

Northwest Regional Office CLEAN WATER PROGRAM

Application Type Renewal Non-Municipal Facility Type Major / Minor Minor

# NPDES PERMIT FACT SHEET INDIVIDUAL SEWAGE

PA0102385 Application No. APS ID 1031495 Authorization ID 1341690

# **Applicant and Facility Information**

Applicant Name IBI RR, LLC d/b/a Iron Bridge Inn		Facility Name	Iron Bridge Inn	
Applicant Address	1438 P	erry Highway	Facility Address	1438 Perry Highway
	Mercer,	PA 16137		Mercer, PA 16137
Applicant Contact	John R	. McKinley	Facility Contact	Michael P. Davidson, operator
Applicant Phone	(412) 302-1518		Facility Phone	(724) 372-3339
Client ID	276623		Site ID	257774
Ch 94 Load Status	Not Ove	erloaded	Municipality	Springfield Township
Connection Status	No Limi	tations	County Mercer County	
Date Application Recei	ived	January 22, 2021	EPA Waived?	Yes
Date Application Accepted		February 5, 2021	If No, Reason	<u>.</u>

## **Summary of Review**

Act 14 - Proof of Notification was submitted and received.

A Part II Water Quality Management permit is not required at this time.

The applicant should be able to meet the limits of this permit, which will protect the uses of the receiving stream.

#### I. OTHER REQUIREMENTS:

- Stormwater into sewers Α.
- В. Right of way
- C. Solids handling
- D. Public Sewerage Availability
- E. Effluent Chlorine Optimization and Minimization

There are no open violations in efacts associated with the subject Client ID (36546) as of 12/1/2021.

Approve	Deny	Signatures	Date	
~		Stephen A. McCauley	10/1/0001	
^		Stephen A. McCauley, E.I.T. / Environmental Engineering Specialist	12/1/2021	
~		Justin C. Dickey	12/3/2021	
^		Justin C. Dickey, P.E. / Environmental Engineer Manager	12/3/2021	

#### SPECIAL CONDITIONS:

Solids Management

II.

ischarge, Receiving Waters and Water Supply I	nformation	
Outfall No.001Latitude41° 09' 27.00"Quad Name-		0.011 -80° 13' 36.00" -
Wastewater Description: Sewage Effluent		
Receiving Waters <u>Neshannock Creek (TSF)</u> NHD Com ID 130031811	Stream Code RMI	<u>35515</u> 20.0
	Viold (ofo/mi2)	0.1
Q <sub>7-10</sub> Flow (cfs) 12.5	Q <sub>7-10</sub> Basis	Calculated
Elevation (ft) 1050	Slope (ft/ft)	0.003157
Watershed No. 20-A	Chapter 93 Class.	TSF
Existing Use	Existing Use Qualifier	-
Exceptions to Use	Exceptions to Criteria	-
Assessment Status Attaining Use(s)		
Cause(s) of Impairment		
Source(s) of Impairment		
TMDL Status	Name	
Background/Ambient Data	Data Source	
рН (SU)	-	
Temperature (°F)	-	
Hardness (mg/L)	_	
Other:	-	
Nearest Downstream Public Water Supply Intake	Beaver Falls Municipal Author	rity - Eastvale
PWS Waters Beaver River	Flow at Intake (cfs)	561
PWS RMI 3.5	Distance from Outfall (mi)	32.0

Sludge use and disposal description and location(s):

All sludge is taken to larger, approved STPs where it is ultimately disposed of at an approved landfill.

#### **Public Participation**

DEP will publish notice of the receipt of the NPDES permit application and a tentative decision to issue the individual NPDES permit in the Pennsylvania Bulletin in accordance with 25 Pa. Code § 92a.82. Upon publication in the Pennsylvania Bulletin, DEP will accept written comments from interested persons for a 30-day period (which may be extended for one additional 15-day period at DEP's discretion), which will be considered in making a final decision on the application. Any person may request or petition for a public hearing with respect to the application. A public hearing may be held if DEP determines that there is significant public interest in holding a hearing. If a hearing is held, notice of the hearing will be published in the Pennsylvania Bulletin at least 30 days prior to the hearing and in at least one newspaper of general circulation within the geographical area of the discharge.

Narrative: This Fact Sheet details the determination of draft NPDES permit limits for an existing discharge of 0.011 MGD of treated sewage from an existing STP in Springfield Township, Mercer County.

