

**HORIZONTAL DIRECTIONAL DRILL ANALYSIS
WETLAND J-47 CROSSING
PADEP SECTION 105 PERMIT NO.S: E22-617; E38-194
PA-LE-0001.0000-SR-16
(SPLP HDD No. S3-0090-16)**

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This reevaluation of the horizontal directional drill (HDD) installation of a 16-inch diameter pipeline that traverses under Wetland J-47, and Streams S-A48, S-K23, and S-K17 has been completed in accordance with Stipulated Order issued under Environmental Hearing Board Docket No. 2017-009-L (Order). During drilling of the first HDD for installation of the 20-inch diameter pipeline, inadvertent returns (IRs) to the land surface occurred as the pilot drill was progressing toward the exit, with the first IR occurring 140 foot (ft) before exiting. Due to these IR events the second HDD requires reevaluation in accordance with the Order.

A Reevaluation for both the 16-inch and 20-inch HDDs was submitted by Sunoco Pipeline, L.P. (SPLP) on October 5, 2017, and was approved by the Pennsylvania Department of Environmental Protection on November 20, 2017. As stated above, an IR occurred in the pilot phase for the 20-inch pipeline. Ultimately, the east exit point was shifted 150 ft eastward and the 20-inch pilot hole drill, HDD reaming phase, and pipeline installation was completed with no additional IRs. The second revision of the 16-inch HDD profile generally follows the field revised and permitted 20-inch HDD profile.

PIPE INFORMATION

16-Inch: 0.438 wall thickness; X-70

Pipe stress allowances are an integral part of the design calculations performed for each HDD.

REDESIGNED HORIZONTAL DIRECTIONAL DRILL DESIGN SUMMARY: 16-INCH

- Horizontal length: 2,200 ft
- Entry/Exit angle: 16 degrees
- Maximum Depth of cover: 137 ft
- Pipe design radius: 1,800 ft

GEOLOGIC AND HYDROGEOLOGIC ANALYSIS

Based upon publications by the Pennsylvania Bureau of Topographic and Geologic Survey (BTGS, 2001 and Sevon, 2000), the site is in the Gettysburg-Newark Lowland Section of the Piedmont Physiographic Province of Pennsylvania, underlain by sedimentary rocks of the Newark Group. Local topography is characterized by rolling lowlands, shallow valleys, and isolated hills (Geyer and Wilshusen, 1982).

The HDD site geology is mapped as the Gettysburg Formation and Gettysburg Formation-Conglomerate. The Gettysburg Formation is described as red shale, red, brown and gray sandstone, and quartz and limestone conglomerate (Geyer and Wilshusen, 1982). The Gettysburg Formation-Conglomerate is described as coarse, quartz conglomerate containing rounded pebbles and cobbles in a matrix of red sand (Geyer and Wilshusen, 1982). The general structure of the Newark Group is a north-northwestward dipping homocline. Typical dip directions are north or northwest and range from 20° to 40° (Newport, 1971). Intrusive diabase has been mapped north and south of the HDD.

Karst geology is not present at this HDD location. At this HDD location the use of geophysics assessments was considered but not conducted because the results from these types of assessments provide limited useable data after 20 to 50 ft below the ground surface (bgs) varying by the nature of the geologic structure.

Attachment 1 provides an extensive discussion on the geology and results of the geotechnical investigation performed at this location.

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HYDROGEOLOGY, GROUND WATER, AND WELL PRODUCTION ZONES

Groundwater in the vicinity of the Wetland J-47 HDD moves in interconnected, secondary openings such as fractures and joints in the sedimentary bedrock aquifer system. Typically, these openings are best developed and found more frequently near the surface. At depth, these openings occur less frequently and tend to be smaller due to compressional loading (Wood, 1980).

Based upon reported data on 332 wells in the Gettysburg Formation, water-bearing zones range from 5 to 900 ft below the ground surface (bgs). Fifty percent of the 669 reported water-bearing zones were penetrated at a depth of 115 ft or less, with 90% of the water-bearing zones encountered at a depth of 288 ft bgs or less. The greatest density of water-bearing zones is from approximately 51 to 100 ft bgs. The density of water-bearing zones encountered at depths greater than 401 feet are based on five or fewer zones per 50 ft interval. Overall density of water-bearing zones in the Gettysburg Formation is 0.41 per 50 ft of well depth (Low, et. al., 2002).

Attachment 1 provides an extensive discussion on the hydrogeology and results of the geotechnical investigation performed at this location.

INADVERTENT RETURN (IR) DISCUSSION

During drilling of the 20-inch HDD pilot hole, a loss of returns (LOR) of between 500-1,000 gallons occurred at 360 ft into the drill with no IR. At 2,087 ft of progress of the pilot hole and at 23.3 ft depth below the ground surface, a 50 gallon IR occurred and the surface flow followed the topography and entered Wetland A-30 and Stream K-23. The drilling contractor attempted injection of Loss Control Materials with no success, and further IRs of 15 gallons and 900 gallons occurred at 2,095 ft of progress before drilling was suspended.

The root cause of the IRs was determined to be the shallow proximity of the HDD profile parallel to the land surface in the last 150 ft of the profile. To achieve greater depth below ground for the pilot hole drilling, an intercept profile was designed with a revised HDD entry point established 150 ft east of the original exit point, and new easement and surface workspace was acquired for the HDD redesign. The 150-ft setback and increased angle of the intercept entry point allowed the pilot HDD to advance more rapidly into intact bedrock and eliminated the IR occurrences. No IRs during drilling of the revised intercept profile.

A matching revision and setback is proposed for drilling and installation of the 16-inch pipeline. The weakest point in the original profile is within the first 400 ft of the beginning and end of the drill. The revised design profiles provide for a maximum of 119 ft of cover above the 16-inch pilot drill, and greater than 58 ft depth of cover where IRs previously occurred.

ADJACENT FEATURES ANALYSIS

The crossing of Wetland J-47 is located in rural Dauphin and Lebanon counties, approximately 3.6 miles southeast of Hershey, Pennsylvania. The pipeline route follows parallel to two previously existing Sunoco pipelines.

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This HDD location is within unmanaged deciduous woodlands. The HDD would cross under two streams and one wetland, none of which are designated as exceptional value. A 3.5-acre impoundment occurs approximately 385 ft north-northwest of the HDD location.

In addition to the resources listed above, based upon the data from the Pennsylvania Groundwater Information System (PaGWIS) and review of aerial photography, five domestic (private) supply wells were identified within 450 ft of the proposed HDD. Of these, three occur north of the HDD profile at distances between 180 and 530 ft. Based upon the data in PaGWIS, the well depths are 100 to 150 ft bgs, with a reported static water level of 30 bgs. Typically, a “good drilling mud program” forms a “cake wall” around the diameter of the pilot or reamer during drilling process which seals fissures within the profile geology and limits the horizontal and vertical movement of drilling fluids. Secondly, controlling the down hole mud weights and pressures should minimize the lateral movement of these materials through the geology. These wells will have to be monitored during the HDD process in accordance with PADEP requirements.

To further avoid and mitigate any adverse effects from the HDD to private water wells, and in accordance with the requirements of the Stipulated Order, SPLP will transmit a copy of this HDD analysis to all landowners having a property line within 450 ft of any direction of this HDD location. SPLP will also inform these landowners that SPLP will conduct pre-, during, and post-construction sampling of their private water wells to ensure that mitigating actions are taken, if necessary.

No impacts to the adjacent private water supplies were reported during drilling and installation of the 20-inch pipeline and no problems with water quality were detected by the well testing and analysis.

ALTERNATIVES ANALYSIS

The 2nd revision of the 16-inch HDD profile is 2,331 ft in horizontal length and passes under the crossings of two stream channels, and approximately 620 ft of an emergent and forested wetland complex. As part of the PADEP Chapter 105 permit process for the Mariner II East Project, SPLP developed and submitted for review a project-wide Alternatives Analysis. During the development and siting of the project, SPLP considered a number of different routings, locations, and designs to determine whether there was a practicable alternative to the proposed impact. SPLP performed this determination through a sequential review of routes and design techniques, which concluded with an alternative that has the least environmental impacts, taking into consideration cost, existing technology, and logistics. The baseline route provided for the pipeline construction was to cross every wetland and stream on the project by open cut construction procedures. The Alternatives Analysis submitted to PADEP conceptually analyzed the potential feasibility of any alternative to baseline route trenched resource crossings (e.g., reroute, conventional bore, HDD). The decision-making processes for selection of the HDD instead of an open cut crossing methodology is discussed thoroughly in the submitted alternatives analysis and was an important part of the overall PADEP approval of HDD plans as currently permitted. As described below, the open cut and re-route analyses have confirmed the conclusions reached in the previously submitted Alternatives Analysis.

Open-cut and Conventional Bore Analysis

Sunoco Pipeline, L.P. (SPLP) specifications require a minimum of 48 inches of cover over the installed pipelines. To meet these cover requirements, during construction through the stream and wetlands would require a minimum authorized open cut work space 75 ft in width to accommodate the 16 and 20-inch pipelines, allowing for each pipeline to be installed with sufficient separation for integrity management. The assessed area of impact by this open cut plan would directly affect approximately 0.09 acres of state water bottoms, 0.12 acres of emergent wetland, and 1.12 acres of forested wetland.

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Due to the existing saturated ground conditions, a significant volume of produced groundwater will fill all the excavations during the open cut process. These water volumes can be pumped to a discharge filtration structure; however, the current feasible filtration ability does not exceed 50 microns, therefore, cloudy water (from suspended fine clay and silt particles) would be discharged downstream regardless of all control methods employed for the entire duration of this open cut installation until completion.

The crossing distance of the emergent and forested wetlands, which are the most expansive natural features crossed by the HDDs, is beyond the technical limits of a conventional auger bore.

In summary, a combination of open-cut and conventional bores are not technically feasible alternatives to the Wetland J-47 HDD.

Re-Route Analysis

The pipeline route as currently permitted follows parallel to two existing Sunoco pipelines.

There are no existing utility corridors to the north or south that provide a practical alternative route. Any alternate route considered north or south of the existing utility corridor would require the clearing of a new "greenfield" corridor through existing woodlands and croplands, increase the number of stream crossings, and possibly encroach on additional private residences before it could rejoin the current route.

During the PADEP Chapter 105 permit process for the Mariner II East Project, SPLP created and submitted for review a project wide alternatives analysis. The baseline route provided for the pipeline construction to cross every wetland and stream on the project by open cut construction procedures. The alternatives analysis submitted to PADEP conceptually analyzed the feasibility of any alternative to trenched resource crossings (e.g., reroute, bore, HDD). The decision making processes for switching from an open cut to HDD is discussed thoroughly in the previously-submitted alternatives analysis and was an important part of the permit application package of HDD plans as currently permitted. The re-route analysis conducted for the Wetland J-47 HDD confirms the conclusions reached in the previously submitted alternatives analysis.

HORIZONTAL DIRECTIONAL DRILL REDESIGN

Additional geologic investigations were completed and utilized in the designs presented in the October Reevaluation Report for these HDDs. As a result of the events occurring during drilling and installation of the 20-inch pipeline, the 16-inch HDD was redesigned to approximate the profile of the field revised 20-inch pipeline. A summary of the redesign factors is provided below. The Original Reevaluation and Post 20-Inch Redesigned HDD plan and profiles are provided in Attachment 2.

REVISED (POST 20-INCH) HORIZONTAL DIRECTIONAL DRILL DESIGN SUMMARY: 16-INCH

- Horizontal length: 2,331 ft
- Entry/Exit angle: 16 degrees
- Maximum Depth of cover: 137 ft
- Change of depth at the 20-inch IR locations: 41-47 ft
- Pipe design radius: 2,000 ft

As shown on Figure 2 the 2nd revised HDD profile for the 16-inch pipeline is 131 longer, with a depth of cover at the IR locations increased by 41-47 ft, and designed for a sharp and quick entry and exit from the horizontal depth.

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CONCLUSION

HDD specialists revised the 16-inch diameter pipeline profile as set forth above to maximize the potential to complete the HDD without an occurrence of an IR.

Upon the start of the 16-inch HDD, SPLP will employ the following HDD best management practices:

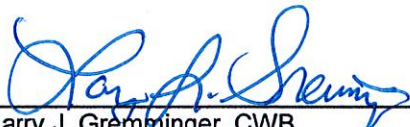
- SPLP will provide the drilling crew and company inspectors (UI, EI, PGs) the location(s) data on potential zones of higher risk for fluid loss and IRs, including the area related to previous IRs, and potential zones of fracture concentration identified by the fracture trace analysis so that monitoring can be enhanced when drilling through these locations.
- SPLP will require and enforce the use of annular pressure monitoring during the drilling of the pilot holes, which assists in immediate identification of pressure changes indicative of loss of return flows or over pressurization of the annulus to manage development of pressures that can induce an IR;
- SPLP inspectors will ensure that an appropriate diameter pilot tool, relative to the diameter of the drilling pipe, is used to ensure adequate "annulus spacing" around the drilling pipe exits to allow good return flows during the pilot drilling;
- SPLP will implement short-tripping of the reaming tools as return flow monitoring indicates to ensure an open annulus is maintained to manage the potential inducement of IRs;
- SPLP will require monitoring of the drilling fluid viscosity, such that fissures and fractures in the subsurface are sealed during the drilling process;
- During all drilling phases, the use of Loss Control Materials (LCMs) can be implemented to control Loss of Circulation of drilling fluids, or if indications of a potential IR are noted, or an IR is observed; and
- If LCMs prove ineffective to mitigate loss of returns or IRs, then grouting of the pilot hole may be implemented to control Loss of Circulation or IRs.

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FEASIBILITY DETERMINATION


Based on the information reviewed by the HDD Reevaluation Team, consisting of the Geotechnical Evaluation Leader, Professional Geologists, Professional Engineers, and HDD Specialists, it is the HDD Re-evaluation Team's professional opinion that the proposed HDD design and implementation of the management measures contained within this re-evaluation report will minimize the risk of IRs and impacts to public and private water supplies during the construction phases of the HDD.

Pertaining to Horizontal Directional Drilling Practices and Procedures; Conventional Construction; Alternatives; and Environmental Effects


Larry J. Gremminger, CWB
Geotechnical Evaluation Leader
Mariner East II Pipeline Project

1/22/2019
Date


Pertaining to the practice of geology as set forth in the attached Hydrogeologic Re-evaluation Report


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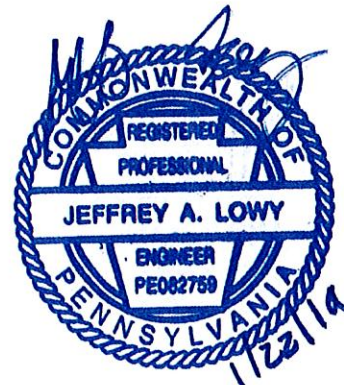
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Pertaining to Pipe Stress and HDD Geometry


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**ATTACHMENT 1
GEOLOGY AND HYDROGEOLOGICAL EVALUATION REPORT**

January 23, 2019

Mr. Matthew Gordon
Sunoco Pipeline, L.P.
535 Fritztown Road
Sinking Spring, PA 19608

Engineers

Environmental
Consultants

Surveyors

Landscape
Architects

Safety
Consultants

RE: Sunoco Pipeline, L.P. Pipeline Project - Mariner East II
Wetland J-47 Horizontal Directional Drill (HDD) S3-0090
Hydrogeological Re-Evaluation Report
Conewago Township, Dauphin County and
South Londonderry Township, Lebanon County, Pennsylvania
RETTEW Project No. 096302011

EXECUTIVE SUMMARY

1. During drilling of HDD S3-0090 (Wetland J-47) for installation of the 20-inch diameter pipeline, multiple inadvertent returns (IRs) were identified during the pilot phase. These IRs began 140 feet (ft) before the east HDD exit point.
2. HDD Wetland J-47 is underlain by sedimentary rocks of the Gettysburg Formation (Trg) and Gettysburg Formation-Conglomerate (Trgc).
3. Geologic mapping and published reports indicate typically open and moderate to steeply dipping beds, with regularly spaced bedrock joints and fractures.
4. Water-bearing zones generally occur in secondary openings along bedding planes, joints, faults and fractures. Water-bearing zones in both formations are most frequent within approximately 200 to 300 feet of the ground surface.
5. To date, the 30-inch ream and 20-inch pipe pullback are complete. Two IRs were observed during completion of the pilot phase of drilling for the 20-inch HDD at two locations along the trajectory length (project survey station STA 11,813+48 and STA 11,813+84). The initial IR at both locations occurred on December 20, 2017 and consisted of approximately 50 gallons of drilling fluid. A second IR occurred at both locations on December 29, 2017 and consisted of approximately 915 gallons of drilling fluid. No HDD operations have begun at the Wetland J-47 site for the proposed 16-inch pipeline.
6. Based on the hydro-structural characteristics of the underlying geology, and proposed HDD profile within shallow unconsolidated soil materials and generally shallow bedrock, the Wetland J-47 16-inch HDD is susceptible to the inadvertent return (IR) of drilling fluid during future HDD operations. The proposed 16-inch HDD profile has been redesigned to allow for a longer profile (2,331') and deeper crossing beneath Streams S-A47 and S-A48, and Wetland J-47. The inclination of the entry and exit angles has been increased to install the 16-inch pipe through protective soils and bedrock in closer proximity to the entry and exit points than the original shorter (1,070') and shallower profile. From a geologic perspective, the longer and deeper profile, in conjunction with the proposed engineering controls and/or drilling BMPs, will be used to reduce the risk of an IR.



1.0 INTRODUCTION

The purpose of this report is to describe the geologic and hydrogeologic setting of the S3-0090, Wetland J-47, HDD location (the site) on the Sunoco Pipeline, L.P. (SPLP) Pennsylvania Pipeline Project - Mariner East II (PPP-ME2) Project. The site is in Conewago Township, Dauphin County and South Londonderry Township, Lebanon County, Pennsylvania (refer to **Figure 1**). The HDD was designed to be drilled under two small unnamed tributary streams (S-A47 and A-S48) to Spring Creek and Wetland J-47.

The S3-0090, Wetland J-47 HDD, site is in the Gettysburg-Newark Lowland Section of the Piedmont Physiographic Province. The dominant topography of the Gettysburg-Newark Lowland Section is typified by rolling lowlands, shallow valleys and isolated hills of low to moderate relief. Underlying rock geology consists of red shale, siltstone and sandstone, with some conglomerate and diabase. The site is drained by two unnamed streams (S-A47 and S-A48) towards the northwest, across and over the HDD profile. Most of the area surrounding the site consists of woodlands. The HDD entry is located at the eastern end of the profile which is situated in an agricultural field.

The 16-inch HDD redesign was completed on May 17, 2018 and is included as **Attachment 1**. The redesign proposes the western entry/exit point at an elevation of approximately 525 feet above mean sea level (AMSL) and forms a slightly concave HDD profile that slopes gently upward towards the east at an elevation of approximately 563 feet AMSL at the eastern entry/exit point. The proposed 16-inch HDD crosses under unnamed stream S-A47 at two points in its profile from west to east, at depths of approximately 89 and 128 feet below ground surface (bgs), respectively. The 16-inch HDD also crosses under unnamed stream S-A48 at approximately 137 feet bgs, and under Wetland J-47 at depths ranging from approximately 50 to 137 bgs. The proposed 16-HDD is located between Pipeline Project Survey Station (STA) locations 11,792+94 and 11,815+25, for an overall horizontal length of 2,331 feet and a total pipeline length of 2,376 feet. The proposed S3-0090 HDD location is shown on **Figure 1**.

2.0 GEOLOGY AND SOILS

The site geology is mapped as the Gettysburg Formation and Gettysburg Formation-Conglomerate, as shown on **Figure 2** (Berg and Dodge, 1981). The Gettysburg Formation is described as red shale, red, brown and gray sandstone, and quartz and limestone conglomerate (Geyer and Wilshusen, 1982). The Gettysburg Formation-Conglomerate is described as coarse, quartz conglomerate containing rounded pebbles and cobbles in a matrix of red sand (Geyer and Wilshusen, 1982). The general structure of the Newark Group, which includes these formations, is a north-northwestward dipping homocline. Typical dip directions are north or northwest and range from 20° to 40° (Newport, 1971). Intrusive diabase has been mapped north and south of the HDD.

