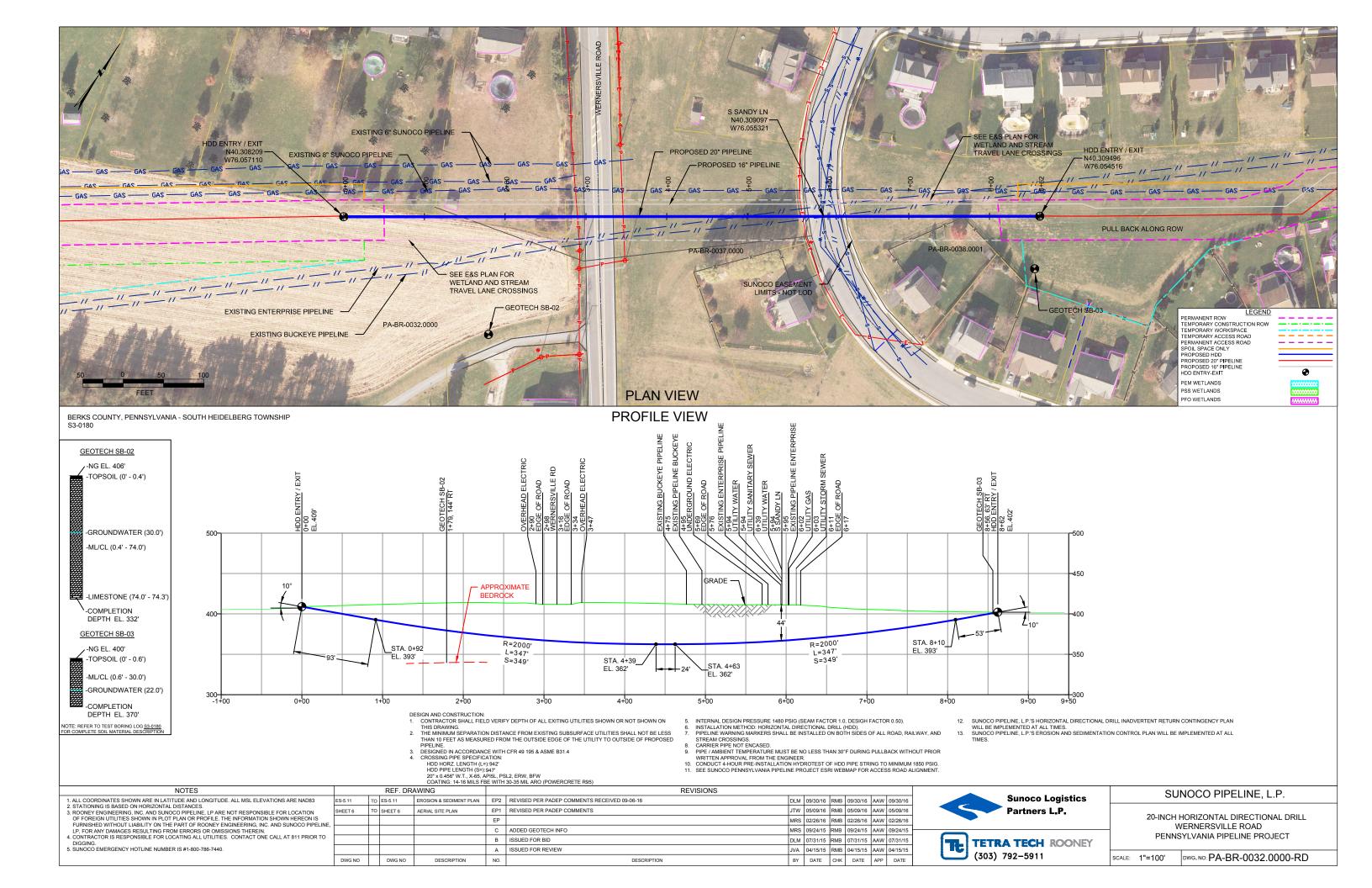
Attachment A HDD Table Berks County

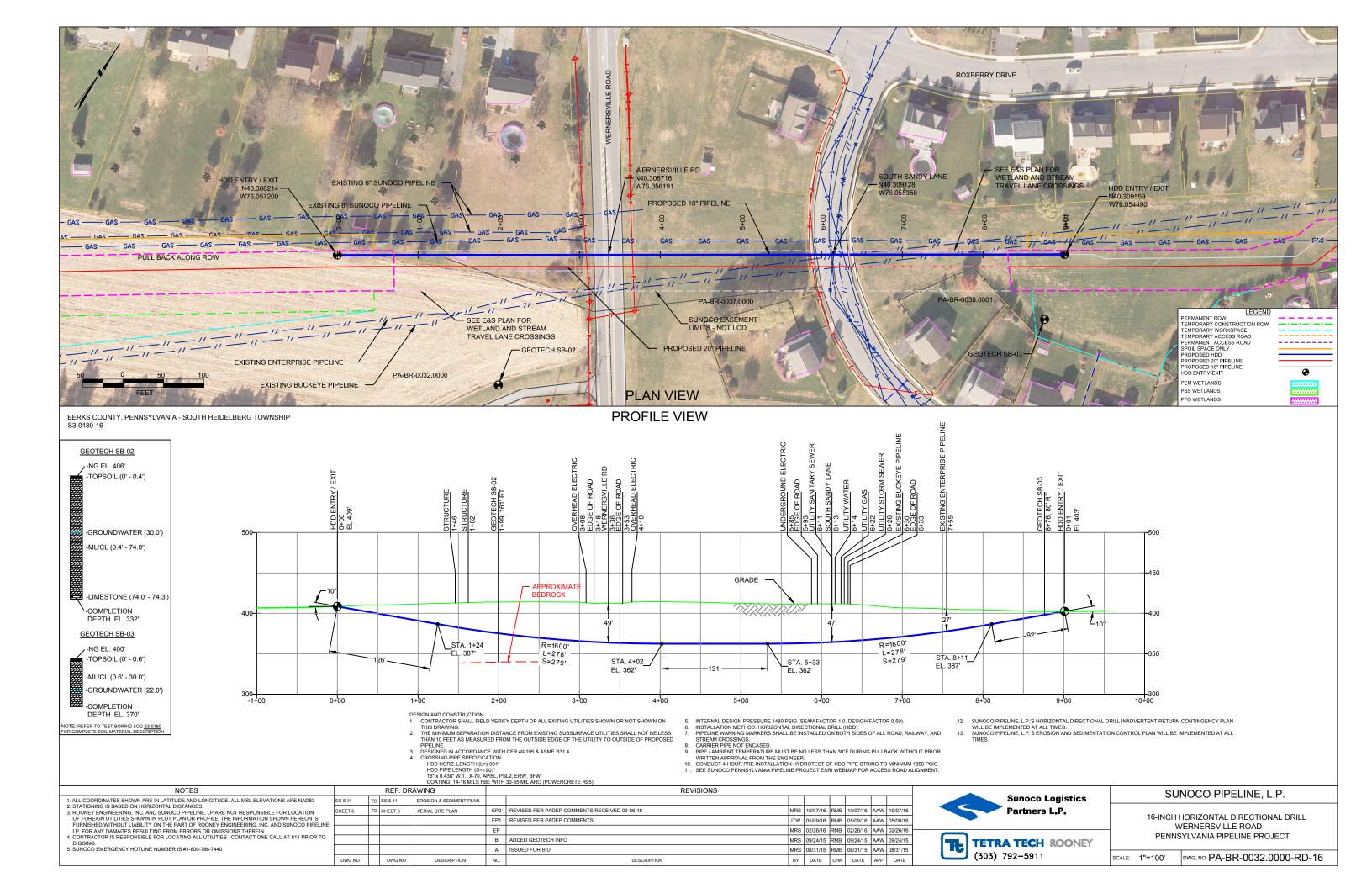
					Risk Assessment Level
Drawing Name	Drill Name	County	Township	Drill Location	(Low / Medium / High)
				N: 40.308209	
PA-BR-0032.0000-RD.pdf	Wernersville Road	Berks	South Heidelberg	W: 76.057110	low
				N: 40.277404	
PA-BR-0075.0000-RD.pdf	HWY 222	Berks	Cumru	W: 76.021026	low
				N: 40.276733	
PA-BR-0079.0000-RD.pdf	Peach Tree Lane	Berks	Cumru	W: 76.014263	low
				N: 40.226330	
PA-BR-0138.0001-RD.pdf	Gerhart School Road	Berks	Brecknock	W: 75.952419	low
				N: 40.170400	
PA-BR-0181.0000-RD.pdf	Joanna Road	Berks	Caernarvon	W: 75.863673	low