Treatment permitted under WQM Permit 4390401 consists of the following: A 2,553 gallon aerated flow equalization tank with an influent grinder pump, a 761.5 gallon settling tank, four 735 gallon step aeration tanks, chemical flocculation with a 617.8 gallon primary tank and a 58.3 gallon secondary tank, a 987 gallon secondary settling tank, two 6,392 gallon sludge digestion tanks, and chlorination with a 536 gallon contact tank and a 122 gallon contact tank. This plant is located at the Iron Bridge Inn site, but also treats sewage from the Rachael's Roadhouse restaurant.

## 1. Streamflow:

Neshannock Creek at Outfall 001:

Yieldrate: Drainage Area:	<u>0.1</u> 125	cfsm sq. mi.	Default (USGS StreamStats)
% of stream allocated:	<u>100%</u>	Basis:	No nearby discharges
Q <sub>7-10</sub> :	<u>12.5</u>	cfs	calculated

## 2. Wasteflow:

Maximum discharge: 0.011 MGD = 0.017 cfs

Runoff flow period: 24 hours Basis: Restaurant flow with equalization

There is greater than 3 parts stream flow (Q7-10) to 1 part effluent (design flow). Therefore, the standards in DEP guidance (391-2000-014) will not be applied.

Flow will be required to be monitored as authorized under Chapter 92a.61, and as recommended in the SOP.

#### 3. Parameters:

The following parameters were evaluated: pH, Total Suspended Solids, Fecal Coliform, E. Coli, Total Phosphorus, Total Nitrogen, NH<sub>3</sub>-N, CBOD<sub>5</sub>, Dissolved Oxygen, and Total Residual Chlorine.

#### a. <u>pH</u>

Between 6.0 and 9.0 at all times

#### b. <u>Total Suspended Solids</u>

Limits are 30.0 mg/l as a monthly average and 60.0 as an instantaneous maximum.

Basis: Application of Chapter 92a47 technology-based limits.

c. Fecal Coliform

05/01 - 09/30:	<u>200/100ml</u> <u>1,000/100ml</u>	(monthly average geometric mean) (instantaneous maximum)
10/01 - 04/30:	<u>2,000/100ml</u> <u>10,000/100ml</u>	(monthly average geometric mean) (instantaneous maximum)

Basis: Application of Chapter 92a47 technology-based limits

Basis: <u>Application of Chapter 93.7 technology-based limits</u>. The measurement frequency was previously set to 4/week but will be increased to 1/day as recommended in the SOP, based on <u>Table 6-3 in the "Technical Guidance for the Development and Specification of Effluent</u> <u>Limitations" (362-0400-001)</u>.

# d. <u>E. Coli</u>

Monitoring was added for E. Coli at a frequency of 1/year.

Basis: Application of Chapter 92a.61 as recommended by the SOP for flows between 0.002 MGD and 0.05 MGD.

## e. Phosphorus

- Limit necessary due to:
  - Discharge to lake, pond, or impoundment
  - Discharge to stream
  - Basis: <u>N/A</u>
- Limit not necessary
  - Basis: <u>Chapter 96.5 does not apply. However, the previous monitoring for Total Phosphorus will</u> be retained in accordance with the SOP, based on Chapter 92a.61.

## f. Total Nitrogen

The previous monitoring for Total Nitrogen will be retained in accordance with the SOP, based on Chapter 92a.61.

## g. <u>Ammonia-Nitrogen (NH<sub>3</sub>-N)</u>

Median discharge pH to be used:	: <u>7.2</u> Standard Units (S.U.)					
	В	Basis: eDMR data for previous 12 months				
Discharge temperature:	<u>25°C</u>	(default value used in the absence of data)				
Median stream pH to be used:	<u>7.0</u>	Standard Units (S.U.)				
	В	Basis: default value used in the absence of data				
Stream Temperature:	<u>25°C</u>	(default value used for TSF modeling)				
Background NH <sub>3</sub> -N concentration:	<u>0.1</u>	mg/l				
	В	Basis: <u>Default value</u>				
Calculated NH <sub>3</sub> -N Summer limits:	<u>25.0</u> 50.0	mg/l (monthly average) mg/l (instantaneous maximum)				
Calculated NH <sub>3</sub> -N Winter limits:	<u>25.0</u> 50.0	mg/l (monthly average) mg/l (instantaneous maximum)				
Result: WO modeling resulted in	the sum	mer limits above (see Attachment 1). The winter limi				