The Gettysburg Formation is moderately well bedded, thin to flaggy, moderately weathered and moderately resistant to weathering. Joint and bedding plane openings provide moderate secondary porosity. Permeability is described as moderate. Weathered rock is reported to be moderately easy to excavate whereas unweathered rock is reported to be difficult to excavate. Drilling rates are moderate to fast. Foundation stability is good when material is excavated to sound rock (Geyer and Wilshusen, 1982).

The Gettysburg Formation-Conglomerate is well bedded and thick to massive. Bedrock fracturing is described as jointed with a blocky pattern that is moderately developed and moderately abundant. The joints are regularly spaced with a moderate distance between fractures that are open and steeply dipping. The joint, bedding and fracture-plane openings provide a secondary porosity of low magnitude and low permeability. Overall, the formation is moderately resistant to weathering. These rocks reportedly provide good foundation stability (Geyer and Wilshusen, 1982).

According to the United States Department of Agriculture (USDA) Soil Surveys of Dauphin County and Lebanon County, Pennsylvania, soils within approximately 450 feet of the drill path for HDD S3-0090 consist of Basher silt loam (Bc); Brinkerton silt loam, 0 to 3 percent slopes (BrA); Brinkerton and Armagh silt loams, 0 to 3 percent slopes (BtA); Bucks silt loam, 3 to 8 percent slopes (ByB); Croton silt loam, occasionally ponded, 0 to 3 percent slopes (Cr); Lewisberry gravelly sandy loam, 3 to 8 percent slopes, moderately eroded (LrB2); Lewisberry gravelly sandy loam, 8 to 15 percent slopes, moderately eroded (LrC2); Lewisberry very stony sandy loam, 5 to 25 percent slopes (LsD); Lewisberry very stony sandy loam, 25 to 60 percent slopes (LsF); Penn channery silt loam, 3 to 8 percent slopes (PeB2); Penn channery silt loam, 8 to 15 percent slopes (PeC and PeC2); Ungers loam, 3 to 8 percent slopes (UnB); and Ungers loam, 8 to 15 percent slopes (UnC). A USDA map identifying the general area, along with the soil profile descriptions is included as **Attachment 2**.

3.0 HYDROGEOLOGY

Groundwater in the vicinity of the Wetland J-47 HDD moves in interconnected, secondary openings such as fractures and joints in the sedimentary bedrock aquifer system. Typically, these openings are best developed and found more frequently near the surface. At depth, these openings occur less frequently and tend to be smaller because compressional loading results in an increase of closed openings (Wood, 1980). Bedrock geology ultimately influences the storage, transmission, and use of groundwater. Geologic factors such as rock type, intergranular porosity, rock strata inclination, faults, joints, bedding planes, and solution channels affect groundwater movement and availability. Groundwater within the Gettysburg Formation and Gettysburg Formation-Conglomerate can occur under both unconfined (i.e., water table) and confined conditions. In general, groundwater generally occurs under unconfined conditions within the upper portion of the aquifer, and under confined or semiconfined conditions in the deeper portions of the aquifer. The groundwater flow system is described as a series of sedimentary beds with relatively high transmissivity separated by beds exhibiting lower transmissivities. This sequence of bedding exhibits different hydraulic properties that collectively act as a series of alternating aquifers and confining or semi-confining units forming a leaky multi-aquifer system (LMAS). The groundwater flow direction within the Gettysburg and Gettysburg Formation-Conglomerate is controlled by hydraulic gradients and the variability in hydraulic conductivity. The predominant flow direction is parallel to bedding (Wood, 1980).

Groundwater flow paths within the sedimentary rocks have both local and regional components. Locally, shallow groundwater discharges to the gaining portions of nearby streams and deeper regional groundwater flow is toward points of regional groundwater discharge such as the Susquehanna River. Groundwater divides may be different for each zone of groundwater flow, and therefore may not coincide with surface water divides. Based on our review of available reference sources, no regional water table mapping is available for the Wetland J-47 HDD or surrounding area. As a result, no water table mapping was available for review or inclusion with this HDD re-evaluation report.

According to Wood (1980), the depths of water-bearing zones from 332 wells in the Gettysburg Formation range from 5 to 900 feet bgs. Fifty percent of the 669 reported water-bearing zones were penetrated at a depth of 115 feet or less, with 90% of the water-bearing zones encountered at a depth of 288 feet bgs or less. The greatest density of water-bearing zones is from approximately 51 to 100 feet bgs. The density of water-bearing zones encountered at depths greater than 401 feet are based on five or fewer zones per 50-foot interval. Overall density of water-bearing zones in the conglomerate is 1.4 per 50-feet of well depth.

Well records reviewed within a 0.5-mile radius of the site were obtained from the Pennsylvania Groundwater Information System (PaGWIS) on January 22, 2019. Records and information from 26 wells within this radius were available and are summarized in the table below. These well locations are shown on **Figures 2 and 3**.

Well No.	Well Use	Casing Depth (feet)	Total Depth (feet)	Water Level (feet)	Yield (gallons per minute [gpm])
670848	WITHDRAWAL	60	275	Not Available	4
86781	WITHDRAWAL	82	125	Not Available	20
86779	WITHDRAWAL	61	125	Not Available	30
637244	WITHDRAWAL	61	160	35	40
541860	WITHDRAWAL	99	225	26	8
86665	WITHDRAWAL	48	100	Not Available	12
258993	Not Available	60	100	30	40
617996	WITHDRAWAL	258	380	200	20
491472	CLOSED-LOOP GEOTHERMAL	60	375	Not Available	Not Available
490905	CLOSED-LOOP GEOTHERMAL	80	375	Not Available	Not Available
17655	WITHDRAWAL	69	100	19	30
17657	WITHDRAWAL	39	140	35	30
86348	WITHDRAWAL	39	140	35	30
86254	WITHDRAWAL	79	150	24	25
666532	WITHDRAWAL	160	225	Not Available	60
17651	WITHDRAWAL	36	160	71	15
534520	WITHDRAWAL	120	250	Not Available	7
86346	WITHDRAWAL	43	100	Not Available	20
415899	Not Available	95	150	Not Available	20
625206	WITHDRAWAL	84	140	38	50

Well No.	Well Use	Casing Depth (feet)	Total Depth (feet)	Water Level (feet)	Yield (gallons per minute [gpm])
669744	WITHDRAWAL	102	275	Not Available	80
127370	WITHDRAWAL	64	100	Not Available	25
667669	WITHDRAWAL	99	450	Not Available	15
514041	CLOSED-LOOP GEOTHERMAL	21	300	Not Available	30
514040	CLOSED-LOOP GEOTHERMAL	21	300	Not Available	100
22818	WITHDRAWAL	63	120	35	10

As a condition of the corrected Stipulated Order, other Sunoco subcontractors researched private water supplies within 450 feet of the Wetland J-47 HDD in January 2019. One well was identified within the 450-foot buffer of the alignment that was not reported in the PaGWIS database. Information regarding depth to bedrock, depth to water, and pump depth were not documented. A map of the 450-foot buffer is included as **Attachment 3**.

4.0 FRACTURE TRACE ANALYSIS

Fracture traces underlying, or in proximity to, the site were evaluated using historical aerial photographs from the years 1992 through 2016 (Google Earth, 2019), the Elizabethtown and Palmyra Quadrangle Geologic Maps (Berg and Dodge, 1981), Plate 1-Part 2 in Wood (1980), and United States Geological Survey (USGS) 7.5-Minute Topographic Quadrangle Maps. The photographs, publications and maps were reviewed to approximate locations of lineaments or natural linear features on the ground surface. The linear features may be the surficial representation of deeper fractures, joints, faults or bedding planes within the subsurface which can transmit groundwater in the fractured bedrock aquifer at the site.

Figures 2 and 3 show the results of the fracture trace analysis overlain on the geologic map of the site and an aerial base map. Five fracture traces were identified in proximity to the Wetland J-47 HDD that are likely related to the primary geologic structure. Two of the fracture traces trend approximately northwest-southeast, similar to the general structure of the regional homocline. The three perpendicular fracture traces trend approximately northeast-southwest and may represent stress-related joints.

5.0 GEOTECHNICAL EVALUATION

Two geotechnical drilling evaluations were performed at the site; one was performed in 2014 and the other in 2017. Test borings were advanced by hollow-stem auger drilling methods. An NQ core barrel/bit was used for rock coring. Geotechnical boring logs are included in **Attachment 1**. The locations of the borings are depicted on **Figure 2** and **Figure 3**.

The first geotechnical drilling program was performed on November 18, 2014, prior to the initiation of HDD operations. Soil Boring 01 (SB-01) was located approximately 45 feet north of the approximate mid-point of the bore path on the north side of Stream A-47 and Wetland J-47. Soil Boring 02 (SB-02) was located approximately 400 feet west of the eastern entry point and 75 feet north of the bore path. Soil

Boring 03 (SB-03) was located approximately 400 feet east of the eastern entry point and 210 feet south of bore path.

Two additional borings were advanced between August 28 and September 7, 2017, prior to the initiation of HDD operations. Boring B-1 was installed near the HDD exit point on the west side of the HDD profile. Boring B-2 was installed near the HDD entry on the east side of the profile.

The subsurface profile at the site, as observed in the borings, is described below:

- Soil and residual soil depths vary from west to east; 6.3 feet at B-1, 21.5 feet at SB-01, 32.5 feet at SB-02, 9.4 feet at B-2, and 30 feet at SB-03. The residual soils are described as follows:
 - **Boring B-1:** Sandy lean CLAY (CL) and poorly graded SAND with gravel (SP) (weathered sandstone)
 - **Boring SB-01:** SILT with sand (ML) and fine SAND with silt and gravel (sandstone)
 - **Boring SB-02:** Fine to medium SAND with silt and gravel (SM); Silty CLAY with sand (CL); Fine SAND with clay (SC); Silty CLAY with sand (CL); and weathered sandstone
 - **Boring B-2:** Sandy SILT (ML) and poorly graded SAND with silt (SP-SM) (weathered sandstone)
 - **Boring SB-03:** Silty CLAY with sand (CL); Fine to medium SAND with clay (SC); Silty CLAY with sand (CL-claystone); and weathered sandstone.
- Refusal, defined as naturally occurring rock that cannot be penetrated by standard soil sampling methods consisting of split-spoon samplers and augers, was encountered at 6.3 feet in B-1, 21.5 feet in SB-01, 32.5 feet at SB-02, 9.4 feet in B-2.
- Beneath auger refusal to the total depth of the NQ cores, bedrock was encountered and is described as follows:
 - **Boring B-1:** B-1 was completed to a total depth of 159 feet. Alternating sequences of grayish to reddish brown, conglomeratic to coarse- to fine-grained, SANDSTONE and reddish-brown SILTSTONE were encountered. Two layers of CONGLOMERATE were encountered (at 40.0 to 45.6 feet and 95.3 to 96.7 feet). A total of 21 distinct strata composed of sandstone, siltstone, and conglomerate bedrock were identified, with thicknesses ranging from 1.5 feet to 34.5 feet. Rock recoveries were generally excellent (100%) in the majority of the core runs. One core run had poor core recovery (44%) from 84.0 to 87.4 feet in a highly weathered siltstone/sandstone zone. RQDs were very poor to excellent (0 to 99%), and in general the lower RQD values correlated to the siltstone layers and the higher RQD values correlate to the sandstone/conglomerate layers. Multiple soil seam and highly weathered rock were encountered in the upper 24 feet of the boring. Fractures ranging from generally horizontal to high angle were recorded in the core logs.
 - **Boring B-2:** B-2 was completed to a total depth of 201 feet. Alternating sequences of grayish to reddish brown, conglomeratic and coarse- to fine-grained SANDSTONE and reddish-brown SILTSTONE were encountered. Two layers of SHALE were encountered (at 84.8 to 102 feet and 153.4 to 170.8 feet). A total of 18 distinct strata of sandstone, siltstone, and shale bedrock were identified, with thicknesses ranging from 2.0 feet to 28.0 feet. Rock recoveries were generally excellent (100%) in the majority of the core runs. RQDs were poor to excellent (40

to 100%) below a depth of 20 feet. Fractures ranging from generally horizontal to high angle were recorded on the core logs.

Please note that Skelly and Loy and RETTEW did not oversee or direct the geotechnical drilling program associated with the Wetland J-47 HDD, including but not limited to, the selection of boring locations and target depths, observations of rock cores during drilling operations, or preparation of boring logs. The geotechnical reports, boring logs, and core photographs that resulted from these programs were generated by other Sunoco Pipeline, L.P. contractors. Skelly and Loy and RETTEW relied on these reports and incorporated the data into the general geologic and hydrogeologic framework included in this report.

6.0 FIELD OBSERVATIONS

A field investigation was performed by a RETTEW geologist on September 18, 2017 to identify rock outcrops for fracture fabric analysis, evaluation and possible ground-truthing of fracture traces identified during the desktop evaluation, and to identify potential sensitive receptors to IRs. Readily accessible bedrock outcrops were not observed. No additional sensitive receptors to IRs were identified during the site reconnaissance.

On December 5, 2017 Laney Directional Drilling (Laney) began drilling the pilot hole for the 20-inch pipeline from the west side. On December 7, 2017 Laney reported a loss of returns (LOR) of 500-1,000 gallons when the drill bit was approximately 1,876 feet from the east side (approximately 360 feet east of the west side). No IRs were observed, and drilling was resumed after a two-hour delay with approval of Laney's construction and project managers and the Environmental Inspector (EI). On December 20, 2017 an IR occurred when the pilot bit was located approximately 2,087 feet from the east entry pit (approximately 149 feet from the west side) and approximately 23 feet bgs. An estimated 50 gallons of drilling fluid was released to the ground surface. Laney immediately ceased drilling and contained the IR with sand bags and silt fencing and cleaned up the release. Drilling activities ceased pending restart approval from the Pennsylvania Department of Environmental Protection (PA DEP). A determination was made that the IR had impacted the eastern end of Wetland A-30 and Stream K-23. Upon restart approval by the PA DEP, Laney resumed pilot drilling on December 29, 2017 with approximately 149 feet of drilling remaining to complete the pilot borehole. On December 29, 2017, an IR occurred at the same location as the December 20, 2017 IR, within the previously constructed containment area. An estimated 900 gallons of drilling fluid was released to the ground surface and was cleaned up immediately by Laney using a vacuum truck. Approximately 3 gallons of drilling fluid seeped through the silt fence and flowed approximately 18 feet downslope but was subsequently cleaned up by Laney. Drilling ceased pending PA DEP restart approval. Following restart approval, on April 19, 2018, Laney moved the rig to the east side and spud in from the east side to intercept the section of pilot bore completed to 2,087 feet from the west. Laney completed the pilot hole on April 26, 2018. There were no subsequent LOR or IRs that occurred during reaming activities. The 20-inch pipe pull was completed on July 21, 2018.

7.0 CONCEPTUAL HYDROGEOLOGIC MODEL AND CONCLUSION

Based on published geologic and hydrogeologic information, and the evaluation of geotechnical borings from the site, the Wetland J-47 HDD location is underlain by sedimentary rocks of the Gettysburg Formation and Gettysburg Formation-Conglomerate. The hydrogeologic setting is dominated by groundwater flow in secondary openings along geologic features that include bedding planes, fractures, and joints. In these formations, secondary openings are more common near the surface. Well records

indicate 90% of the water-bearing zones in the Gettysburg Formation are within 280 feet of the surface. Geotechnical core observations indicate that bedrock near the center of the HDD profiles is fractured.

The proposed 16-inch HDD profile is relatively shallow compared to the land surface, streams S-A47 and S-A48, and Wetland J-47, and passes through both unconsolidated overburden and fractured bedrock. The weakest point of the profile is beneath the first crossing at Stream S-A47. Based on the hydro-structural characteristics of the underlying geology described in this report, and the known 20-inch HDD profile through shallow soils and bedrock, the proposed 16-inch HDD at the Wetland J-47 site is susceptible to the inadvertent return of drilling fluids during HDD operations. The HDD profile has been lengthened to allow for deeper crossings beneath the streams and wetland. The inclination of the entry and exit angles has been increased to install the pipe through the protective soils and bedrock, and in closer proximity to the entry and exit points than the original, shorter profile. From a geologic perspective, the longer and deeper profile, in conjunction with the proposed engineering controls and/or drilling best management practices will be used to reduce the risk of an IR.

8.0 REFERENCES

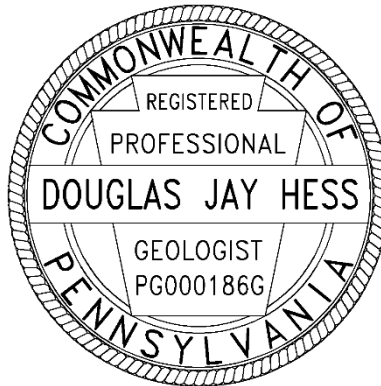
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- United States Department of Agriculture, 2017, Natural Resources Conservation Service, Published Soil Surveys for Pennsylvania, Dauphin and Lebanon Counties, Pennsylvania: website address: <https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx> accessed September 21, 2017.
- Wood, C. R., 1980, Groundwater Resources of the Gettysburg and Hammer Creek Formations, Southeastern, Pennsylvania, Pennsylvania Geologic Survey, 4th Series, Water Resources Report 49, 87 pages.

9.0 CERTIFICATION

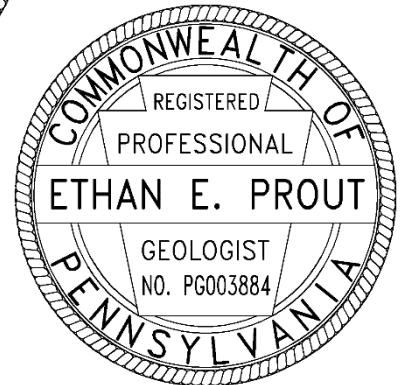
The studies and evaluations presented in this report (other than Section 5.0) were completed under the direction of a licensed professional geologist (PG) and are covered under the PG seals that follow.

By affixing my seal to this document, I am certifying that, to my knowledge and belief, the information herein is true and correct. I further certify, that I am licensed to practice in the Commonwealth of Pennsylvania and that it is within my professional expertise to verify the correctness of the information herein.