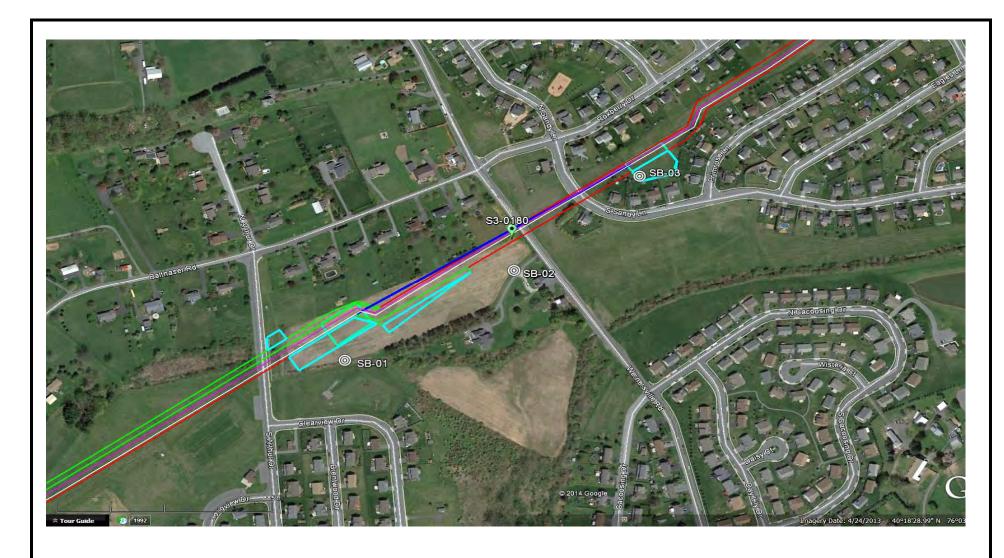
HDD PA-BR-0032.0000-RD (S-A47, S-K18, PFO-J47, PEM-J47)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 370 feet from the western edge of Wernersville Road and enter/exit 560 feet from the eastern edge. The horizontal directional drill will enter/exit 640 feet from the western edge of N. Sandy Lane and enter/exit 270 feet from the eastern edge. The drill will pass approximately 40 feet below both roads. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrates being drilled through are silty clays and fine sands. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.







LEGEND:

© Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S3-0180
BERKS COUNTY, SOUTH HEIDELBERG TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA P	JNOCO PENNSYLVANIA PIPELINE PROJECT							
Project Location:	ARTHUR DRIVE, SOUTH HE	ARTHUR DRIVE, SOUTH HEIDELBERG TWP							
HDD No.:	S3-0180	Dates(s) Drilled: 12-10-14	Inspector:	E. WATT					
Boring No.:	SB-01	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER					
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 26.0	Total Depth (ft):	30.0					
Boring Location Coor	dinates:	40° 18' 25.187" N	76° 3' 31.680" W						

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" I	ncreme	ant Dia	we *	N
No.	From	То	From	То	Rec	(USCS)	Description of waterials	0 11	ICI CITIC	2111 DIO	ws	IN
			0.0	0.0			TOPSOIL (NONE)					
1	3.0	5.0	0.0		11		MOTTLED ORANGE BROWN, LIGHT BROWN, AND GRAY SILTY CLAY	1	2	4	6	6
							WITH SOME FINE SAND, TRACE F-C UNWEATHERED GRAVEL.					
2	8.0	10.0			12		MOTTLED BROWN, LIGHT BROWN, AND GRAY SILTY CLAY WITH	1	1	3	5	4
							SOME F-SAND, TRACE F-C UNWEATHERED GRAVEL. (USCS: CL)					
3	13.0	15.0			20		MOTTLED ORANGE BROWN, LIGHT BROWN, AND GRAY SILTY CLAY	1	2	3	5	5
						CL	WITH A LITTLE F-SAND.					
4	18.0	20.0			24		MOTTLED GRAY AND ORANGE BROWN SILTY CLAY WITH SOME FINE	1	1	2	3	3
							SAND, TRACE FINE UNWEATHERED GRAVEL.					
5	23.0	25.0			24		LIGHT GRAY TO WHITE SILTY CLAY WITH SOME FINE SAND	1	2	5	9	7
6	28.0	30.0		30.0	22		LIGHT GRAY T WHIITE SILTY CLAY, TRACE FINE SAND (USCS: CL).	4	8	7	7	15
								-				
								1				
								-				
								-				
							CAVED AT 29'.	-				
							WATER LEVEL ON CAVE AT 26'.	-				
							VANTER ELVEL ON ONVE AT 20.	-				
								-				
								-				
								-			-	
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										<u> </u>		
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Notes/Comments:

Pocket Pentrometer Testing

S2: 1.25 TSF S5: 2.25 TSF

S3: 1.25 TSF S4: 1.0 TSF DR: DECOMPOSED ROCK

ALL ABOVE SAMPLES ARE DR.