Result: WQ modeling resulted in the summer limits above (see Attachment 1). The winter limits are calculated as three times the summer limits, but since the technology-based limits would govern, they will be used. Since the calculated limits are the same as in the previous permit, they will be retained. Since the summer and winter limits are technology-based, per the SOP, the winter limits will remain monitoring only with this renewal. h. <u>CBOD₅</u>

Median discharge pH to be used:	<u>7.2</u>	Standard Units (S.U.)
	В	Basis: eDMR data for previous 12 months
Discharge temperature:	<u>25°C</u>	(default value used in the absence of data)
Median stream pH to be used:	<u>7.0</u>	Standard Units (S.U.)
	В	Basis: default value used in the absence of data
Stream Temperature:	<u>25°C</u>	(default value used for TSF modeling)
Background CBOD5 concentration:	<u>2.0</u>	mg/l
	В	Basis: <u>Default value</u>
Calculated CBOD <sub>5</sub> Summer limits:	<u>25.0</u> 50.0	mg/l (monthly average) mg/l (instantaneous maximum)
Calculated CBOD <sub>5</sub> Winter limits:	<u>25.0</u> 50.0	mg/l (monthly average) mg/l (instantaneous maximum)

Result: WQ modeling resulted in the summer limits above (see Attachment 1). The winter limits are calculated as three times the summer limits, but since the technology-based limits would govern, they will be used. Since the calculated limits are the same as in the previous permit, they will be retained. Since the summer and winter limits are technology-based, the year-round limit of 25.0 mg/l monthly average and 50.0 mg/l instantaneous maximum will be retained with this renewal.

# i. <u>Dissolved Oxygen (DO)</u>

- 4.0 mg/l minimum desired in effluent to protect all aquatic life
- 5.0 mg/l desired in effluent for CWF, WWF, or TSF
- 6.0 mg/l minimum required due to discharge falling under guidance document 391-2000-014
- 8.0 mg/l required due to discharge going to a naturally reproducing salmonid stream

Discussion: The Dissolved Oxygen minimum of 4.0 mg/l will be retained with this renewal. The technologybased minimum of 4.0 mg/l is recommended by the WQ Model (see Attachment 1) and the SOP based on Chapter 93.7, under the authority of Chapter 92a.61. The measurement frequency was previously set to 4/week but will be increased to 1/day as recommended in the SOP, based on Table 6-3 in the "Technical Guidance for the Development and Specification of Effluent Limitations" (362-0400-001).

# j. <u>Total Residual Chlorine (TRC)</u>

No limit necessary

Basis: <u>N/A</u>

- $\square$  TRC limits: <u>0.5</u> mg/l (monthly average)
  - <u>1.6</u> mg/l (instantaneous maximum)
  - Basis: The technology-based TRC limits above were calculated using the Department's TRC Calculation Spreadsheet (see Attachment 2). The measurement frequency was previously set to 4/week but will be increased to 1/day as recommended in the SOP, based on Table 6-3 in the "Technical Guidance for the Development and Specification of Effluent Limitations" (362-0400-001).

#### 4. Reasonable Potential Analysis for Receiving Stream:

A Reasonable Potential Analysis was not performed in accordance with State practices for Outfall 001 since no sampling was reported in the renewal application.

Result: <u>N/A</u>

#### 5. Reasonable Potential for Downstream Public Water Supply (PWS):

The Department's Toxics Management Spreadsheet does not calculate limits for parameters that are based on PWS criteria (TDS, Chloride, Bromide, and Sulfate). However, since no sample data was provided, mass-balance calculations were not performed.

Nearest Downstream potable water supply (PWS):	Beaver Falls Municipal Authority - Eastvale
Distance downstream from the point of discharge:	<u>32.0</u> miles (approximate)

- No limits necessary
- Limits needed
  - Basis: Significant dilution available

#### 7. Anti-Backsliding:

Since all the permit limits in this renewal are the same or more restrictive than the previous NPDES Permit, antibacksliding is not applicable.