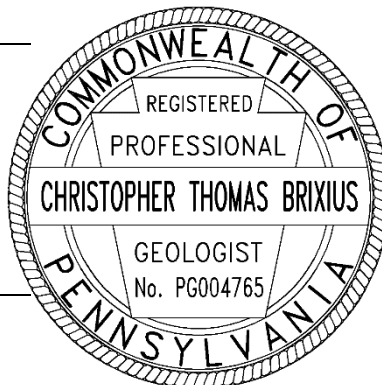
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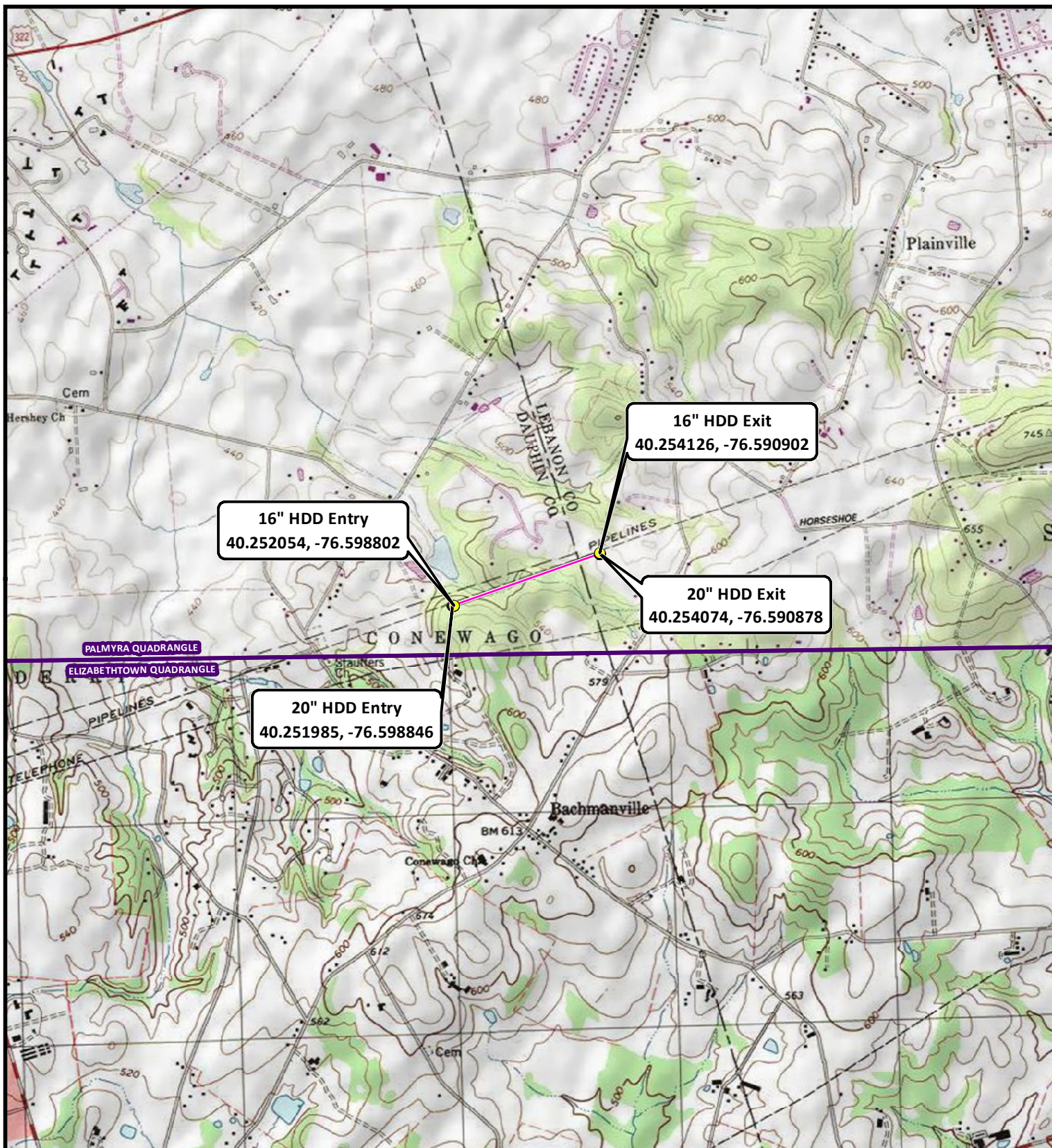
Ethan E. Prout, PG
License No. PG003884



Christopher T. Brixius, PG
License No. PG004765



FIGURES



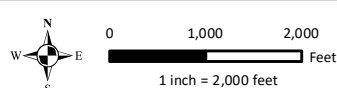
- 16" HDD Entry/Exit
- 20" HDD Entry/Exit
- 16" HDD Profile
- 20" HDD Profile

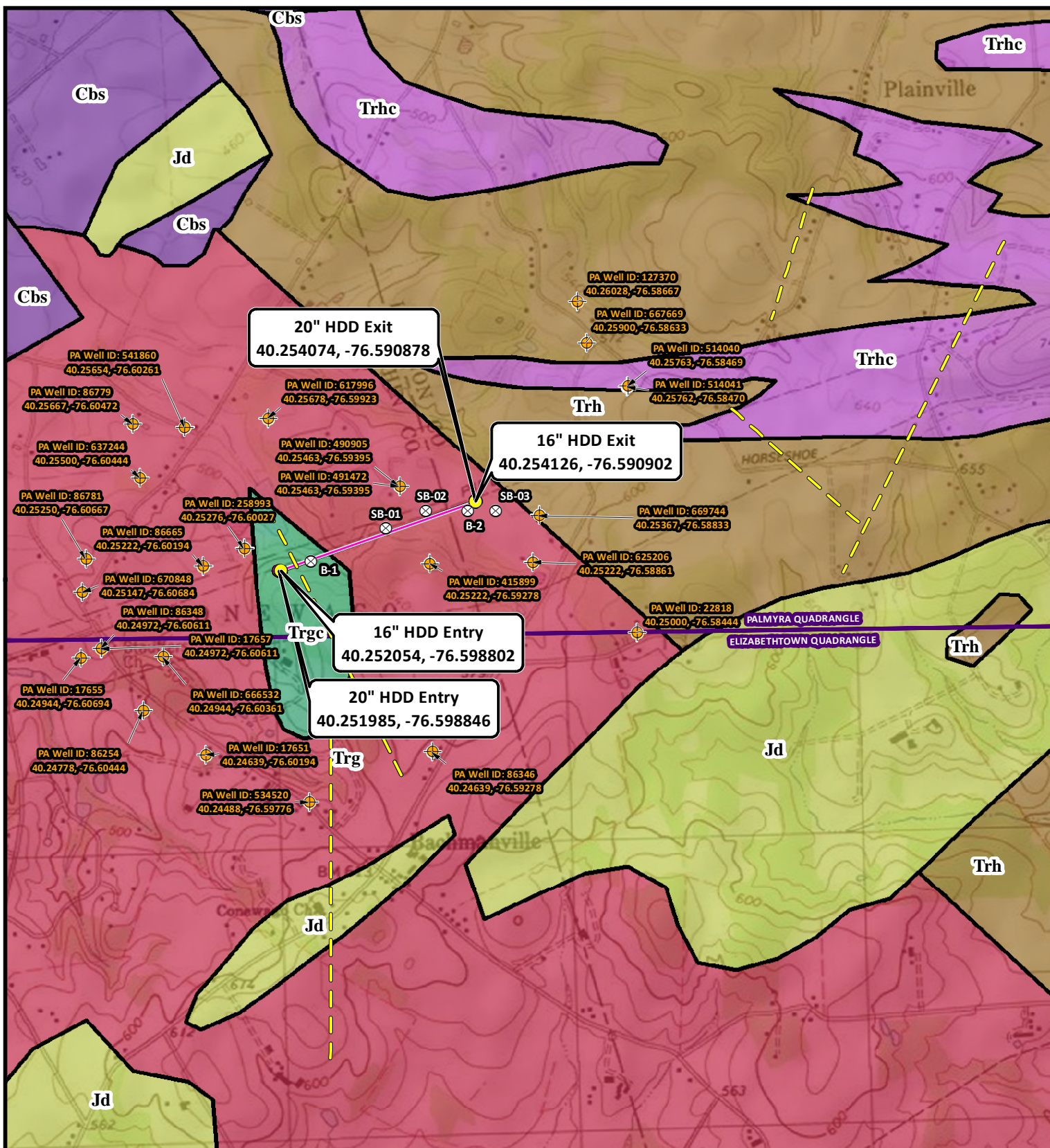
Sunoco Pipeline, L.P.

Wetland J-47 HDD Location

Figure 1 - Topographic Basemap

Conewago Twp., Dauphin County, & South Londonderry Twp., Lebanon County, PA
Project No. 096302011





- Residential Well
- Boring Location
- 16" HDD Entry/Exit
- 20" HDD Entry/Exit
- 16" HDD Profile
- 20" HDD Profile
- Inferred Fracture Trace

- Geologic Formation**
- Cbs - Buffalo Springs Formation
 - Jd - Diabase
 - Trg - Gettysburg Formation
 - Trgc - Gettysburg conglomerate
 - Trh - Hammer Creek Formation
 - Trhc - Hammer Creek conglomerate

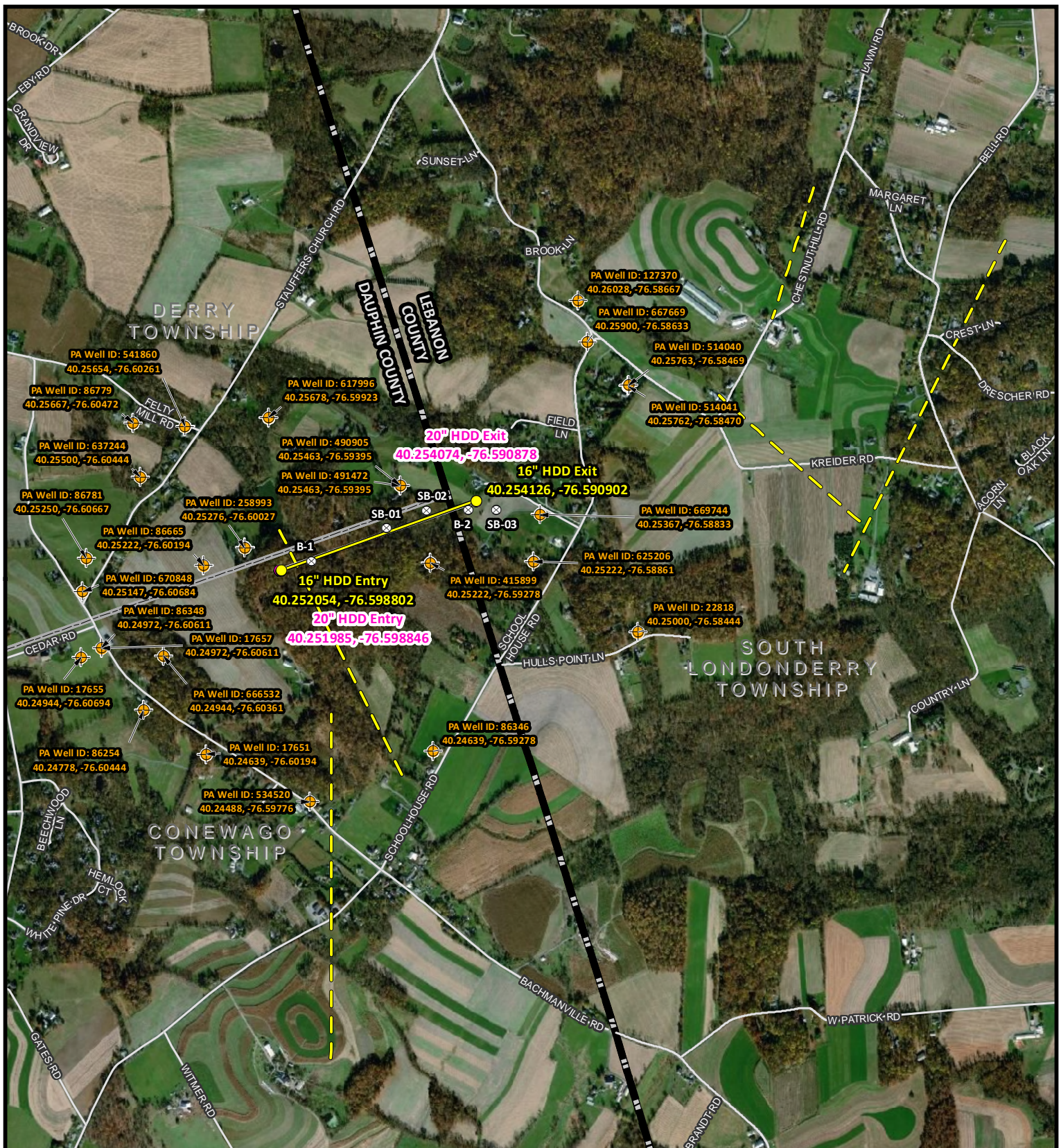
Sunoco Pipeline, L.P.

Wetland J-47 HDD Location

Figure 2 - Geologic Map

Conewago Twp., Dauphin County, & South Londonderry Twp., Lebanon County, PA
Project No. 096302011





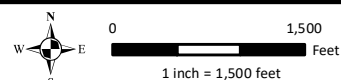
- | | | | |
|--|--------------------|--|-------------------------|
| | Residential Well | | 20" HDD Profile |
| | Boring Location | | Inferred Fracture Trace |
| | 16" HDD Entry/Exit | | Road |
| | 20" HDD Entry/Exit | | Municipal Boundary |
| | 16" HDD Profile | | County Boundary |

1/23/2019

Sunoco Pipeline, L.P. Wetland J-47 HDD Location

Figure 3 - Aerial Basemap

Conewago Twp., Dauphin County, & South Londonderry Twp., Lebanon County, PA
Project No. 096302011

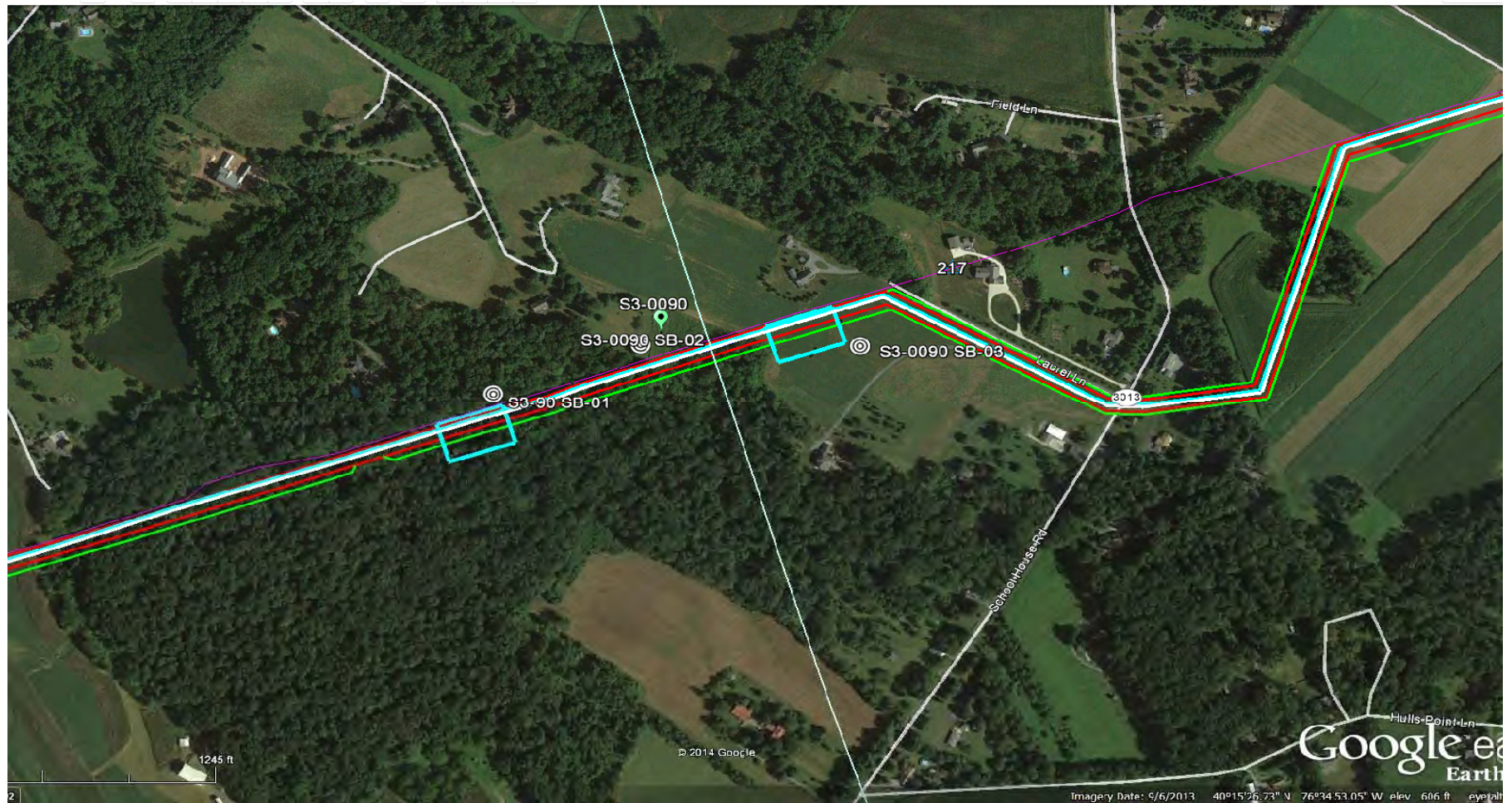


Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community





ATTACHMENT 1
GEOTECHNICAL BORING LOGS



LEGEND:

- ⊙ Geotechnical Soil Boring (SB) Locations

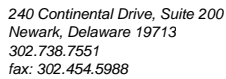


TETRA TECH

GEOTECHNICAL BORING LOCATIONS

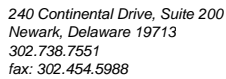
HDD S3-0090

LEBANON COUNTY, SOUTH LONDONDERRY TOWNSHIP &
DAUGHIN COUNTY, CONEWAGO TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT

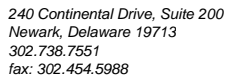


Project Name:	SUNOCO PENNSYLVANIA PIPELINE PROJECT			Project No.: 103IP3406
Project Location:	95 LAUREL LANE, PALMYRA, PA			Page 1 of 1
HDD No.:	S3-0090	Dates(s) Drilled: 11-18-14	Inspector:	E. WATT
Boring No.:	SB-01	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	21.5
Boring Location Coordinates:	40° 15' 11.995" N		76° 35' 40.296" W	

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.
N: Number of blows to drive spoon from 6" to 18" interval.



N: Number of blows to drive spoon from 6" to 18" interval.



Project Name:	SUNOCO PENNSYLVANIA PIPELINE PROJECT			Project No.: 103IP3406
Project Location:	95 LAUREL LANE, PALMYRA, PA			Page 1 of 1
HDD No.:	S3-0090	Dates(s) Drilled: 11-18-14	Inspector:	E. WATT
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0
Boring Location Coordinates:	40° 15' 13.813" N		76° 35' 24.341" W	

[illegible]

GEOTECHNICAL LABORATORY TESTING SUMMARY
SUNOCO PENNSYLVANIA PIPELINE PROJECT
HDD S3-0090

HDD No.	Test Boring No.	Sample No.	Depth of Sample (ft.)		Water Content, % (ASTM D2216)	Percent Silts/Clays, % (ASTM D1140)	Atterburg Limits (ASTM D4318)			USCS Classif. (ASTM D2487)
			From	To			Liquid Limit, %	Plastic Limit, %	Plasticity Index, %	
S3-0090	SB-01	1	3.0	5.0	6.4	41.2	-	-	-	-
		2	8.0	8.7	3.6	39.2	-	-	-	-
		4	18.0	19.2	5.7	39.8	-	-	-	-
		5	20.0	20.3	6.3	41.9	-	-	-	-
	SB-02	1	3.0	5.0	9.2	21.9	-	-	-	-
		2	8.0	10.0	10.4	80.8	30	19	11	CL
		3	13.0	15.0	12.5	75.1	-	-	-	-
		4	18.0	20.0	14.2	16.3	-	-	-	-
		5	23.0	24.4	10.5	90.7	-	-	-	-
		6	28.0	28.5	5.7	21.4	-	-	-	-
	SB-03	1	3.0	5.0	14.2	99.2	32	19	13	CL
		2	8.0	10.0	9.2	21.1	-	-	-	-
		4	18.0	20.0	13.7	38.8	-	-	-	-
		5	23.0	24.4	9.6	75.3	-	-	-	-

Notes:

- 1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY
SUNOCO PENNSYLVANIA PIPELINE PROJECT
HDD S3-0090

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S3-0090	Wetland J47	SB-01	Gettysburg Fm - reddish-brown to maroon silty mudstone and shale and soft, red-brown, medium- to fine-grained sandstone, with minor amounts of yellowish-brown shale and sandstone and thin beds of impure limestone.	Gently sloping lowland to forested wetlands	Gettysburg Fm	Silty mudstone-shale-sandstone w/ some impure limestone		12-22	
		SB-02							
		SB-03							

Note : Source of well log data - <http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm>. All other sources as referenced in comments section.

