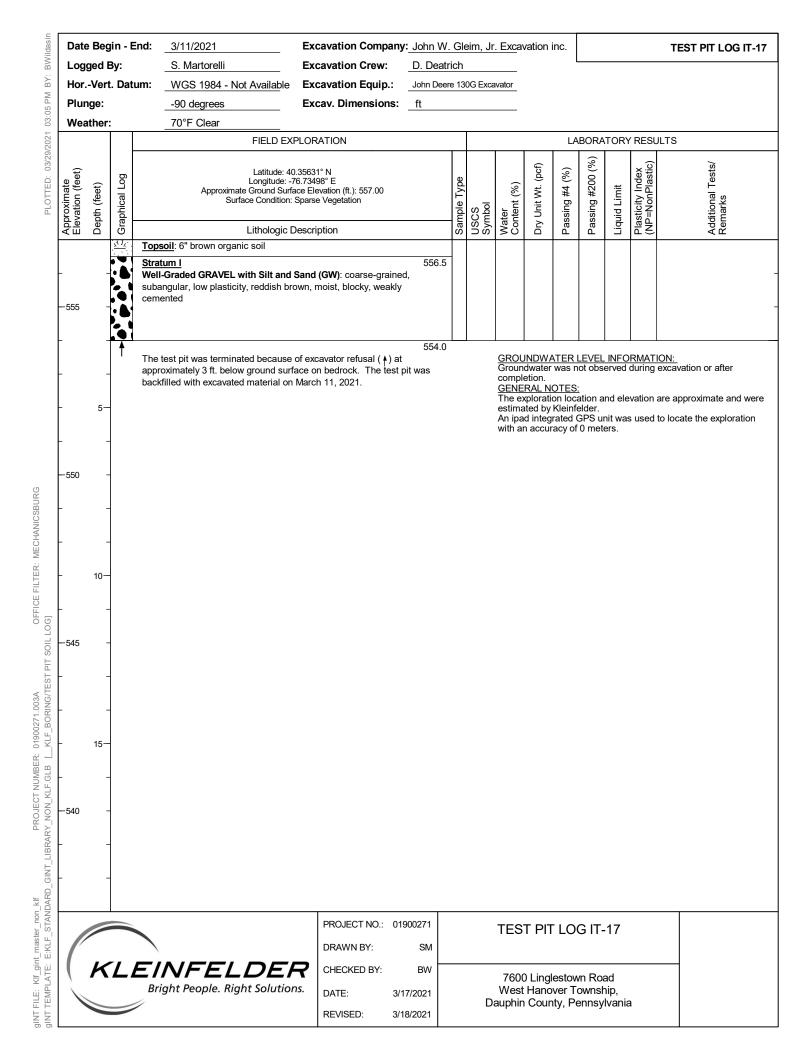
OFFICE FILTER: MECHANICSBURG PROJECT NUMBER: 01900271.003A Klf\_gint\_master\_non\_klf