#### 8. Attachment List:

- Attachment 1 WQ Modeling Printouts
- Attachment 2 TRC\_Calc Spreadsheet

(The Attachments above can be found at the end of this document)

# **Compliance History**

# DMR Data for Outfall 001 (from November 1, 2020 to October 31, 2021)

Parameter	OCT-21	SEP-21	AUG-21	JUL-21	JUN-21	MAY-21	APR-21	MAR-21	FEB-21	JAN-21	DEC-20	NOV-20
Flow (MGD)												
Average Monthly	0.005	0.005	0.006	0.006	0.006	0.006	0.005	0.005	0.005	0.004	0.003	0.005
Flow (MGD)												
Daily Maximum	0.007	0.008	0.006	0.007	0.008	0.007	0.008	0.007	0.007	0.006	0.007	0.007
pH (S.U.)												
Minimum	7.1	7.0	7.1	7.1	7.1	7.1	7.0	7.0	7.2	7.0	7.0	7.0
pH (S.U.)												
Maximum	7.4	7.5	7.5	7.5	7.4	7.4	7.3	7.4	7.6	7.8	7.8	7.7
DO (mg/L)												
Minimum	7.4	6.8	5.1	6.1	6.0	7.2	6.5	7.5	7.7	8.2	9.3	5.2
TRC (mg/L)												
Average Monthly	0.4	0.4	0.3	0.3	0.4	0.4	0.4	0.3	0.4	0.3	0.3	0.4
TRC (mg/L)										<b>.</b>		
Instantaneous Maximum	0.6	0.6	0.4	0.5	0.5	0.5	0.7	0.4	0.6	0.4	0.4	0.7
CBOD5 (mg/L)				0.5	07		5.0	10.0	. 1.0	. 1.0	. 4.5	
Average Monthly	< 2.0	< 3.2	< 3.6	6.5	< 9.7	< 2.0	5.6	< 12.6	< 4.3	< 4.9	< 4.5	< 4.5
TSS (mg/L)	< 5.0	< 5.0	< 5.0	5.0	< 5.0	< 5.0	. 5.0	1 O F	. 5.0	. 5.0	. 5.0	15.0
Average Monthly Fecal Coliform (CFU/100 ml)	< 5.0	< 5.0	< 5.0	5.0	< 5.0	< 5.0	< 5.0	< 9.5	< 5.0	< 5.0	< 5.0	< 5.0
Geometric Mean	5	< 5	5	< 5	99	< 7	< 7	220	< 10	< 10	309	< 62
Fecal Coliform (CFU/100 ml)	5	< 5	5	~ 5		~ /	~ /	220			303	< 02
Instantaneous Maximum	5	< 5	22	< 5	612	< 10	10	2420	10	< 10	1842	383
Total Nitrogen (mg/L)	Ű	~ ~ ~		~ ~ ~	012		10	2120	10		1012	000
Average Monthly		8.93			1.92			4.61			3.48	
Ammonia (mg/L)												
Average Monthly	< 0.8	< 0.8	< 0.8	< 0.8	< 0.8	< 0.8	< 0.8	< 0.8	< 1	< 1	< 0.8	< 0.8
Total Phosphorus (mg/L)												
Average Monthly		4.9			0.54			2.0			2.0	

#### **Proposed Effluent Limitations and Monitoring Requirements**

The limitations and monitoring requirements specified below are proposed for the draft permit, and reflect the most stringent limitations amongst technology, water quality and BPJ. Instantaneous Maximum (IMAX) limits are determined using multipliers of 2 (conventional pollutants) or 2.5 (toxic pollutants). Sample frequencies and types are derived from the "NPDES Permit Writer's Manual" (362-0400-001), SOPs and/or BPJ.