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	<u>N (blows)*</u>
Very Loose	5 or less
Loose	6 to 10
Medium Dense	11 to 30
Dense	31 to 50
Very Dense	51 or more

Relative Proportions

<u>Description Term</u>	<u>Percent</u>
Trace	1 - 10
Little	11 - 20
Some	21 - 35
And	36 - 50

Particle Size Identification

Boulders	8 in. diameter or more
Cobbles	3 to 8 in. diameter
Gravel	Coarse (C) 3 in. to ¾ in. sieve Fine (F) ¾ in. to No. 4 sieve
Sand	Coarse (C) No. 4 to No. 10 sieve (4.75mm-2.00mm) Medium (M) No. 10 to No. 40 sieve (2.00mm – 0.425mm) Fine (F) No. 40 to No. 200 sieve (0.425 – 0.074mm)
Silt/Clay	Less Than a No. 200 sieve (<0.074mm)

COHESIVE SOILS

(Silt, Clay & Combinations)

<u>Consistency</u>	<u>N (blows)*</u>
Very Soft	3 or less
Soft	4 to 5
Medium Stiff	6 to 10
Stiff	11 to 15
Very Stiff	16 to 30
Hard	31 or more

Plasticity

<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
None to Slight	0 - 4
Slight	5 - 7
Medium	8 - 22
High to Very High	> 22

ROCK

(Rock Cores)

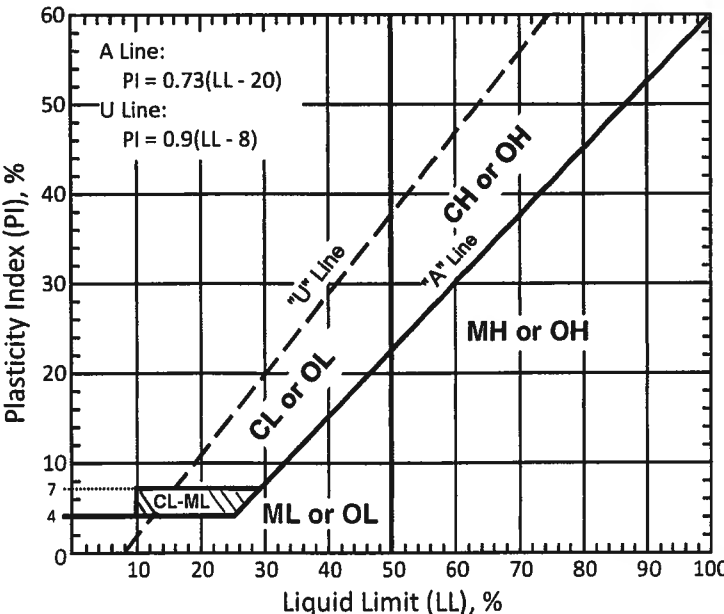
<u>Rock Quality Designation (RQD), %</u>	<u>Rock Quality Description</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

***N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions			Group Symbols	Typical Descriptions	Laboratory Classifications			
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM, GC, SM, SC 5 to 12 percent Borderline cases requiring dual symbols ⁽¹⁾	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
			GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		Not meeting C_u or C_c requirements for GW		
		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures		Atterberg limits below A Line or I_p less than 4	Limits plotting in hatched zone with I_p between 4 and 7 are borderline cases requiring use of dual symbols	
			GC	Clayey gravels, gravel-sand-clay mixtures		Atterberg limits above A line with I_p greater than 7		
	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SW	Well graded sands, gravelly sands, little or no fines		$C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
			SP	Poorly graded sands, gravelly sands, little or no fines		Not meeting C_u or C_c requirements for SW		
		Sands with fines (Appreciable amount of fines)	SM	Silty sands, sand-silt mixtures		Atterberg limits below A Line or I_p less than 4	Limits Plotting in hatched zone with I_p between 4 and 7 are borderline cases requiring use of dual symbols	
			SC	Clayey sands, sand-clay mixtures		Atterberg limits above A line with I_p greater than 7		

Major Divisions		Group Symbols	Typical Descriptions	<div>For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$, $w_L = 60$ gives CH-MH. When w_L is near 50 use CL-CH or ML-MH. Take near as ± 2 percent.</div> <div></div>
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Sils and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	
		OL	Organic silts and organic silty clays of low plasticity	
	Sils and Clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	
		CH	Inorganic clays of high plasticity, fat clays	
		OH	Organic clays of medium to high plasticity, organic silts	
	Highly organic soils	Pt	Peat and other highly organic soils	

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

Figure 1: Site Vicinity Map

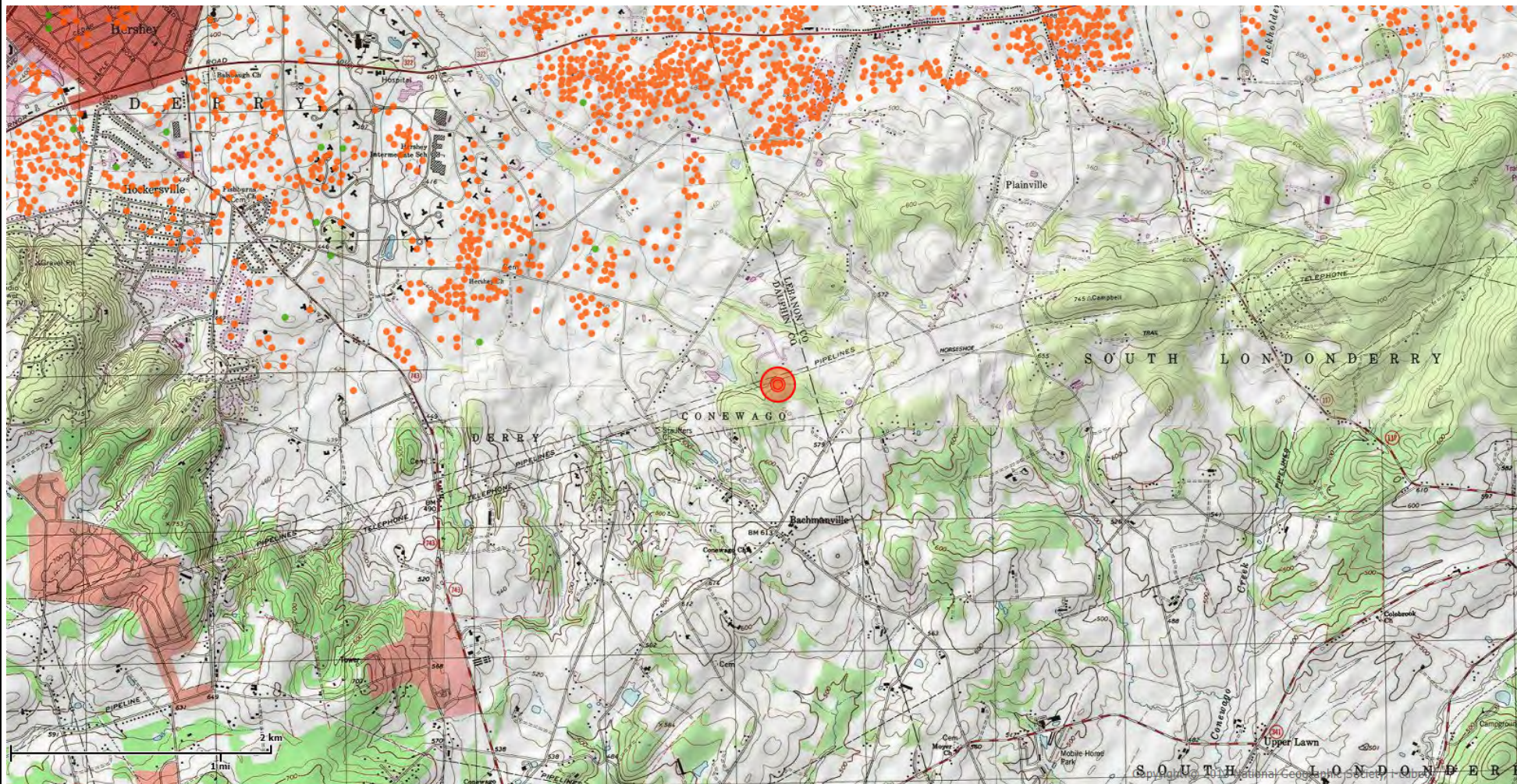
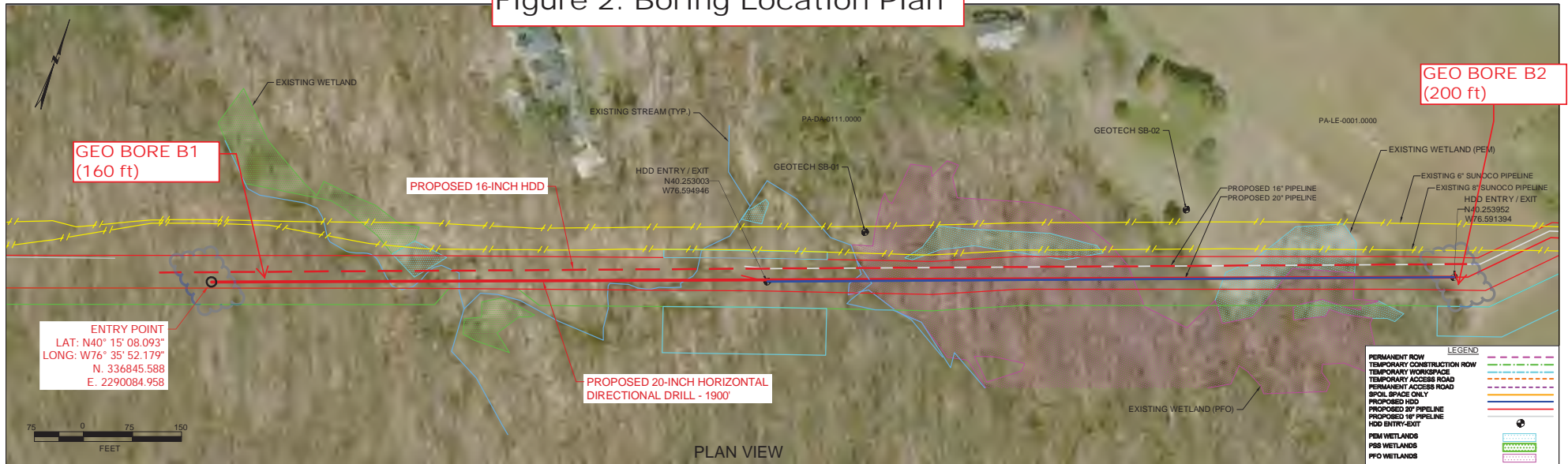
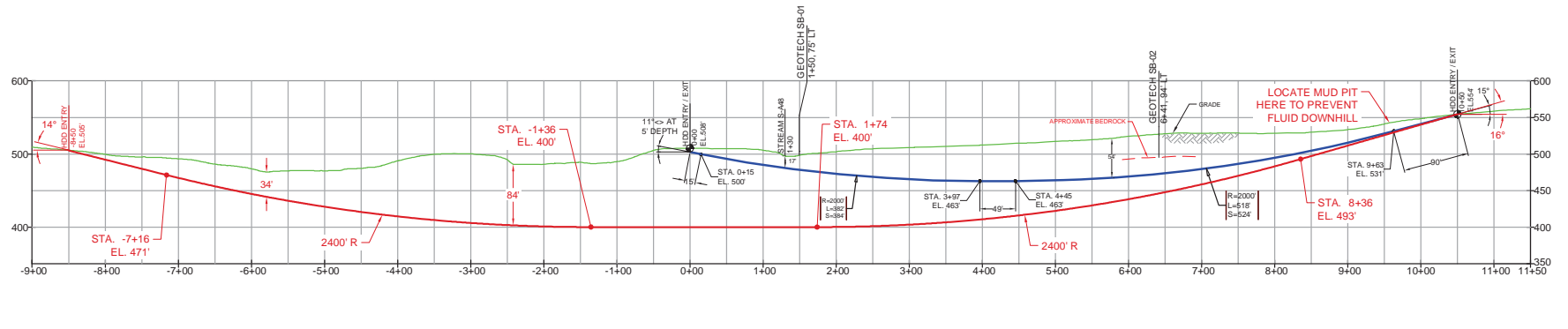
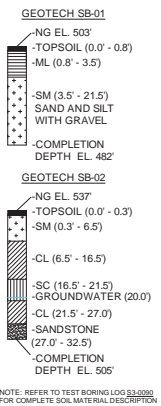


Figure 2: Boring Location Plan



PLAN VIEW

PROFILE VIEW



DESIGN AND CONSTRUCTION

- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
- THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
- DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- CROSSING PIPE SPECIFICATION:
HDD HORZ. LENGTH (L) = 1900'
HDD PIPE LENGTH (S) = 1902'
20" x 0.486" W.T., X-65, API 5L, PSL2, ERW, BFW
COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWDERCURE R95)

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
- INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
- PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
- CARRIER PIPE NOT ENCASED.
- PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
- CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1800 PSIG.
- SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESR WEBSITE FOR ACCESS ROAD ALIGNMENT.

- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
- SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.
- THE MINIMUM ALLOWABLE THREE JOINT RADIUS SHALL NOT BE LESS THAN 1800 FEET.

NOTES

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS 1-800-786-7440.

REVISIONS

NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE
7	LANEY DESIGN CHANGE	KBP	07/25/17				
6	LANEY DESIGN CHANGE	KBP	07/21/17				
5	LANEY DESIGN CHANGE	KBP	06/27/17				
4	DESIGN CHANGE - MOVED HDD ENTRY / EXIT	MRS	02/21/17	RMB	02/21/17	AMC	02/21/17
3	REVISED PROFILE WITH 2017 LIDAR	MRS	02/15/17	RMB	02/15/17	AAW	02/15/17
2	REVISED PER ENGINEERING COMMENTS	MRS	08/19/16	RMB	08/19/16	AAW	08/19/16



ISSUED FOR APPROVAL

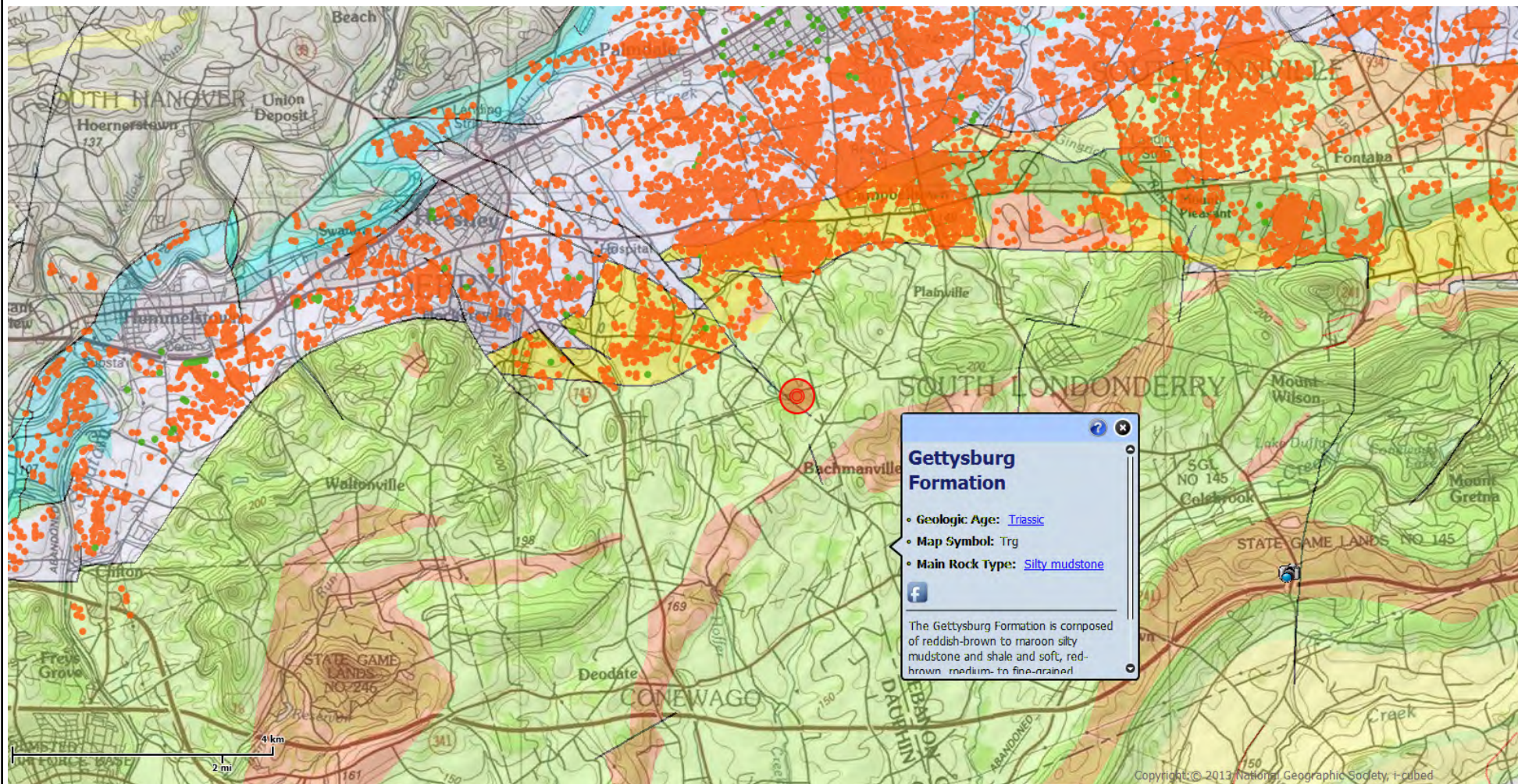
SUNOCO PIPELINE, L.P.




HORIZONTAL DIRECTIONAL DRILL
WETLAND
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=150' DWG. NO: PA-LE-0001.0000-SR

Figure 3: Site Geology Map

Visit us at <http://www.dcnr.state.pa.us>



BORING B-1			
Water		While Drilling	Not Enc.
		Post-Core	17 feet
			

BORING LOCATION:
 See Boring Location Plan

[illegible]

DATE STARTED: 8/28/17		DRILL COMPANY: Eichelberger's, Inc.		BORING B-1	
DATE COMPLETED: 8/31/17		DRILLER: S. Taylor LOGGED BY: L. Proczko			
COMPLETION DEPTH: 159.0 ft		DRILL RIG: Diedrich D-50		<div style="display: flex; justify-content: space-between;"> <div style="width: 30%;"> Water <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> While Drilling </div> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> Post-Core </div> <div style="width: 30%;"> Not Enc. 17 feet </div> </div>	
BENCHMARK: N/A		DRILLING METHOD: Casing/Rock Coring			
ELEVATION: N/A		SAMPLING METHOD: 2-in SS1.874-in Core		BORING LOCATION: See Boring Location Plan	
LATITUDE: n/a°		HAMMER TYPE: Automatic			
LONGITUDE: n/a°		EFFICIENCY: N/A			
STATION: N/A OFFSET: N/A		REVIEWED BY: F. Hoffman			
REMARKS:					




Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										<div> <div> <div>×</div>Moisture <div>■</div>PL <div>+</div>LL </div> <div> <div>▲</div>Qu <div>✱</div>Qp </div> </div>	
30				R-6	48	SILTSTONE -Brown to red-brown, Very fine grained, Weathered to Highly Weathered, very broken to slightly broken, moderately hard		RQD=0 Rec=80%			
35				R-7	60	Conglomeratic SANDSTONE -Gray-brown to dark gray-brown, Fine to very coarse grained, Weathered to Slightly Weathered, very broken to massive, hard to very hard		RQD=59 Rec=100%			17 min.
40				R-8	60	CONGLOMERATE -Light gray-brown to gray-brown, Fine to very coarse grained, Weathered to Slightly Weathered, very broken to massive, hard to very hard, trace pits and vugs		RQD=70 Rec=100%			>>▲ Q _u = 425.1 tsf 155.7 pcf 14 min.
45				R-9	48	Conglomeratic SANDSTONE -Dark gray-brown, Fine to coarse grained, Weathered to Highly Weathered, very broken to massive, very hard		RQD=26 Rec=100%			>>▲ Q _u = 379.0 tsf 154.5 pcf 9 min.
				R-10	12	Conglomeratic SANDSTONE -Gray to gray-brown, Fine to very coarse grained, Slightly Weathered, slightly broken to massive, hard to very hard		RQD=88 Rec=100%			4 min. 2 min. 2 min. 3 min. 4 min. 3 min.
50				R-11	60	SANDSTONE -Light gray to dark gray-brown, Fine to coarse grained, Slightly Weathered, very broken to massive, hard to very hard, trace pits		RQD=98 Rec=100%			3 min. >>▲ Q _u = 283.6 tsf 149.5 pcf 3 min.
55				R-12	60			RQD=77 Rec=100%			4 min. >>▲ Q _u = 441.1 tsf 149.0 pcf 3 min.
60											3 min. 3 min. 3 min. 3 min.