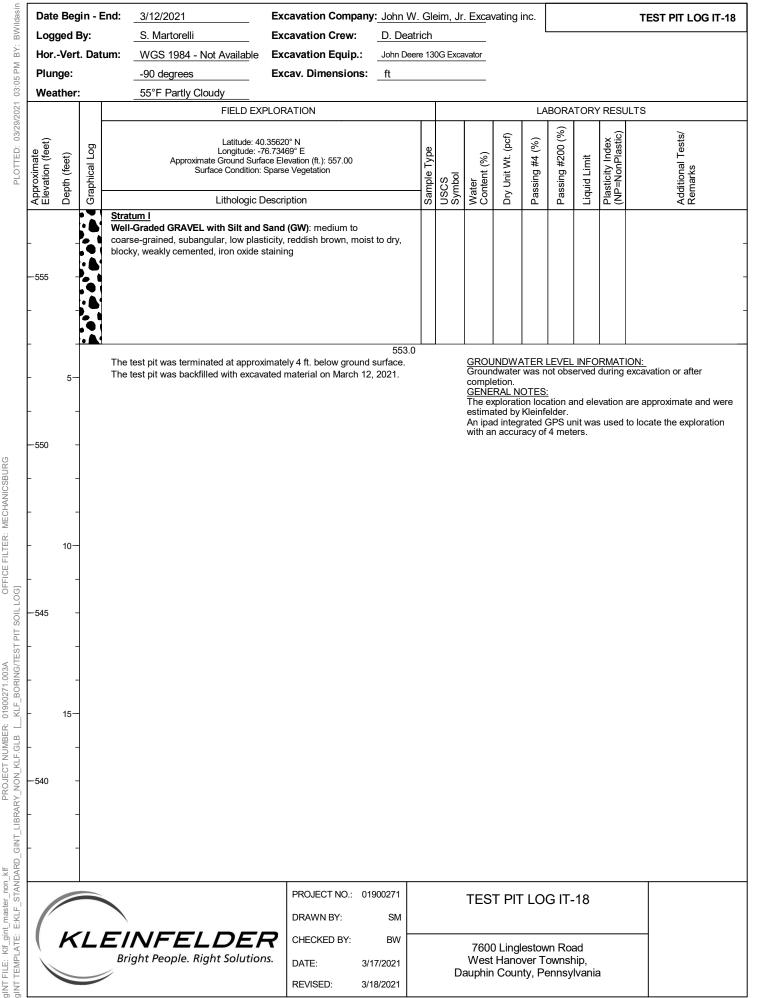


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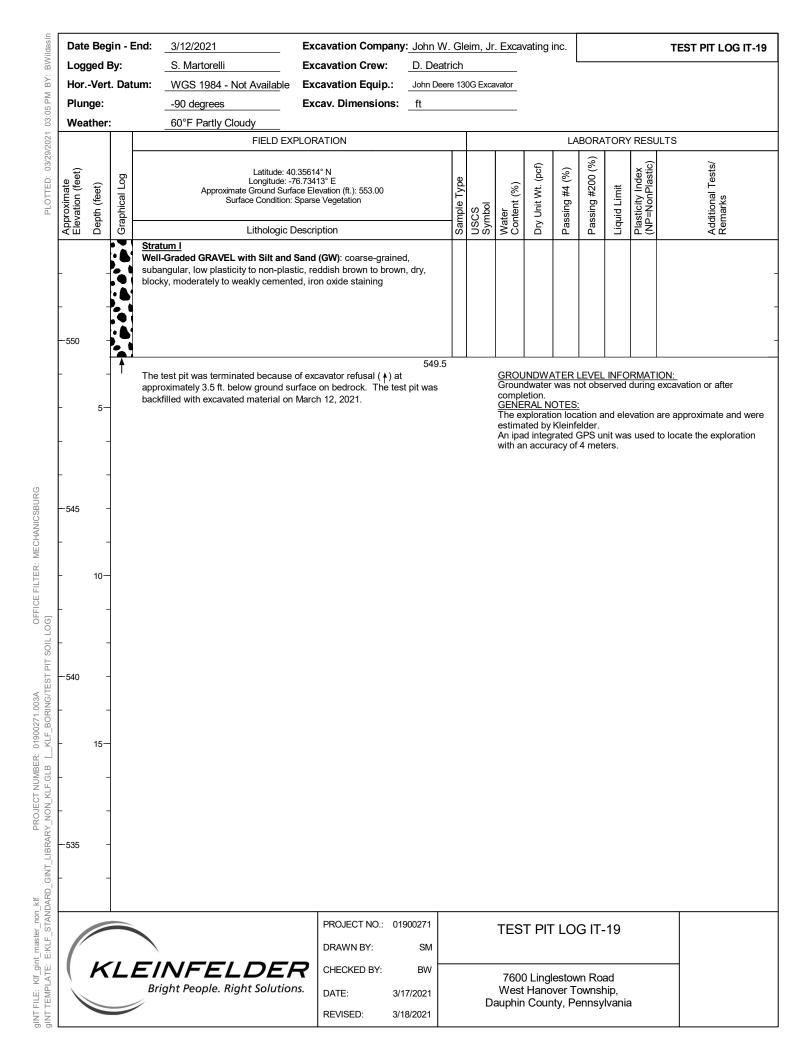