#### Outfall 001, Effective Period: Permit Effective Date through Permit Expiration Date.

		Monitoring Requirements						
Parameter	Mass Units	; (lbs/day) <sup>(1)</sup>		Concentrat	Minimum <sup>(2)</sup>	Required		
Falameter	Average Monthly	Average Weekly	Minimum	Average Monthly	Maximum	Instant. Maximum	Measurement Frequency	Sample Type
Flow (MGD)	Report	Report Daily Max	xxx	xxx	xxx	xxx	1/week	Measured
рН (S.U.)	xxx	xxx	6.0 Inst Min	xxx	xxx	9.0	1/day	Grab
DO	xxx	xxx	4.0 Inst Min	xxx	xxx	xxx	1/day	Grab
TRC	XXX	XXX	XXX	0.5	XXX	1.6	1/day	Grab
CBOD5	xxx	XXX	XXX	25.0	XXX	50	2/month	8-Hr Composite
TSS	ххх	XXX	XXX	30.0	XXX	60	2/month	8-Hr Composite
Fecal Coliform (No./100 ml) Oct 1 - Apr 30	ххх	xxx	xxx	2000 Geo Mean	XXX	10000	2/month	Grab
Fecal Coliform (No./100 ml) May 1 - Sep 30	ххх	xxx	xxx	200 Geo Mean	XXX	1000	2/month	Grab
E. Coli (No./100 ml)	XXX	XXX	XXX	XXX	XXX	Report	1/year	Grab
Total Nitrogen	XXX	XXX	XXX	Report Avg Qrtly	XXX	XXX	1/quarter	8-Hr Composite
Ammonia-Nitrogen Nov 1 - Apr 30	ххх	XXX	XXX	Report	XXX	XXX	2/month	8-Hr Composite
Ammonia-Nitrogen May 1 - Oct 31	xxx	xxx	xxx	25.0	xxx	50	2/month	8-Hr Composite
Total Phosphorus	XXX	XXX	XXX	Report Avg Qrtly	xxx	XXX	1/quarter	8-Hr Composite

Compliance Sampling Location: at Outfall 001, after disinfection.

Flow is monitor only based on Chapter 92a.61. The limits for pH and Dissolved Oxygen are technology-based on Chapter 93.7. The Total Residual Chlorine (TRC) limits are technology-based on Chapter 92a.48. The limits for CBOD<sub>5</sub>, Total Suspended Solids, and Fecal Coliforms are technology-based on Chapter 92a.47. The summer limits for Ammonia-Nitrogen are technology-based on Chapter 93.7. Monitoring for winter NH3-N, E. Coli, Total Nitrogen, and Total Phosphorus is based on Chapter 92a.61.

Attachment 1

	<u>SWP Basin</u> 20A	Stream Code 35515		<u>Stream Name</u> NESHANNOCK C	-		
RMI	Name	Permit Number	Disc Flow (mgd)	Parameter	Effl. Limit 30-day Ave. (mg/L)	Effl. Limit Maximum (mg/L)	Effl. Limit Minimum (mg/L)
20.000	Iron Bridge	PA0102385	0.011	CBOD5	25		
				NH3-N	25	50	
				Dissolved Oxygen			4

# WQM 7.0 Effluent Limits

Wednesday, December 1, 2021

Version 1.1

<u>SWP Basin</u> <u>St</u> 20A	ream Code 35515		NE	<u>Stream Name</u> SHANNOCK CREEK	
<u>RMI</u> 20.000	<u>Total Discharge</u> 0.01	SAND.	) <u>Ana</u>	lysis Temperature (°C) 25.000	<u>Analysis pH</u> 7.000
Reach Width (ft)	Reach De			Reach WDRatio	Reach Velocity (fps)
54.433	0.79			68.131	0.288
<u>Reach CBOD5 (mg/L)</u> 2.03 <u>Reach DO (mg/L)</u> 8.237	<u>Reach Kc (</u> 0.02 <u>Reach Kr (</u> 6.98	2 1/days)	<u>R</u>	<u>each NH3-N (mg/L)</u> 0.03 <u>Kr Equation</u> Tsivoglou	<u>Reach Kn (1/days)</u> 1.029 <u>Reach DO Goal (mg/L)</u> 5
<u>Reach Travel Time (days)</u> 0.127	TravTime (days)	Subreach CBOD5 (mg/L)	n <b>Results</b> NH3-N (mg/L)	D.O. (mg/L)	
	0.013	2.03	0.03	7.54	
	0.025	2.03	0.03	7.54	
	0.038	2.03	0.03	7.54	
	0.051	2.03	0.03	7.54	
	0.064	10-0000	0.03	7.54	
	0.076	4.7776(7.67677)	0.03	7.54	
	0.089 0.102	2.03	0.03	7.54 7.54	
	0.102	2.03 2.02	0.03 0.03	7.54 7.54	
	0.113	2.02	0.03	7.54	

# WQM 7.0 D.O.Simulation

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# WQM 7.0 Modeling Specifications

Parameters	Both	Use Inputted Q1-10 and Q30-10 Flows	✓
WLA Method	EMPR	Use Inputted W/D Ratio	
Q1-10/Q7-10 Ratio	0.64	Use Inputted Reach Travel Times	
Q30-10/Q7-10 Ratio	1.36	Temperature Adjust Kr	✓
D.O. Saturation	90.00%	Use Balanced Technology	$\checkmark$
D.O. Goal	5		