Continued Next Page



Professional Service Industries, Inc.
1707 S. Cameron Street, Suite B
Harrisburg, PA 17104
Telephone: (717) 230-8622

PROJECT NO.: 04911456
PROJECT: Energy Transfer HDD (DPS)
LOCATION: Wetland "J47" (PPP5)
Dauphin Co., PA
PA-LE-0001.0000-SR/PO#201770816-2




Water		While Drilling	Not Enc.
		Post-Core	17 feet
			

BORING LOCATION:
See Boring Location Plan

Continued Next Page

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PROJECT NO.: 04911456
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LOCATION: Wetland "J47" (PPP5)
Dauphin Co., PA
PA-LE-0001.0000-SR/PO#201770816-2

Water		While Drilling	Not Enc.
		Post-Core	17 feet
			

BORING LOCATION:
See Boring Location Plan

STANDARD PENETRATION TEST DATA				Additional Remarks
N in blows/ft ©				
×	Moisture	■	PL	
		■	LL	
0	25	50		
STRENGTH, tsf				
▲	Qu	✱	Qp	
0	2.0	4.0		

DATE STARTED: 8/28/17		DRILL COMPANY: Eichelberger's, Inc.		BORING B-1	
DATE COMPLETED: 8/31/17		DRILLER: S. Taylor LOGGED BY: L. Proczko			
COMPLETION DEPTH: 159.0 ft		DRILL RIG: Diedrich D-50		<div style="display: flex; justify-content: space-between;"> <div style="width: 30%;"> Water <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> While Drilling </div> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> Post-Core </div> <div style="width: 30%;"> Not Enc. 17 feet </div> </div>	
BENCHMARK: N/A		DRILLING METHOD: Casing/Rock Coring			
ELEVATION: N/A		SAMPLING METHOD: 2-in SS1.874-in Core		BORING LOCATION: See Boring Location Plan	
LATITUDE: n/a°		HAMMER TYPE: Automatic			
LONGITUDE: n/a°		EFFICIENCY: N/A			
STATION: N/A OFFSET: N/A		REVIEWED BY: F. Hoffman			
REMARKS:					

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @				Additional Remarks
										X Moisture PL LL				
										STRENGTH, tsf				
										▲ Qu	✱	Qp		
										0	2.0	4.0		
	120			R-29	60	SANDSTONE -Gray-brown to red-gray-brown, Fine to coarse grained, Slightly Weathered, very broken to massive, hard		RQD=61 Rec=100%					9 min. Q _u = 818.5 tsf 162.4 pcf	
						Conglomeratic SANDSTONE -Gray-brown to dark brown, Fine to very coarse grained, Weathered to Highly Weathered, hard to very hard							6 min.	
													3 min.	
													3 min.	
	125			R-30	26			RQD=13 Rec=87%					4 min.	
													3 min.	
													7 min.	
				R-31	30	SILTSTONE -Red-brown to dark red-brown, Very fine grained, Weathered, very broken to slightly broken, moderately hard		RQD=0 Rec=100%					6 min.	
													6 min.	
	130					SANDSTONE -Gray-brown, Fine grained, Weathered to Slightly Weathered, very broken to massive, moderately hard, multiple fractures throughout							5 min.	
													3 min.	
				R-32	50			RQD=73 Rec=83%					3 min.	
													3 min.	
						SANDSTONE -Light gray-brown to dark gray-brown, Fine grained, Weathered to Highly Weathered, very broken to massive, hard to very hard							3 min.	
	135												2 min.	
													3 min.	
				R-33	42			RQD=14 Rec=69%					3 min.	
						Conglomeratic SANDSTONE -Gray-brown to dark gray-brown, Fine to medium grained, Weathered, very broken to massive, very hard							3 min.	
						Conglomeratic SANDSTONE -Light gray-brown to dark gray-brown to brown, Fine to very coarse grained, Slightly Weathered, very broken to massive, hard to extremely hard							4 min.	
	140					Weathered layer @ 142.4 feet (~ 7 inches thick)							Q _u = 801.6 tsf 157.1 pcf	
													4 min.	
				R-34	60			RQD=78 Rec=100%					3 min.	
													4 min.	
													3 min.	
	145												3 min.	
													3 min.	
				R-35	55			RQD=58 Rec=92%					3 min.	
						Conglomerate seam @ 147.3 feet (~ 3 inches thick)							2 min.	
													3 min.	
													3 min.	
	150													

Continued Next Page

	Professional Service Industries, Inc.	PROJECT NO.: 04911456
	1707 S. Cameron Street, Suite B	PROJECT: Energy Transfer HDD (DPS)
	Harrisburg, PA 17104	LOCATION: Wetland "J47" (PPP5)
	Telephone: (717) 230-8622	Dauphin Co., PA
		PA-LE-0001.0000-SR/PO#201770816-2

DATE STARTED: 8/28/17 DATE COMPLETED: 8/31/17 COMPLETION DEPTH: 159.0 ft BENCHMARK: N/A ELEVATION: N/A LATITUDE: n/a° LONGITUDE: n/a° STATION: N/A OFFSET: N/A REMARKS:	DRILL COMPANY: Eichelberger's, Inc. DRILLER: S. Taylor LOGGED BY: L. Proczko DRILL RIG: Diedrich D-50 DRILLING METHOD: Casing/Rock Coring SAMPLING METHOD: 2-in SS1.874-in Core HAMMER TYPE: Automatic EFFICIENCY: N/A REVIEWED BY: F. Hoffman	<div style="text-align: center; font-weight: bold; font-size: 1.2em;">BORING B-1</div> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td rowspan="3" style="width:5%; text-align: center; font-weight: bold;">Water</td> <td style="width:10%; text-align: center;">▽</td> <td style="width:65%;">While Drilling</td> <td style="width:20%;">Not Enc.</td> </tr> <tr> <td style="text-align: center;">▼</td> <td>Post-Core</td> <td>17 feet</td> </tr> <tr> <td style="text-align: center;">▽</td> <td></td> <td></td> </tr> </table> BORING LOCATION: See Boring Location Plan	Water	▽	While Drilling	Not Enc.	▼	Post-Core	17 feet	▽		
Water	▽	While Drilling		Not Enc.								
	▼	Post-Core		17 feet								
	▽											

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										<div> <div> X Moisture </div> <div> <div> <div>0</div> <div>25</div> <div>50</div> </div> <div> <div>PL</div> <div>LL</div> </div> </div> </div>	
	150			R-36	60	Conglomeratic SANDSTONE -Light gray-brown to dark gray-brown to brown, Fine to very coarse grained, Slightly Weathered, very broken to massive, hard to extremely hard		RQD=86 Rec=100%			3 min. 2 min. >>▲ Qu = 412.9 tsf 2 min. 152.2 pcf
	155			R-37	60	SILTSTONE -Red-brown, Very fine grained, Weathered to Highly Weathered, very broken to broken, moderately hard SILTSTONE -Dark red-brown, Very fine grained, Slightly Weathered, slightly broken to massive, moderately hard Conglomeratic SANDSTONE -Light gray to red-brown, Fine to coarse grained, Slightly Weathered, massive, hard Siltstone seam @ 156.9 feet (~ 2-1/2 inches thick) SILTSTONE -Dark red-brown, Very fine grained, Slightly Weathered, massive, moderately hard Test boring terminated @ 159 feet		RQD=87 Rec=100%			3 min. 4 min. 7 min. 4 min. 4 min. 5 min. 5 min.

<div style="display: flex; align-items: center;"> <div> Professional Service Industries, Inc. 1707 S. Cameron Street, Suite B Harrisburg, PA 17104 Telephone: (717) 230-8622 </div> </div>	PROJECT NO.: 04911456 PROJECT: Energy Transfer HDD (DPS) LOCATION: Wetland "J47" (PPP5) Dauphin Co., PA PA-LE-0001.0000-SR/PO#201770816-2
---	--

0111456
Spec Core B-1
6-28-17
6'3"-2'
Box 1 of
Wetlands 347

Run	Depth (ft.)	Recovery (%)	R.R.D. (in.)
R-1	6'3"-9'	31.5	5.5
R-2	9'-16'	60	13
R-3	16'-17'	12	0.0
R-4	17'-24'	37	0.0



04911456
Core Box B-1
B-23-17
Box 2 of
Wetland 347

RUN	Depth (ft.)	Rec (in.)	RAD (in.)
R5	24-29	42	0.0
R6	29-34	48	14
R7	34-59	60	40

29

34

39

07/11/456
Geo Boring R-1
B: 28.17
39-52.9
Box 3 of
WETLAND J47

8217

RUN	Depth (ft.)	Roc (in.)	RQD (in.)
R-8	39-44	60	47
R-9	44-48	48	18
R-10	48-49	12	10.5
R-11	49-54	60	59



04911456
640 Box B-1
829.17 / 820.17
629.827
Box 5 of
Wellhead J97

04911456
640 Box B-1
829.17
529.679
Box 4 of
Wellhead J97

RUN	Dipth (ft)	Rac (in)	R&D (in)
R-12	54-59	60	46
R-13	59-64	60	37.5
R-14	64-69	60	39.5



PPH5
06/01/1952
B-1
6-29-17
67-2-1
Box
Wetland 347

	DVN	Dpt (ft)	Rc (in)	Rd (in)
8-7-17	R-15	69-74	60	55.5
8-20-17	R-16	74-79	60	57.5
	P-17	79-81	71	0
	R-18	81-81.2	42	0
	P-19	81.2-84	34	14



PPP 35
 0491456
 Geo Box B-1
 8-30-17
 B27-99
 Box 6 of
 wetland 547

Run	Depth (ft.)	Rac (m.)	Rac (ft.)
R-20	84-87.4	18	0
R-21	87.4-89	19	12.5
R-22	89-94	57	39.5
R-23	94-99	58	43.5

water 17'



PDP#5
04911456
Geo Bore B-1
8-30-17 / 8-31-17
94-114
Box 7 of
Wetland J47

Run	Depth(ft)	Rec(in.)	RQD(in.)
R-24	99-104	60	14
R-25	104-109	60	22.5
R-26	109-113	48	4
R-27	113-114	12	7

8-30-17

8-31-17

65

101

113

114

PPP#5
04911456
B-1
831.17
114-128.6
Box 8 of
Wetland J47

RUN	Depth(ft)	Rec(in.)	R&D(in.)
R-28	114-119	54.60	41
R-29	119-124	60	36.5
R-30	124-126.5	26	4
R-31	126.5-129	32	0



PPT#5
 04911456
 B-1
 8/31/17
 1286-1452
 Box 9 of
 Wetland J47

Run	Depth (ft)	Rac (in.)	RQ Dens
R-32	129-134	50	44
R-33	134-139	41.5	8.5
R-34	139-144	60	46.5
R-35	144-149	55	35



N5-2

PPP#5
04911456
B-1
8.31.17
145.2-159
Box 10 of 10
Wetland J47

RUN	Depth (ft.)	REC (in.)	R&D (in.)
R-36	149-154	60	51.5
R-37	154-159	60	52

143-2
149

154




149

159

04911456
Box 10 of 10

BORING LOCATION:
See Boring Location Plan

STANDARD PENETRATION TEST DATA				Additional Remarks
N in blows/ft ©				
✕ Moisture	▣ PL			
0 25	+ LL	50		
STRENGTH, tsf				
▲ Qu	✱ Qp			
0 2.0	4.0			

BORING B-2			
Water		While Drilling	Not Enc.
		Post-Core	22.3 feet
			

BORING LOCATION:
 See Boring Location Plan

[illegible]

DATE STARTED: 9/5/17 DATE COMPLETED: 9/7/17 COMPLETION DEPTH: 201.0 ft BENCHMARK: N/A ELEVATION: N/A LATITUDE: n/a° LONGITUDE: n/a° STATION: N/A OFFSET: N/A REMARKS:	DRILL COMPANY: Eichelberger's, Inc. DRILLER: T. Growden LOGGED BY: C. Lehman DRILL RIG: Diedrich D-50 DRILLING METHOD: Casing/Rock Coring SAMPLING METHOD: 2-in SS1.874-in Core HAMMER TYPE: Automatic EFFICIENCY: N/A REVIEWED BY: F. Hoffman	<h2 style="margin:0;">BORING B-2</h2>
		<div style="display: flex; justify-content: space-between;"> <div style="width: 30%;"> Water <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> While Drilling </div> <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> Post-Core </div> </div> <div style="width: 60%; text-align: right;"> Not Enc. 22.3 feet </div> </div>
BORING LOCATION: See Boring Location Plan		

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> X Moisture PL LL </div> <div style="width: 50%;"> 0 25 50 </div> </div> <div style="display: flex; justify-content: space-between; margin-top: 5px;"> <div style="width: 45%;"> ▲ Qu * Qp </div> <div style="width: 50%;"> 0 2.0 4.0 </div> </div>	
60						SANDSTONE -Gray-brown to dark gray-brown, Fine grained, Weathered to Slightly Weathered, very broken to massive, very hard to extremely hard					4 min.
						SILTSTONE -Red-brown to dark red-brown, Very fine grained, Weathered to Slightly Weathered, broken to massive, moderately hard					>>▲ 6 min. 63.7 tsf 164.7 pcf
						SILTSTONE -Dark gray-brown, Very fine grained, Highly Weathered, very broken to broken, moderately hard		RQD=49 Rec=84%			4 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					4 min.
						SILTSTONE -Red-gray-brown, Very fine grained, Highly Weathered, very broken to slightly broken, moderately hard					4 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					4 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					3 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					3 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard		RQD=97 Rec=100%			>>▲ 3 min. 41.2 tsf 167.7 pcf
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					3 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					3 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					4 min.
						SILTSTONE -Red-brown to red-gray-brown, Very fine grained, Slightly Weathered, broken to massive, moderately hard					4 min.
						SILTSTONE/SANDSTONE -Red-brown to dark gray-brown, Very fine to fine grained, Weathered, broken to slightly broken, moderately hard to very hard		RQD=62 Rec=100%			>>▲ 4 min. 106.1 tsf 163.5 pcf
						SANDSTONE -Gray-brown to dark gray-brown, Fine to medium grained, Weathered to Slightly Weathered, hard to very hard, trace calcite stringers					4 min.
						Broken layer @ 80.1 feet (~ 5-1/4 inches thick)					4 min.
						Weathered seam @ 82.5 feet (~ 1-1/2 inches thick)					4 min.
						Broken layer @ 83.3 feet (~ 4-1/2 inches thick)					>>▲ 4 min. 106.1 tsf 163.5 pcf
						SHALE -Red-brown, Very fine grained, Slightly Weathered, very broken to massive, moderately hard		RQD=88 Rec=100%			4 min.
											4 min.
											4 min.

Continued Next Page



Professional Service Industries, Inc.
1707 S. Cameron Street, Suite B
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Telephone: (717) 230-8622

PROJECT NO.: 04911456
PROJECT: Energy Transfer HDD (DPS)
LOCATION: Wetland "J47" (PPP5)
Dauphin Co., PA
PA-LE-0001.0000-SR/PO#201770816-2

DATE STARTED: 9/5/17 DATE COMPLETED: 9/7/17 COMPLETION DEPTH: 201.0 ft BENCHMARK: N/A ELEVATION: N/A LATITUDE: n/a° LONGITUDE: n/a° STATION: N/A OFFSET: N/A REMARKS:		DRILL COMPANY: Eichelberger's, Inc. DRILLER: T. Growden LOGGED BY: C. Lehman DRILL RIG: Diedrich D-50 DRILLING METHOD: Casing/Rock Coring SAMPLING METHOD: 2-in SS1.874-in Core HAMMER TYPE: Automatic EFFICIENCY: N/A REVIEWED BY: F. Hoffman		BORING B-2 <div style="display: flex; justify-content: space-between;"> <div style="width: 30%;"> Water <input checked="" type="checkbox"/> While Drilling <input checked="" type="checkbox"/> Post-Core </div> <div style="width: 30%;"> Not Enc. 22.3 feet </div> </div> BORING LOCATION: See Boring Location Plan	
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Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @ <div style="display: flex; justify-content: space-between; font-size: 0.8em;"> × Moisture ■ PL </div> <div style="display: flex; justify-content: space-between; font-size: 0.8em;"> ▲ Qu * Qp </div>	Additional Remarks	
90						Broken layer @ 89.6 feet (~ 5 inches thick) SHALE -Red-brown, Very fine grained, Slightly Weathered, very broken to massive, moderately hard					4 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min.	
95			R-12	120		Weathered layer @ 99.7 feet (~ 3-1/2 inches thick)		RQD=95 Rec=100%			>>▲ $Q_u = 161.1$ tsf 161.1 pcf	
											5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min. 5 min.	
100											>>▲ $Q_u = 352.4$ tsf 352.4 pcf	
											4 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min.	
105												>>▲ $Q_u = 848.8$ tsf 848.8 pcf
											3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min. 3 min.	
110												4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min.
												4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min.
115												4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min.
120												

Continued Next Page

Professional Service Industries, Inc.
 1707 S. Cameron Street, Suite B
 Harrisburg, PA 17104
 Telephone: (717) 230-8622

PROJECT NO.: 04911456
PROJECT: Energy Transfer HDD (DPS)
LOCATION: Wetland "J47" (PPP5)
 Dauphin Co., PA
 PA-LE-0001.0000-SR/PO#201770816-2

DATE STARTED: 9/5/17 DATE COMPLETED: 9/7/17 COMPLETION DEPTH: 201.0 ft BENCHMARK: N/A ELEVATION: N/A LATITUDE: n/a° LONGITUDE: n/a° STATION: N/A OFFSET: N/A REMARKS:	DRILL COMPANY: Eichelberger's, Inc. DRILLER: T. Growden LOGGED BY: C. Lehman DRILL RIG: Diedrich D-50 DRILLING METHOD: Casing/Rock Coring SAMPLING METHOD: 2-in SS1.874-in Core HAMMER TYPE: Automatic EFFICIENCY: N/A REVIEWED BY: F. Hoffman	<h2 style="margin:0;">BORING B-2</h2>			
		<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:10%; text-align: center;">Water</td> <td style="width:80%;"> <div style="display: flex; justify-content: space-between;"> ▽ While Drilling Not Enc. </div> <div style="display: flex; justify-content: space-between;"> ▼ Post-Core 22.3 feet </div> </td> <td style="width:10%;"></td> </tr> </table>	Water	<div style="display: flex; justify-content: space-between;"> ▽ While Drilling Not Enc. </div> <div style="display: flex; justify-content: space-between;"> ▼ Post-Core 22.3 feet </div>	
Water	<div style="display: flex; justify-content: space-between;"> ▽ While Drilling Not Enc. </div> <div style="display: flex; justify-content: space-between;"> ▼ Post-Core 22.3 feet </div>				
		BORING LOCATION: See Boring Location Plan			

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft   Moisture  PL 			
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	Professional Service Industries, Inc. 1707 S. Cameron Street, Suite B Harrisburg, PA 17104 Telephone: (717) 230-8622	PROJECT NO.: 04911456 PROJECT: Energy Transfer HDD (DPS) LOCATION: Wetland "J47" (PPP5) Dauphin Co., PA PA-LE-0001.0000-SR/PO#201770816-2
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The stratification lines represent approximate boundaries. The transition may be gradual.