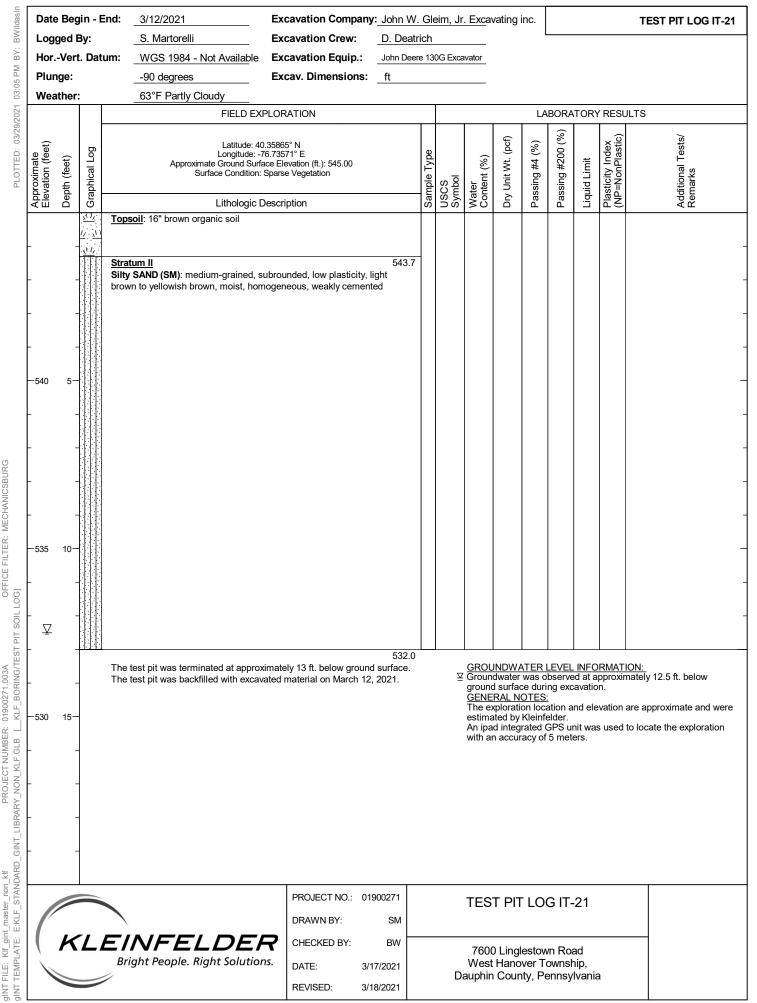




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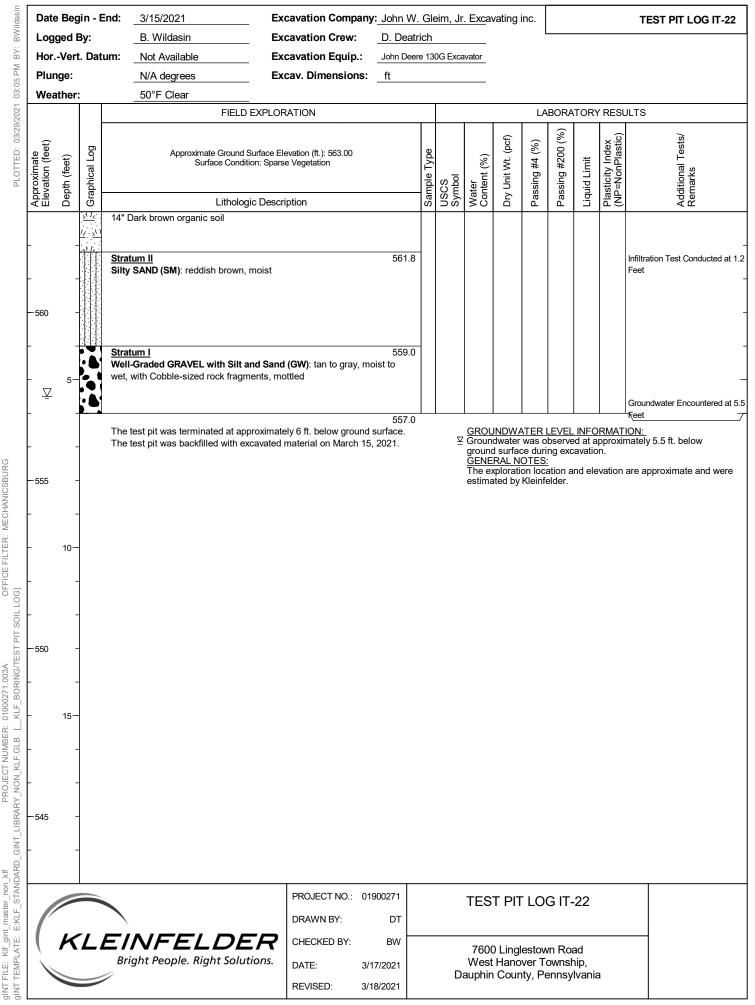


Date	Beg	gin - E	End:	3/12/2021	Excavation Company	Excavation Company: John W. Gleim, Jr. Excavating inc.								TEST PIT LOG IT-20								
Logo	ged E	By:		S. Martorelli	Excavation Crew:	D. Deatr	ich				I											
Hor.	-Vert	t. Dat	um:	WGS 1984 - Not Available								e 130G Excavator										
Plun	ge:			-90 degrees	Excav. Dimensions:	ft																
Wea	ther	:		60°F Partly Cloudy																		
				FIELD EXF	PLORATION						LA	BORA	TOR	( RESL	JLTS							
Approximate				Longitude: -7 Approximate Ground Surfa	Latitude: 40.35861° N Longitude: -76.73534° E imate Ground Surface Elevation (ft.): 540.00 Surface Condition: Sparse Vegetation				Water Content (%)	Dry Unit Wt. (pcf)	Passing #4 (%)	Passing #200 (%)	Liquid Limit	Plasticity Index (NP=NonPlastic)	Additional Tests/ Remarks							
Ele	Dep	Gra		Lithologic D	escription		Sample Type	USCS Symbol	Cor	Dry	Pas	Pas	Liqu	Pla	Adc							
		<u>×17/</u>	Tops	<b>soil</b> : 12" brown organic soil																		
- -  535	- - 5—		Silty plast	tum II r SAND (SM): medium to coarse-g ticity, light brown to yellowish bro ented, trace gravel		539.0 weakly																
- ⊻ - -530	- - 10	-		test pit was terminated at approx test pit was backfilled with excava				<u>.</u> ⊻	Groun surfac <u>GENE</u> The ex estima An ipa	e durin RAL N xplorati ated by	was o g exca OTES on loca Kleinfo rated (	bserve vation ation a elder. GPS u	ed at a Ind ele nit wa:	pproxir evation	I <u>ON:</u> nately 7 ft. below ground are approximate and were to locate the exploration							
-525	- - 15 - -	-																				
(					PROJECT NO.: 01 DRAWN BY:	SM			TES	T PII	LO	G IT-	-20									
	K			<b>WFELDER</b> ight People. Right Solution	<b>PS.</b> DATE: 3/1	BW 7/2021 8/2021		Da		0 Ling t Hano n Cour	ver T	ownsł	nip,	a								



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OFFICE FILTER: MECHANICSBURG

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