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Input Data WQM	7.0	
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	SWP Basir			Stre	eam Name		RMI	Eleva (ft)		Drainage Area (sq mi)	Slop (ft/f	Witho	VS Irawal gd)	Apply FC
	20A	35	515 NESH	ANNOCK	CREEK		20.00	<b>)0</b> 10	50.00	125.0	0.00	000	0.00	✓
					St	ream Dat	ta							
Design Cond.	LFY	Trib Flow	Stream Flow	Rch Trav Time	Rch Velocity	WD Ratio	Rch Width	Rch Depth	Tem	<u>Tributary</u> p pH	I	<u>Strear</u> Temp	n pH	
Conu.	(cfsm)	(cfs)	(cfs)	(days)	(fps)		(ft)	(ft)	(°C)	)		(°C)		
Q7-10 Q1-10 Q30-10	0.100	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	0.000 0.000 0.000	0.0	0.00	0.00	2	5.00 7	00	0.00	0.00	
	Ĩ				Di	scharge	Data						1	
			Name	Per	mit Number	Disc	Permitte Disc Flow (mgd)	Disc Flow	Res Fa	erve Te ctor	isc :mp 'C)	Disc pH		
		Iron E	Bridge	PA	0102385	0.011	0 0.000	0 0.000	10 (	0.000	25.00	7.20		
					Pa	rameter	Data							
		Parameter Na			r Name				ream Conc	Fate Coef				
	_		10		444500008491450	(m	ng/L) (m	ng/L) (n	ng/L)	(1/days)				
			CBOD5				25.00	2.00	0.00	1.50				
			Dissolved	Oxygen			4.00	8.24	0.00	0.00				
			NH3-N				25.00	0.00	0.00	0.70				

Input Data WQM	7.0	
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	SWP Basin			Stre	eam Name		RMI	Eleva (ft		Drainage Area (sq mi)	Slop (ft/ft	Witho	VS Irawal gd)	Apply FC
	20A	355	515 NESH	ANNOCK	CREEK		19.40	<b>)0</b> 10	40.00	133.00	0.000	000	0.00	✓
					St	ream Dat	a							
Design Cond.	LFY	Trib Flow	Stream Flow	Rch Tra∨ Time	Rch Velocity	WD Ratio	Rch Width	Rch Depth	Tem	<u>Tributary</u> p pH	1	<u>Strear</u> Temp	n pH	
oona.	(cfsm)	(cfs)	(cfs)	(days)	(fps)		(ft)	(ft)	(°C)	)		(°C)		
Q7-10	0.100	0.00	0.00	0.000	0.000	0.0	0.00	0.00	2	5.00 7	.00	0.00	0.00	
Q1-10 Q30-10		0.00	0.00 0.00	0.000 0.000	0.000									
Q30-10		0.00	0.00	0.000	0.000									
					Di	scharge	Data							
			Name	Per	mit Number	Disc	Permitte Disc Flow (mgd)	ed Design Disc Flow (mgd)	Res Fa	Di erve Te ctor ( <sup>o</sup>	mp	Disc pH		
		-				0.000	0 0.000	0 0.000	)0 (	0.000	25.00	7.00		
					Pa	rameter	Data							
		Parameter Name							ream Conc	Fate Coef				
			i.	aramete	IName	(m	ıg/L) (n	ng/L) (r	ng/L)	(1/days)				
		CBOD5				25.00	2.00	0.00	1.50					
			Dissolved	Oxygen			3.00	8.24	0.00	0.00				
			NH3-N				25.00	0.00	0.00	0.70				