DATE STARTED: 9/5/17 DATE COMPLETED: 9/7/17 COMPLETION DEPTH: 201.0 ft BENCHMARK: N/A ELEVATION: N/A LATITUDE: n/a° LONGITUDE: n/a° STATION: N/A OFFSET: N/A REMARKS:	DRILL COMPANY: Eichelberger's, Inc. DRILLER: T. Growden LOGGED BY: C. Lehman DRILL RIG: Diedrich D-50 DRILLING METHOD: Casing/Rock Coring SAMPLING METHOD: 2-in SS1.874-in Core HAMMER TYPE: Automatic EFFICIENCY: N/A REVIEWED BY: F. Hoffman	<h2 style="margin: 0;">BORING B-2</h2>			
		<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:10%; text-align: center;">Water</td> <td style="width:40%;"> <input checked="" type="checkbox"/> While Drilling <input checked="" type="checkbox"/> Post-Core <input checked="" type="checkbox"/> </td> <td style="width:50%;"> Not Enc. 22.3 feet </td> </tr> </table>	Water	<input checked="" type="checkbox"/> While Drilling <input checked="" type="checkbox"/> Post-Core <input checked="" type="checkbox"/>	Not Enc. 22.3 feet
Water	<input checked="" type="checkbox"/> While Drilling <input checked="" type="checkbox"/> Post-Core <input checked="" type="checkbox"/>	Not Enc. 22.3 feet			
		BORING LOCATION: See Boring Location Plan			

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										X Moisture <input checked="" type="checkbox"/> PL <input checked="" type="checkbox"/> LL 0 25 50 STRENGTH, tsf ▲ Qu * Qp 0 2.0 4.0	
150						thick) SANDSTONE -Light gray-brown to dark red-brown, Fine to medium grained, Slightly Weathered, broken to massive, moderately hard to very hard, trace calcite stringers					4 min. 4 min. Q _u = 383.3 tsf 163.9 pcf
				R-19	115	SHALE -Red-brown to dark red-brown, Very fine grained, Weathered to Slightly Weathered, very broken to massive, moderately hard		RQD=59 Rec=96%			4 min. 4 min. 4 min. 4 min. 4 min. 4 min. 4 min.
						Weathered/Highly Weathered seam @ 157.1 feet (~ 2-3/4 inches thick) Highly Weathered layer @ 158.3 feet (~ 5 inches thick)					4 min. 4 min. 4 min. 4 min. 4 min.
160				R-20	58	SHALE -Dark red-brown to dark red-gray-brown, Very fine grained, Slightly Weathered, very broken to massive, moderately hard Weathered layer @ 160 feet (~ 4-1/2 inches thick)		RQD=60 Rec=96%			4 min. 4 min. 4 min. 4 min. 4 min. 4 min.
											4 min. 4 min. Q _u = 410.3 tsf 195.6 pcf
165											4 min. 4 min. Q _u = 284.8 tsf 168.5 pcf
						Weathered/Highly Weathered layer @ 168.4 feet (~ 11 inches thick)					4 min. 4 min. 4 min. 4 min.
170				R-21	120	SANDSTONE -Light gray-brown to dark red-brown, Fine grained, Slightly Weathered, very broken to massive, hard to very hard		RQD=54 Rec=100%			4 min. 4 min. Q _u = 846.0 tsf 163.0 pcf
											4 min. 4 min. 4 min. 4 min.
175						Broken/very broken layer @ 174.7 feet (~ 4 inches thick)					4 min. 4 min. 4 min. 4 min.
				R-22	60	Broken/very broken layer @ 178.1 feet (~ 8 inches thick)		RQD=42 Rec=100%			4 min. 4 min. 4 min. 4 min.
180											
Continued Next Page											

Professional Service Industries, Inc. 1707 S. Cameron Street, Suite B Harrisburg, PA 17104 Telephone: (717) 230-8622	PROJECT NO.: 04911456 PROJECT: Energy Transfer HDD (DPS) LOCATION: Wetland "J47" (PPP5) Dauphin Co., PA PA-LE-0001.0000-SR/PO#201770816-2	
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DATE STARTED: 9/5/17		DRILL COMPANY: Eichelberger's, Inc.		BORING B-2	
DATE COMPLETED: 9/7/17		DRILLER: T. Growden LOGGED BY: C. Lehman			
COMPLETION DEPTH: 201.0 ft		DRILL RIG: Diedrich D-50		<div style="display: flex; justify-content: space-between;"> <div style="width: 30%;"> Water <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> While Drilling </div> <div style="display: flex; align-items: center;"> <div style="width: 10px; height: 10px; border: 1px solid black; margin-right: 5px;"></div> Post-Core </div> </div> <div style="width: 60%; text-align: right;"> Not Enc. 22.3 feet </div> </div>	
BENCHMARK: N/A		DRILLING METHOD: Casing/Rock Coring			
ELEVATION: N/A		SAMPLING METHOD: 2-in SS1.874-in Core		BORING LOCATION: See Boring Location Plan	
LATITUDE: n/a°		HAMMER TYPE: Automatic			
LONGITUDE: n/a°		EFFICIENCY: N/A			
STATION: N/A		OFFSET: N/A			
REMARKS:		REVIEWED BY: F. Hoffman			

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS) RQD & Recovery % (NX)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft				Additional Remarks
										X Moisture PL LL				
										STRENGTH, tsf				
										▲ Qu	✱	Qp		
										0	2.0	4.0		
	180					Conglomeratic SANDSTONE -Light gray-brown to gray-brown, Fine to very coarse grained, Slightly Weathered, very broken to massive, hard to very hard Broken seam @ 180.6 feet (~ 2 inches thick)							4 min. 0 min. 661.4 tsf 150.6 pcf 3 min.	
				R-23	118	SANDSTONE -Red-gray-brown, Fine to medium grained, Weathered to Highly Weathered, very broken to slightly broken, hard		RQD=69 Rec=98%					3 min.	
						Conglomeratic SANDSTONE -Gray-brown to dark red-brown, Fine to very coarse grained, Slightly Weathered, massive, moderately hard to very hard							3 min.	
						SILTSTONE -Brown to dark red-brown, Very fine grained, Slightly Weathered, very broken to massive, moderately hard							3 min.	
	185												3 min.	
				R-24	60			RQD=100 Rec=100%					3 min.	
													3 min.	
													3 min.	
	190												3 min.	
													3 min.	
													3 min.	
				R-25	60	SANDSTONE -Light gray to dark red-brown, Fine to coarse grained, Slightly Weathered, slightly broken to massive, hard to very hard		RQD=86 Rec=100%					3 min.	
													3 min.	
													3 min.	
	195												3 min.	
													3 min.	
													3 min.	
	200					Weathered seam @ 200 feet (~ 2 inches thick) Test boring terminated @ 201 feet							3 min.	

Qu

Qp

	Professional Service Industries, Inc. 1707 S. Cameron Street, Suite B Harrisburg, PA 17104 Telephone: (717) 230-8622		PROJECT NO.: 04911456 PROJECT: Energy Transfer HDD (DPS) LOCATION: Wetland "J47" (PPP5) Dauphin Co., PA
			PA-LE-0001.0000-SR/PO#201770816-2

0491-1456
PPS
With and J47
B=2
9/5/17
Box 1 of 1

Run	Depth	Per	Red
R-1	94-130	2.0	0.0
R-2	13.0-20.0	6.3	1.9
R-3	20.0-30.0	9.2	5.4

TOP



0491-1456
PPPS
Wetland J47
B-2
9/5/17
Box 2 of

Run	Depth	Ree	R00
R3(amt)	20.0-30.0	9.2	5.4
R-4	30.0-38.0	8.0	3.9
R-5	38.0-42.0	25	6.2

TOP



0491-1456
 PPP5
 Wetland J47
 B-2
 Box 3 of -
 9/5/17

Run	Depth	Rc.	RQD
R-5(cont)	38.0-45.0	25	100
R-6	45.0-50.0	100	100



TOP



450

550

0491-1456
PPPS
Wetland J47
B-2
Box 11 of
9/6/17

Run	Depth	Rec.	MSD
R-7	55.0-61.0	58	4.3
R-8	61.0-68.5	63	3.7
R-9	68.5-75.0	65	3.3

TOP

550

630

550



0491-1956
PPPS
Wetland 547
B-2
Box 5 of
91677

Run	Depth	Re	RAD
R-10	750-850	10.0	10.2
\$			



0491-1456

PPP5

Wetland J47

B-2

Box 6 of -

9/6/17

Run	Depth	Re.	RQD
R-11	85.0-90.0	5.0	11.4
R-12	90.0-100.0	10.0	9.5

Top

90.0

90.0

0491-1456

PPDS

Wellhead J47

B-2

Box 7 of

9/6/17

Km	Depth	Rec.	Run
R-13	100.0-105.0	5.0	2.7
R-14	105.0-115.0	10.0	5.5

Top

105.0

0517

0101-1456

PPPS

Wetland J47

B-2

B-8 of -

9/6/17

Run	Depth	Per.	Rad
R-15	115.0-125.0	7.1	5.3
R-16	125.0-130.0	5.0	2.5
R-17	130.0-140.0	10.0	7.5

TOP

130.0

135.0

0491-1456

PPFS

Wetland J47

B-2

Box 9 of

9/7/17

Run	Depth	Rec	RCD
R-76	130.0-140.0	10.0	7.5
R-78	140.0-150.0	10.0	7.8

Top

140.0

140.0

D491-1456
PPPS
Wetland J47
B-2
Box 10 of -
9/87+17

Run	Depth	R _g	R _{2D}
R-18(cont)	140.0-150.0	10.0	7.8
R-19	150.0-160.0	9.6	5.9
R-20	160.0-165.0	4.8	3.0

Top



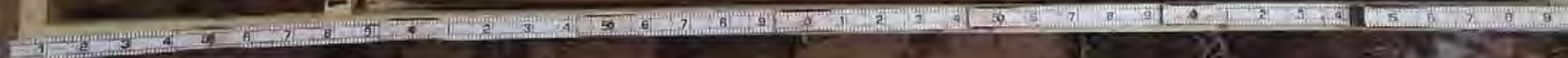
0491-1456
PPPS
Wetland 547
B-2
Box 11 of 1
9/7/17

Run	Depth	Rc	RSD
R-20 (cont)	162.0 - 165.0	4.8	3.0
R-21	165.0 - 175.0	10.0	5.4
R-22	175.0 - 180.0	5.0	2.1

TOP

162.0

175.0



0491-1456

PPD5

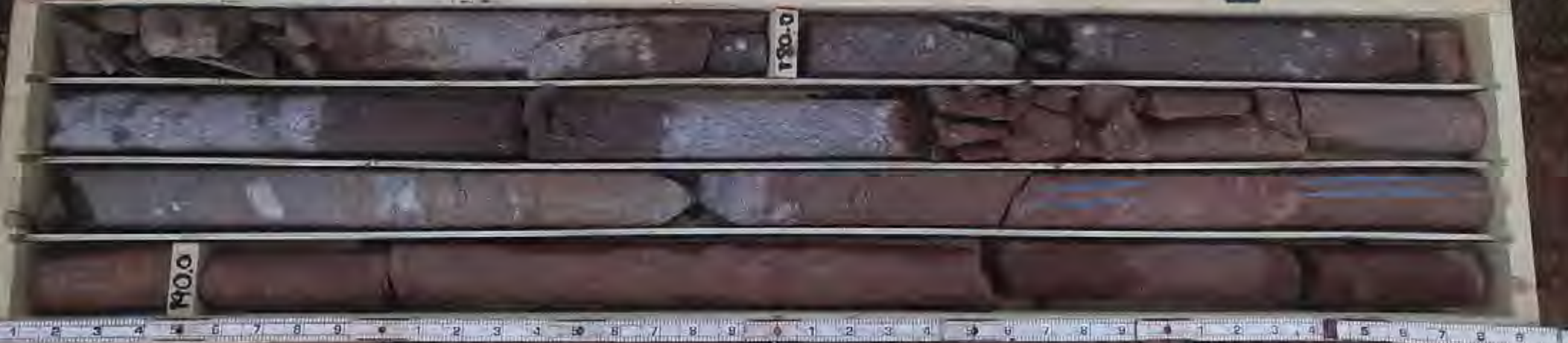
Wetland J47

B-2

Box 12 of —

9/7/17

Run	Depth	Res.	R90
R-22 (cont)	175.0-180.0	5.0	2.1
R-23	180.0-190.0	9.8	6.9
R-24	190.0-195.0	5.0	5.0



0491-1456
PPPS
Wetland J47
B-2
Box 13 of 13
9/7/17

Rim	Depth	Rer.	R30
R-24 (cont)	190.0-195.0	5.0	5.0
R-25	195.0-200.0	5.0	9.3
	EOB		

TOP

195.0

200.0

210.2



GENERAL NOTES

SAMPLE IDENTIFICATION

The Unified Soil Classification System (USCS), AASHTO 1988 and ASTM designations D2487 and D-2488 are used to identify the encountered materials unless otherwise noted. Coarse-grained soils are defined as having more than 50% of their dry weight retained on a #200 sieve (0.075mm); they are described as: boulders, cobbles, gravel or sand. Fine-grained soils have less than 50% of their dry weight retained on a #200 sieve; they are defined as silts or clay depending on their Atterberg Limit attributes. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size.

DRILLING AND SAMPLING SYMBOLS

SFA: Solid Flight Auger - typically 4" diameter flights, except where noted.	☒ SS: Split-Spoon - 1 3/8" I.D., 2" O.D., except where noted.
HSA: Hollow Stem Auger - typically 3 1/4" or 4 1/4" I.D. openings, except where noted.	■ ST: Shelby Tube - 3" O.D., except where noted.
M.R.: Mud Rotary - Uses a rotary head with Bentonite or Polymer Slurry	▮ RC: Rock Core
R.C.: Diamond Bit Core Sampler	⬇ TC: Texas Cone
H.A.: Hand Auger	✋ BS: Bulk Sample
P.A.: Power Auger - Handheld motorized auger	☒ PM: Pressuremeter
	CPT-U: Cone Penetrometer Testing with Pore-Pressure Readings

SOIL PROPERTY SYMBOLS

N: Standard "N" penetration: Blows per foot of a 140 pound hammer falling 30 inches on a 2-inch O.D. Split-Spoon.
N ₆₀ : A "N" penetration value corrected to an equivalent 60% hammer energy transfer efficiency (ETR)
Q _u : Unconfined compressive strength, TSF
Q _p : Pocket penetrometer value, unconfined compressive strength, TSF
w%: Moisture/water content, %
LL: Liquid Limit, %
PL: Plastic Limit, %
PI: Plasticity Index = (LL-PL), %
DD: Dry unit weight, pcf
▼, ▽, ▾ Apparent groundwater level at time noted

RELATIVE DENSITY OF COARSE-GRAINED SOILS ANGULARITY OF COARSE-GRAINED PARTICLES

Relative Density	N - Blows/foot	Description	Criteria
Very Loose	0 - 4	Angular:	Particles have sharp edges and relatively plane sides with unpolished surfaces
Loose	4 - 10	Subangular:	Particles are similar to angular description, but have rounded edges
Medium Dense	10 - 30	Subrounded:	Particles have nearly plane sides, but have well-rounded corners and edges
Dense	30 - 50	Rounded:	Particles have smoothly curved sides and no edges
Very Dense	50 - 80		
Extremely Dense	80+		

GRAIN-SIZE TERMINOLOGY

Component	Size Range
Boulders:	Over 300 mm (>12 in.)
Cobbles:	75 mm to 300 mm (3 in. to 12 in.)
Coarse-Grained Gravel:	19 mm to 75 mm (3/4 in. to 3 in.)
Fine-Grained Gravel:	4.75 mm to 19 mm (No.4 to 3/4 in.)
Coarse-Grained Sand:	2 mm to 4.75 mm (No.10 to No.4)
Medium-Grained Sand:	0.42 mm to 2 mm (No.40 to No.10)
Fine-Grained Sand:	0.075 mm to 0.42 mm (No. 200 to No.40)
Silt:	0.005 mm to 0.075 mm
Clay:	<0.005 mm

PARTICLE SHAPE

Description	Criteria
Flat:	Particles with width/thickness ratio > 3
Elongated:	Particles with length/width ratio > 3
Flat & Elongated:	Particles meet criteria for both flat and elongated

RELATIVE PROPORTIONS OF FINES

Descriptive Term	% Dry Weight
Trace:	< 5%
With:	5% to 12%
Modifier:	>12%

GENERAL NOTES

(Continued)

CONSISTENCY OF FINE-GRAINED SOILS

Q_u - TSF	N - Blows/foot	Consistency
0 - 0.25	0 - 2	Very Soft
0.25 - 0.50	2 - 4	Soft
0.50 - 1.00	4 - 8	Firm (Medium Stiff)
1.00 - 2.00	8 - 15	Stiff
2.00 - 4.00	15 - 30	Very Stiff
4.00 - 8.00	30 - 50	Hard
8.00+	50+	Very Hard

MOISTURE CONDITION DESCRIPTION

Description	Criteria
Dry:	Absence of moisture, dusty, dry to the touch
Moist:	Damp but no visible water
Wet:	Visible free water, usually soil is below water table

RELATIVE PROPORTIONS OF SAND AND GRAVEL

Descriptive Term	% Dry Weight
Trace:	< 15%
With:	15% to 30%
Modifier:	>30%

STRUCTURE DESCRIPTION

Description	Criteria	Description	Criteria
Stratified:	Alternating layers of varying material or color with layers at least ¼-inch (6 mm) thick	Blocky:	Cohesive soil that can be broken down into small angular lumps which resist further breakdown
Laminated:	Alternating layers of varying material or color with layers less than ¼-inch (6 mm) thick	Lensed:	Inclusion of small pockets of different soils
Fissured:	Breaks along definite planes of fracture with little resistance to fracturing	Layer:	Inclusion greater than 3 inches thick (75 mm)
Slickensided:	Fracture planes appear polished or glossy, sometimes striated	Seam:	Inclusion 1/8-inch to 3 inches (3 to 75 mm) thick extending through the sample
		Parting:	Inclusion less than 1/8-inch (3 mm) thick

SCALE OF RELATIVE ROCK HARDNESS

Q_u - TSF	Consistency
2.5 - 10	Extremely Soft
10 - 50	Very Soft
50 - 250	Soft
250 - 525	Medium Hard
525 - 1,050	Moderately Hard
1,050 - 2,600	Hard
>2,600	Very Hard

ROCK BEDDING THICKNESSES

Description	Criteria
Very Thick Bedded	Greater than 3-foot (>1.0 m)
Thick Bedded	1-foot to 3-foot (0.3 m to 1.0 m)
Medium Bedded	4-inch to 1-foot (0.1 m to 0.3 m)
Thin Bedded	1¼-inch to 4-inch (30 mm to 100 mm)
Very Thin Bedded	½-inch to 1¼-inch (10 mm to 30 mm)
Thickly Laminated	1/8-inch to ½-inch (3 mm to 10 mm)
Thinly Laminated	1/8-inch or less "paper thin" (<3 mm)

ROCK VOIDS

Voids	Void Diameter
Pit	<6 mm (<0.25 in)
Vug	6 mm to 50 mm (0.25 in to 2 in)
Cavity	50 mm to 600 mm (2 in to 24 in)
Cave	>600 mm (>24 in)

GRAIN-SIZED TERMINOLOGY

(Typically Sedimentary Rock)	
Component	Size Range
Very Coarse Grained	>4.76 mm
Coarse Grained	2.0 mm - 4.76 mm
Medium Grained	0.42 mm - 2.0 mm
Fine Grained	0.075 mm - 0.42 mm
Very Fine Grained	<0.075 mm

ROCK QUALITY DESCRIPTION

Rock Mass Description	RQD Value
Excellent	90 - 100
Good	75 - 90
Fair	50 - 75
Poor	25 - 50
Very Poor	Less than 25

DEGREE OF WEATHERING

Slightly Weathered:	Rock generally fresh, joints stained and discoloration extends into rock up to 25 mm (1 in), open joints may contain clay, core rings under hammer impact.
Weathered:	Rock mass is decomposed 50% or less, significant portions of the rock show discoloration and weathering effects, cores cannot be broken by hand or scraped by knife.
Highly Weathered:	Rock mass is more than 50% decomposed, complete discoloration of rock fabric, core may be extremely broken and gives clunk sound when struck by hammer, may be shaved with a knife.

Degree of Brokenness

Characteristic	Description
Less than 1 inch	Very Broken
1 inch to 3 inches	Broken
3 inches to 6 inches	Slightly Broken
Greater than 6 inches	Massive

SOIL CLASSIFICATION CHART

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	CLEAN GRAVELS (LITTLE OR NO FINES)		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
				GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
				GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
	SAND AND SANDY SOILS MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		SM	SILTY SANDS, SAND - SILT MIXTURES
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50			ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS LIQUID LIMIT GREATER THAN 50			MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HIGHLY ORGANIC SOILS				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

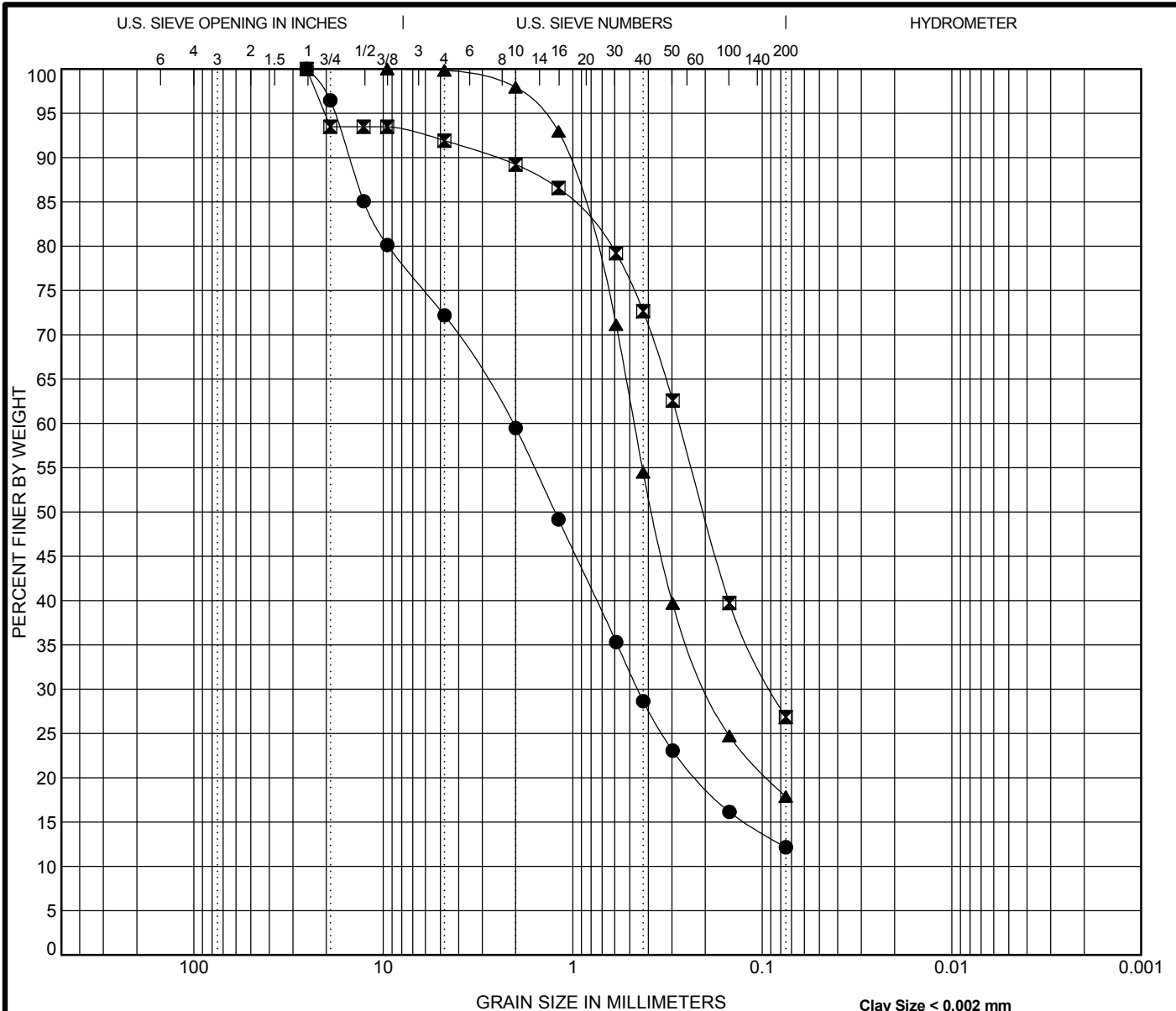
Table 4-3 Hardness and unconfined compressive strength of rock materials

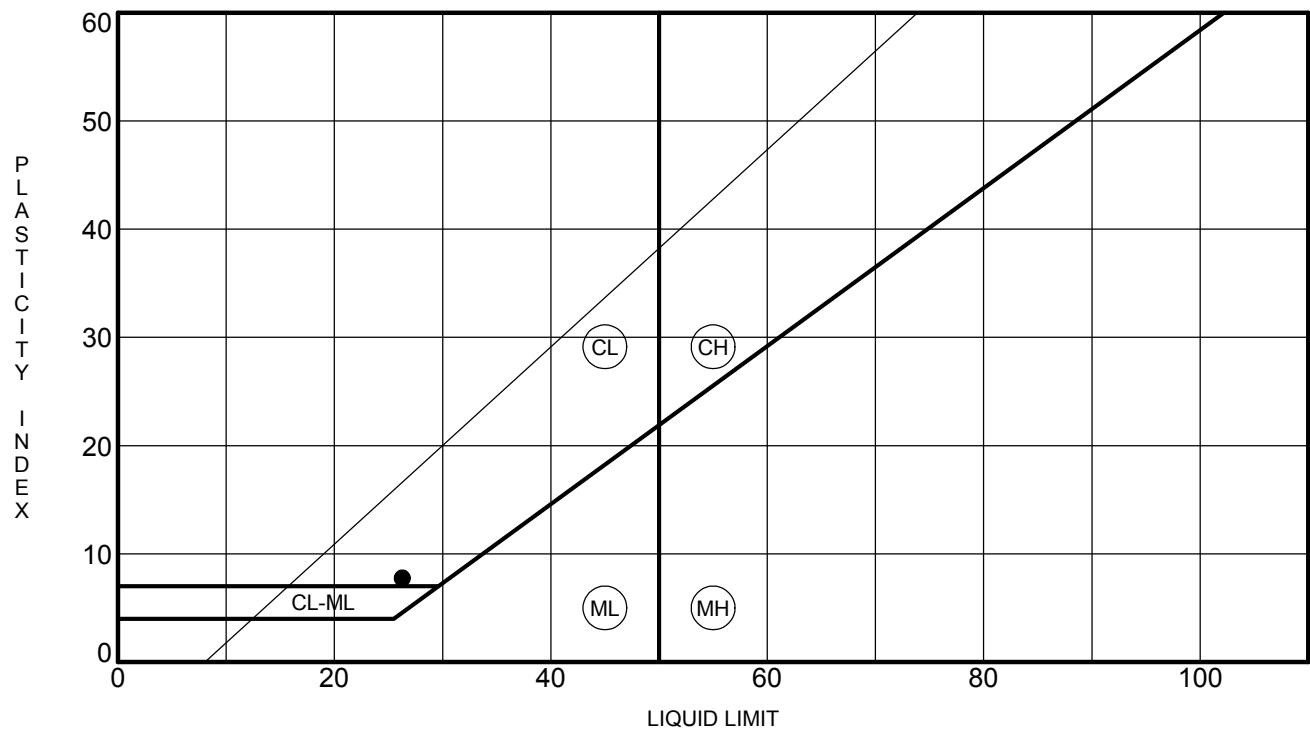
Hardness category	Typical range in unconfined compressive strength (MPa)	Strength value selected (MPa)	Field test on sample	Field test on outcrop
Soil*	< 0.60		Use USCS classifications	
Very soft rock or hard, soil-like material	0.60–1.25		Scratched with fingernail. Slight indentation by light blow of point of geologic pick. Requires power tools for excavation. Peels with pocket knife.	
Soft rock	1.25–5.0		Permits denting by moderate pressure of the fingers. Handheld specimen crumbles under firm blows with point of geologic pick.	Easily deformable with finger pressure.
Moderately soft rock	5.0–12.5		Shallow indentations (1–3 mm) by firm blows with point of geologic pick. Peels with difficulty with pocket knife. Resists denting by the fingers, but can be abraded and pierced to a shallow depth by a pencil point. Crumbles by rubbing with fingers.	Crumbles by rubbing with fingers.
Moderately hard rock	12.5–50		Cannot be scraped or peeled with pocket knife. Intact handheld specimen breaks with single blow of geologic hammer. Can be distinctly scratched with 20d common steel nail. Resists a pencil point, but can be scratched and cut with a knife blade.	Unfractured outcrop crumbles under light hammer blows.
Hard rock	50–100		Handheld specimen requires more than one hammer blow to break it. Can be faintly scratched with 20d common steel nail. Resistant to abrasion or cutting by a knife blade, but can be easily dented or broken by light blows of a hammer.	Outcrop withstands a few firm blows before breaking.
Very hard rock	100–250		Specimen breaks only by repeated, heavy blows with geologic hammer. Cannot be scratched with 20d common steel nail.	Outcrop withstands a few heavy ringing hammer blows but will yield large fragments.
Extremely hard rock	> 250		Specimen can only be chipped, not broken by repeated, heavy blows of geologic hammer.	Outcrop resists heavy ringing hammer blows and yields, with difficulty, only dust and small fragments.

Method used to determine consistency or hardness (check one):

Field assessment: _____ Uniaxial lab test: _____ Other: _____ Rebound hammer (ASTM D5873): _____

* See NEH631.03 for consistency and density of soil materials. For very stiff soil, SPT N values = 15 to 30. For very soft rock or hard, soil-like material, SPT N values exceed 30 blows per foot.



[illegible]

Professional Service Industries, Inc.
1707 S. Cameron Street, Suite B
Harrisburg, PA 17104
Telephone: (717) 230-8622
Fax: (717) 230-8626

ATTERBERG LIMIT RESULTS

PSI Job No.: 04911456
Project: Energy Transfer HDD (DPS)
Location: Wetland "J47" (PPP5)
Dauphin Co., PA

Laboratory Summary Sheet

Sheet 1 of 1

Borehole	Approx. Depth	Liquid Limit	Plastic Limit	Plasticity Index	Qu (tsf)	%<#200 Sieve	Est. Specific Gravity	Water Content (%)	Dry Density (pcf)	Saturation (%)	Void Ratio
B-1	1	26	19	7				15			
B-1	6					12.2%		7			
B-1	38.1				425.13						
B-1	43.2				379.04						
B-1	51.4				283.58						
B-1	54.2				441.13						
B-1	68				359.38						
B-1	75.4				162.82						
B-1	91.4				335.87						
B-1	106.6				511.08						
B-1	114.2				518.68						
B-1	120.9				818.48						
B-1	139.5				801.56						
B-1	151.4				412.90						
B-2	1							14			
B-2	3					26.8%		10			
B-2	9					17.9%		10			
B-2	15.5				594.20						
B-2	23.5				564.69						
B-2	38.8				43.62						
B-2	46.1				417.28						
B-2	55.1				603.02						
B-2	61.2				63.68						
B-2	71.3				411.16						
B-2	79.5				836.80						
B-2	85				106.08						
B-2	93.7				161.13						
B-2	100.7				352.44						
B-2	106.4				848.76						
B-2	123.8				496.44						
B-2	132				409.51						
B-2	138				789.65						
B-2	143.2				380.64						
B-2	151.8				383.27						
B-2	164.4				410.27						
B-2	166.2				284.76						
B-2	171.8				846.02						
B-2	181				661.42						
B-2	192.3				772.44						

Summary of Laboratory Results



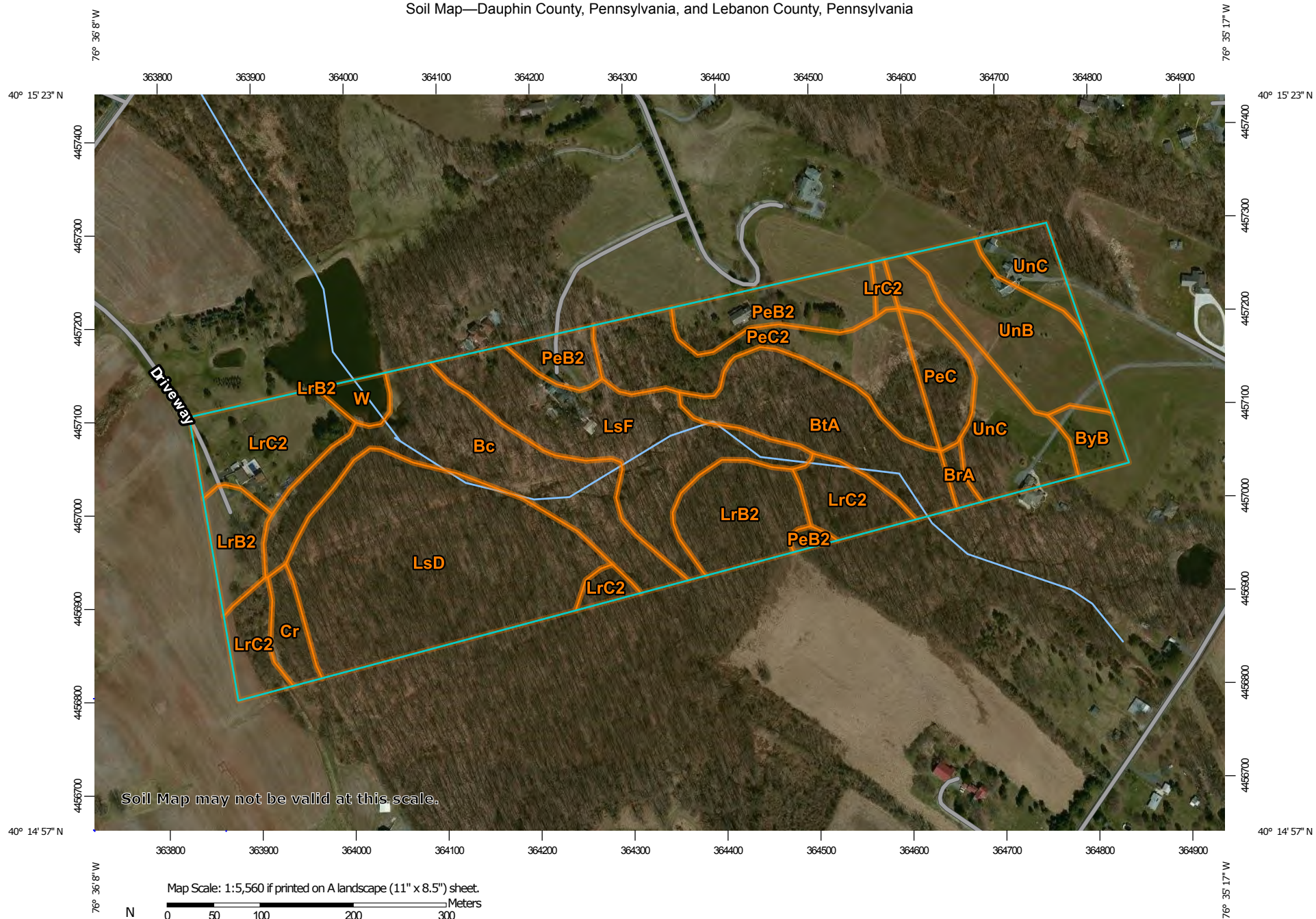
Professional Service Industries
1707 S. Cameron Street, Suite B
Harrisburg, PA 17104
Telephone: (717) 230-8622
Fax: (717) 230-8626

PSI Job No.: 04911456
Project: Energy Transfer HDD (DPS)
Location: Wetland "J47" (PPP5)
Dauphin Co., PA
PA-LE-0001.0000-SR/PO#201770816-2



ATTACHMENT 2
SOIL RESOURCES MAP AND PROFILE DESCRIPTIONS

Soil Map—Dauphin County, Pennsylvania, and Lebanon County, Pennsylvania



Soil Map may not be valid at this scale.



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey


9/21/2017
Page 1 of 4


MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)




















Soils







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features






 Blowout
 Borrow Pit
 Clay Spot
 Closed Depression
 Gravel Pit
 Gravelly Spot
 Landfill
 Lava Flow
 Marsh or swamp
 Mine or Quarry
 Miscellaneous Water
 Perennial Water
 Rock Outcrop
 Saline Spot
 Sandy Spot
 Severely Eroded Spot
 Sinkhole
 Slide or Slip
 Sodic Spot

 Spoil Area
 Stony Spot
 Very Stony Spot
 Wet Spot
 Other
 Special Line Features


Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at scales ranging from 1:15,800 to 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Dauphin County, Pennsylvania
 Survey Area Data: Version 10, Sep 19, 2016

Soil Survey Area: Lebanon County, Pennsylvania
 Survey Area Data: Version 11, Sep 19, 2016

Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or at different levels of detail. This may result in map unit symbols, soil properties, and interpretations that do not completely agree across soil survey area boundaries.