		N	<u>/QM 7</u>	<u>.0 Was</u>	teloac	<u>ollA k</u>	catio	ns		
	SWP Basin	Stream	n Code			<u>Stream</u>	Name			
	20A	35	515		NE	SHANNO		ΞK		
NH3-N	Acute Alloca	tions								
RMI	Discharge N	ame	Baseline Criterion (mg/L)	Baseline WLA (mg/L)	Multip Criteri (mg/L	on ∖	ultiple VLA ng/L)	Critical Reach	Percent Reductio	
20.00	0 Iron Bridge		11.07	50	) 11	1.07	50	0	0	
NH3-N	Chronic Allo	catio	ns							
RMI	Discharge Na	me C	aseline Criterion (mg/L)	Baseline WLA (mg/L)	Multiple Criterior (mg/L)	n W	iple LA g/L)	Critical Reach	Percent Reduction	
20.00	0 Iron Bridge		1.37	25	i 1	1.37	25	0	0	
Dissolv	ed Oxygen A	lloca	tions							
				CBOD5		<u>13-N</u>		ed Oxygen	Critical	Percent
RMI	Discharge	Name	Baselin (mg/L	Share Sheer and Sheer a	Baseline (mg/L)	Multiple (mg/L)	Baseline (mg/L)	San Silan San San San San San San San San San S	Reach	Reduction
20.0	0 Iron Bridge		:	25 25	25	25	4	4	0	0

# WOM 7.0 Wasteload Allocations

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Version 1.1

	-10 <del>-</del>	<u>P Basin</u> 20A	<u>Stream Code</u> 35515			<u>Stream Name</u> NESHANNOCK CREEK						
RMI	Stream Flow	PWS With	Net Stream Flow	Disc Analysis Flow	10	Depth	Width	W/D Ratio	Velocity	Reach Tra∨ Time	Analysis Temp	Analysis pH
	(cfs)	(cfs)	(cfs)	(cfs)	(ft/ft)	(ft)	(ft)		(fps)	(days)	(°C)	
Q7-10	0 Flow											
20.000	12.50	0.00	12.50	.017	0.00316	.799	54.43	68.13	0.29	0.127	25.00	7.00
Q1-10	0 Flow											
20.000	8.00	0.00	8.00	.017	0.00316	NA	NA	NA	0.22	0.163	25.00	7.00
Q30-	10 Flow	l										
20.000	17.00	0.00	17.00	.017	0.00316	NA	NA	NA	0.34	0.107	25.00	7.00

# WQM 7.0 Hydrodynamic Outputs

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Attachment 2

TRC EVALU/	ATION								
Input appropria	te values in A	3:A9 and D3:D9							
12.5	= Q stream (cf	s)	0.5	= CV Daily					
0.011	= Q discharge	(MGD)	0.5	= CV Hourly					
30	= no. samples		1	= AFC_Partial I	Aix Factor				
0.3	= Chlorine Dei	mand of Stream	1	= CFC_Partial I	Aix Factor				
0	= Chlorine Dei	mand of Discharge	15	= AFC_Criteria	Compliance Time (min)				
0.5	= BAT/BPJ Va	lue	720	= CFC_Criteria	Compliance Time (min)				
0	= % Factor of	Safety (FOS)	0	=Decay Coeffic	eient (K)				
Source	Reference	AFC Calculations		Reference	CFC Calculations				
TRC	1.3.2.iii	WLA afc =	234.343	1.3.2.iii	WLA cfc = 228.459				
PENTOXSD TRG	5.1a	LTAMULT afc =	0.373	5.1c	LTAMULT cfc = 0.581				
PENTOXSD TRG	5.1b	LTA_afc=	87.322	5.1d	LTA_cfc = 132.815				
Source		Efflue	nt Limit Calcu	lations					
PENTOXSD TRG	5.1f		AML MULT =	1.231					
PENTOXSD TRG	5.1g AVG MON LIMIT (mg/l) = 0.500 BAT/BPJ								
		INST MAX	_IMIT (mg/l) =	1.635					
WLA afc LTAMULT afc LTA_afc	+ Xd + (AFC	C_tc)) + [(AFC_Yc*Qs*.019 _Yc*Qs*Xs/Qd)]*(1-FOS/10 vh^2+1))-2.326*LN(cvh^2+ ULT_afc	0)	9_tc))					
WLA_cfc	(S) (S)	C_tc) + [(CFC_Yc*Qs*.011/ _Yc*Qs*Xs/Qd)]*(1-FOS/10	120	_tc) )					
LTAMULT_cfc <b>LTA_cfc</b>	EXP((0.5*LN(c wla_cfc*LTAM	vd^2/no_samples+1))-2.32 ULT_cfc	6*LN(cvd^2/n	o_samples+1)^(	).5)				
AML MULT AVG MON LIMIT	3 <b>5</b> 3 20	((cvd^2/no_samples+1)^0. ,MIN(LTA_afc,LTA_cfc)*AM	- 10 U	^2/no_samples+	-1))				