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 29, 2011—Apr 14, 2011

MAP LEGEND

MAP INFORMATION

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

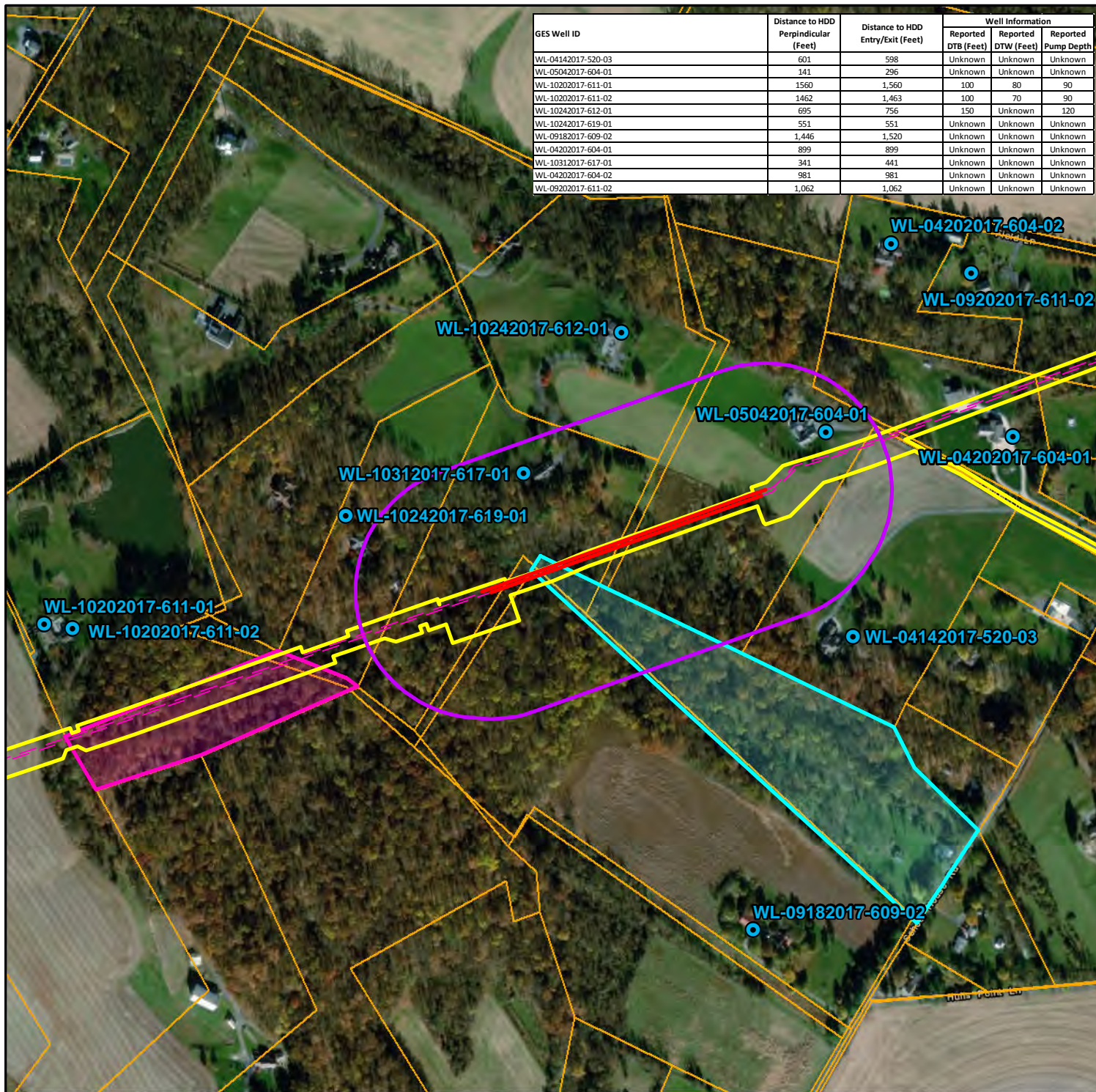
Map Unit Legend

Dauphin County, Pennsylvania (PA043)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Bc	Basher silt loam	7.5	10.9%
BtA	Brinkerton and Armagh silt loams, 0 to 3 percent slopes	5.4	7.7%
Cr	Croton silt loam, occasionally ponded, 0 to 3 percent slopes	1.1	1.5%
LrB2	Lewisberry gravelly sandy loam, 3 to 8 percent slopes, moderately eroded	4.9	7.1%
LrC2	Lewisberry gravelly sandy loam, 8 to 15 percent slopes, moderately eroded	7.3	10.5%
LsD	Lewisberry very stony sandy loam, 5 to 25 percent slopes	13.2	18.9%
LsF	Lewisberry very stony sandy loam, 25 to 60 percent slopes	7.4	10.7%
PeB2	Penn channery silt loam, 3 to 8 percent slopes	4.1	5.9%
PeC2	Penn channery silt loam, 8 to 15 percent slopes	5.4	7.7%
W	Water	0.7	1.0%
Subtotals for Soil Survey Area		56.9	81.9%
Totals for Area of Interest		69.5	100.0%

Lebanon County, Pennsylvania (PA075)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BrA	Brinkerton silt loam, 0 to 3 percent slopes	0.4	0.5%
ByB	Bucks silt loam, 3 to 8 percent slopes	1.0	1.4%
PeC	Penn channery silt loam, 8 to 15 percent slopes	1.7	2.4%
UnB	Ungers loam, 3 to 8 percent slopes	4.4	6.4%
UnC	Ungers loam, 8 to 15 percent slopes	5.2	7.5%
Subtotals for Soil Survey Area		12.6	18.1%
Totals for Area of Interest		69.5	100.0%



ATTACHMENT 3
SUPPLEMENTAL WATER SUPPLY INFORMATION



GES Well ID	Distance to HDD Perpendicular (Feet)	Distance to HDD Entry/Exit (Feet)	Well Information		
			Reported DTB (Feet)	Reported DTW (Feet)	Reported Pump Depth
WL-04142017-520-03	601	598	Unknown	Unknown	Unknown
WL-05042017-604-01	141	296	Unknown	Unknown	Unknown
WL-10202017-611-01	1560	1,560	100	80	90
WL-10202017-611-02	1462	1,463	100	70	90
WL-10242017-612-01	695	756	150	Unknown	120
WL-10242017-619-01	551	551	Unknown	Unknown	Unknown
WL-09182017-609-02	1,446	1,520	Unknown	Unknown	Unknown
WL-04202017-604-01	899	899	Unknown	Unknown	Unknown
WL-10312017-617-01	341	441	Unknown	Unknown	Unknown
WL-04202017-604-02	981	981	Unknown	Unknown	Unknown
WL-09202017-611-02	1,062	1,062	Unknown	Unknown	Unknown

Legend

- LOD
- Parcel
- PPP Centerline
- HDD
- 450 foot buffer of HDD alignment
- Public Water Supply/Landowner Confirmed No Well
- Testing Refused

****Testing locations current as of 01/23/2019**

- GES Testing Location

Location



0 250 500
Feet

Well Location Map
HDD# PA-LE-0001.0000-SR
Lebanon County, PA.

Prepared By:



Date:
1/23/2019

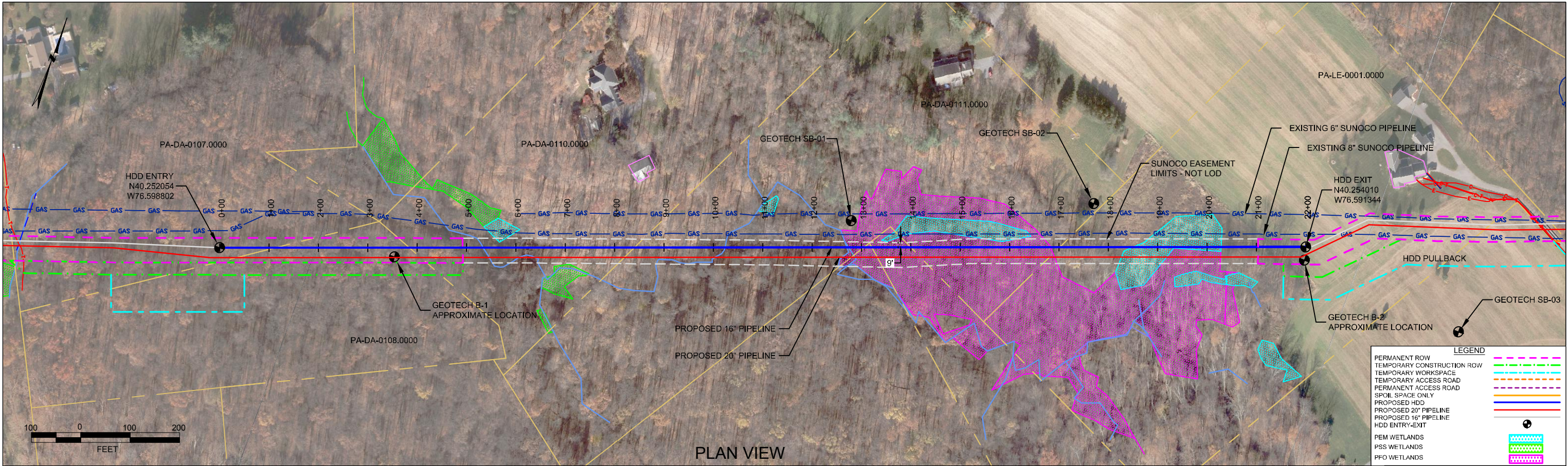
Base Map:
ESRI World Imagery, 09/24/2015

Coordinate System: NAD 83 Stateplane, PA South, Feet

**WETLAND J-47 CROSSING
PADEP SECTION 105 PERMIT NO.S: E22-617; E38-194
PA-LE-0001.0000-SR-16
(SPLP HDD No. S3-0090-16)**

ATTACHMENT 2

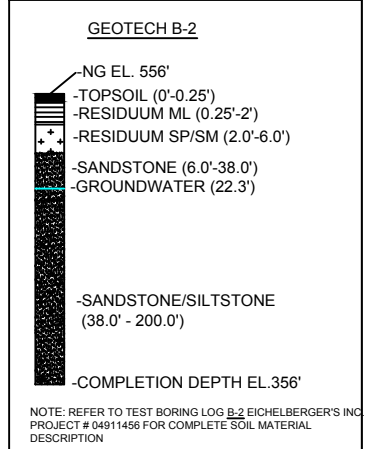
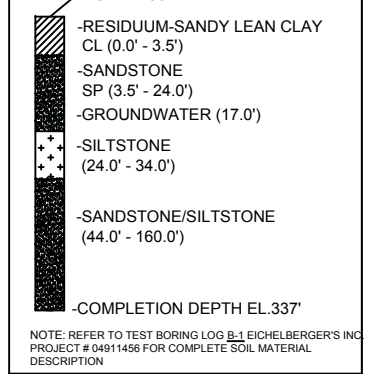
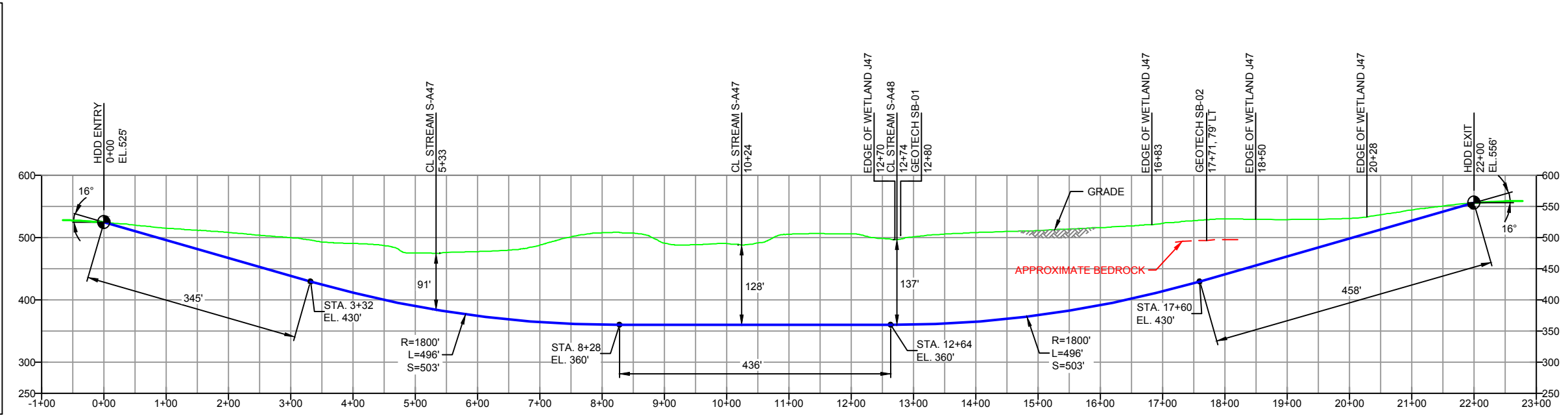
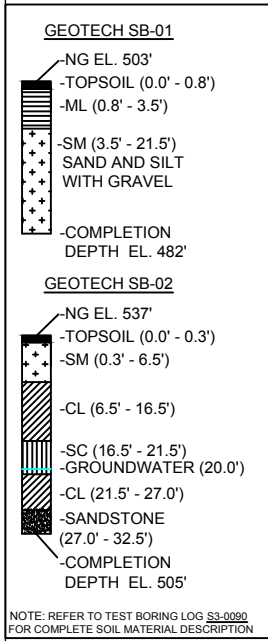
**SEPTEMBER 2017 16-INCH HDD PLAN AND PROFILE
NOVEMBER 2018 2ND REVISED 16-INCH HDD PLAN AND PROFILE**



LEBANON COUNTY, PENNSYLVANIA - CONEWAGO TOWNSHIP
S3-0090-16



PLAN VIEW

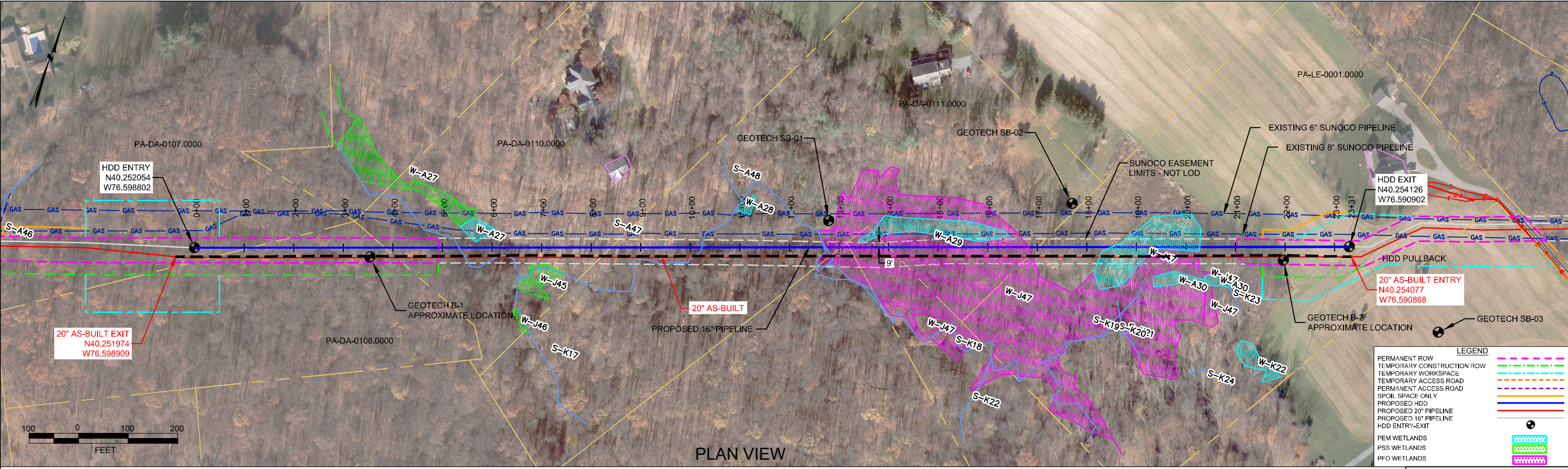
PROFILE VIEW



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
 - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
 - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
 - CROSSING PIPE SPECIFICATION:
HDD HORZ. LENGTH (L=): 2200'
HDD PIPE LENGTH (S=): 2245'
16" x 0.438" W.T., X-70, API 5L, PSL2, ERW, BFW
COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWERCRETE R95)
 - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
 - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
 - CARRIER PIPE NOT ENCASED.
 - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
 - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
 - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
 - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

Figure 1. Revised 16-Inch Plan and Profile 9/2017

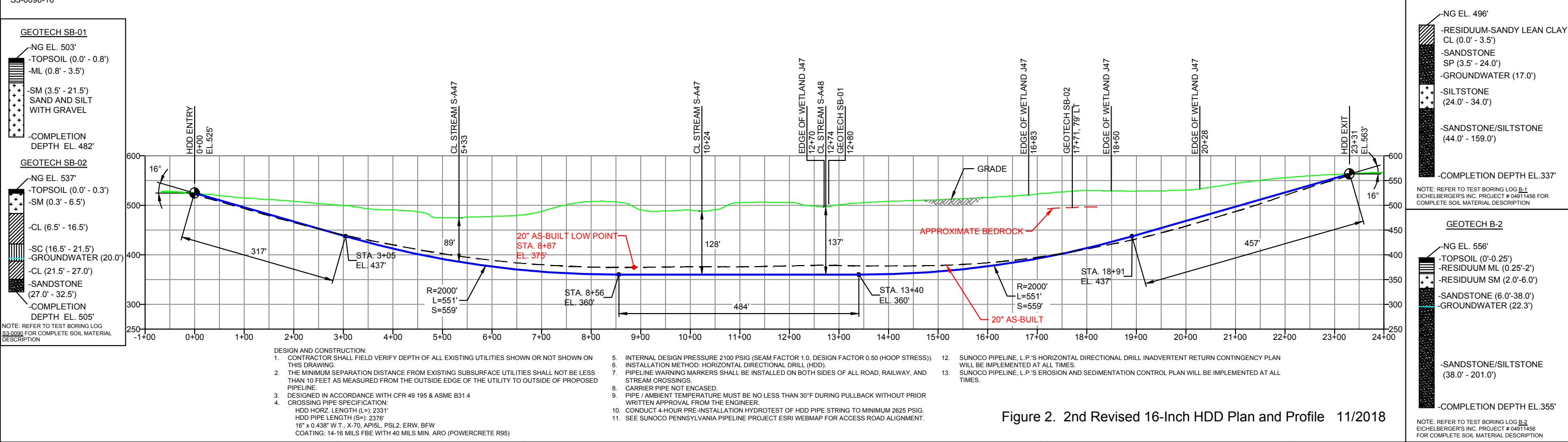
NOTES		REF. DRAWING			REVISIONS										<div><div>Sunoco Logistics Partners L.P.</div></div> <div><div>TETRA TECH ROONEY (303) 792-5911</div></div>		SUNOCO PIPELINE, L.P.		
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83 2. STATIONING IS BASED ON HORIZONTAL DISTANCES. 3. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN. 4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING. 5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.		ES-4.38	TO	ES-5.01	EROSION & SEDIMENT PLAN	EP3	DESIGN CHANGE - EXTENDED DRILL PER LANEY DRILLING AND CM SPREAD 5 APPROVAL			MRS	09/25/17	RMB	09/25/17	AMC			09/25/17	HORIZONTAL DIRECTIONAL DRILL WETLAND PENNSYLVANIA PIPELINE PROJECT	
		SHEET 24	TO	SHEET 1	AERIAL SITE PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09/06/16			MRS	10/07/16	RMB	10/07/16	AAW			10/07/16		
						EP1	REVISED PER PADEP COMMENTS			DLM	05/17/16	RMB	05/17/16	AAW			05/17/16		
						EP				MRS	03/05/16	RMB	03/05/16	AAW			03/05/16		
						B	ADDED GEOTECH INFO			MRS	09/22/15	RMB	09/22/15	AAW	09/22/15				
						A	ISSUED FOR BID			MRS	08/31/15	RMB	08/31/15	AAW	08/31/15				
DWG NO			DWG NO	DESCRIPTION	NO.	DESCRIPTION			BY	DATE	CHK	DATE	APP	DATE	SCALE: 1"=200'		DWG. NO: PA-LE-0001.0000-SR-16		





LEBANON COUNTY, PENNSYLVANIA - CONEWAGO TOWNSHIP
S3-0090-16

PLAN VIEW

PROFILE VIEW



NOTES			REF. DRAWING			REVISIONS										<div><div>Sunoco Logistics Partners L.P.</div><div><div>TETRA TECH ROONEY (303) 792-5911</div></div></div>		SUNOCO PIPELINE, L.P.		HORIZONTAL DIRECTIONAL DRILL WETLAND J47 PENNSYLVANIA PIPELINE PROJECT		SCALE: 1"=200'	DWG. NO: PA-LE-0001.0000-SR-16		
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83			ES-4.38	TO ES-5.01	EROSION & SEDIMENT PLAN	EP5	ADDED 20" AS-BUILT INFORMATION					MRS	11/15/18	RMB	11/15/18									AMC	11/15/18
2. STATIONING IS BASED ON HORIZONTAL DISTANCES			SHEET 24	TO SHEET 1	AERIAL SITE PLAN	EP4	UPDATED TO MATCH 20" DESIGN AND NOTE 5 AND 10 PER INCREASED 16" MOP					MRS	05/17/18	RMB	05/17/18									AMC	05/17/18
3. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.						EP3	DESIGN CHANGE - EXTENDED DRILL PER LANEY DRILLING AND CM SPREAD 5 APPROVAL					MRS	09/25/17	RMB	09/25/17									AMC	09/25/17
4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.						EP2	REVISED PER PADEP COMMENTS RECEIVED 09/06/16					MRS	10/07/16	RMB	10/07/16			AAW	10/07/16						
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.						EP1	REVISED PER PADEP COMMENTS					DLM	05/17/16	RMB	05/17/16			AAW	05/17/16						
						EP						MRS	03/05/16	RMB	03/05/16			AAW	03/05/16						
			DWG NO	DWG NO	DESCRIPTION	NO.	DESCRIPTION					BY	DATE	CHK	DATE			APP	DATE						