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA	INOCO PENNSYLVANIA PIPELINE PROJECT Project						
Project Location:	WERNERSVILLE ROAD, S	SOUTH HEIDELBERG TWP		Page 1 of 1				
HDD No.:	S3-0180	Dates(s) Drilled: 12-09-14	Inspector:	E. WATT				
Boring No.:	SB-02	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER				
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 30.0	Total Depth (ft):	74.3				
Boring Location Coor	dinates:	40° 18' 29.311" N	76° 3' 22.567" W	76° 3' 22.567" W				
	0 5							

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" 1	orom	ent Blov	wo *	N
No.	From	То	From	То	Rec	(USCS)	Description of Materials	0 11	icrem	EIII DIO	v5	IN
			0.0	0.4			TOPSOIL (5")					
1	3.0	5.0	0.4		18		MOTTLE BROWN AND ORANGE BROWN SLT AND CLAY WITH SOME		2	5	6	7
							FINE SAND, TRACE F-GRAVEL.					
2	8.0	10.0			18		MOTTLED ORANGE TO YELLOW BROWN SILT AND CLAY, AND FINE	4	4	5	5	9
							SAND.					
3	13.0	15.0			22		MOTTLED ORANGE BROWN AND LIGHT GRAY CLAY AND SILT, AND	1	2	6	6	8
							FINE SAND. (USCS: CL/ML)					
4	18.0	20.0			19		MOTTLED ORANGE BROWN AND LIGHT GRAY CLAY AND SILT, AND	2	4	5	5	9
							FINE SAND.					
5	23.0	25.0			24		MOTTLED ORANGE BROWN AND LIGHT GRAY CLAYEY SILT WITH	1	1	3	8	4
						M	SOME FINE SAND. (USCS: ML)					
6	28.0	30.0			24	CL/ML	MOTTLED LIGHT GRAY AND BROWN SILT/CLAY AND FINE SAND,	1	2	3	6	5
						AND	TRACE UNWEATHERED FINE GRAVEL.					
7	33.0	35.0			24	,Ή, Α	MOTTLED LIGHT GRAY AND BROWN SILT/CLAY AND FINE SAND,	3	6	6	8	12
						ML/CL,	TRACE UNWEATHERED FINE GRAVEL.					
8	38.0	40.0			20	ML, N	MOTTLED BROWN AND GRAY SILT/CLAY, AND FINE SAND, TRACE	1	1	4	13	5
						LL	UNWEATHERED FINE GRAVEL.					
9	43.0	45.0			12	IG 01	MOTTLED BROWN AND GRAY CLAY/SILT AND FINE SAND, TRACE	2	5	9	10	14
						띪	UNWEATHERED FINE GRAVEL. (USCS: CL/ML)					
10	48.0	50.0			14	AYE	MOTTLED BROWN AND GRAY CLAY/SILT AND FINE SAND, TRACE	WH	WH	1	1	1
						ERLAYERING	UNWEATHERED FINE GRAVEL.					
11	53.0	55.0			18	Ĕ	MOTTLED (SHADES OF BROWN) SILT/CLAY AND FINE SAND, TRACE	1	3	7	15	10
							UNWEATHERED FINE GRAVEL.					
12	58.0	60.0			17		MOTTLED (SHADES OF BROWN) SILT/CLAY AND FINE SAND, TRACE	11	26	20	15	46
							UNWEATHERED FINE GRAVEL.					
13	63.0	65.0			14		MOTTLED (SHADES OF BROWN) SILT/CLAY AND FINE SAND, TRACE	4	8	10	10	18
							UNWEATHERED FINE GRAVEL.					
14	68.0	70.0			16		MOTTLED (SHADES OF BROWN) SILT/CLAY AND FINE SAND, TRACE	2	5	5	6	10
							UNWEATHERED FINE GRAVEL.					
15	73.0	74.3			12	1	MOTTLED (SHADES OF BROWN) SILT/CLAY AND FINE SAND, TRACE 1 17 50/4"			>50		
				74.0		=	UNWEATHERED FINE GRAVEL.					
			74.0	74.3			GRAY LIMESTONE (BROKEN UP)					

Notes/Comments:

Pocket Pentrometer Testing

WET ON SPOON AT 33'. CAVED AT 67' WATER LEVEL ON CAVE AT 38'. WATER LEVEL THROUGH AUGERS AT 30'.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVA	NIA PIPELINE PROJECT		Project No.: 103IP3406		
Project Location:	S. SANDY LANE, SOU	TH HEIDELBERG TWP	Page 1 of 1			
HDD No.:	S3-0180	Dates(s) Drilled: 12-10-14	Inspector:	E. WATT		
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER		
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 22.0	Total Depth (ft):	30.0		
Boring Location Coor	dinates:	40° 18' 33.618" N	76° 3' 15.803" W	1		

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	. (د	Strata	Description of Materials	6" 1	norom	ont Dia	*	N
No.	From	То	From	То	Recov.	(USCS)	Description of Materials	0.1	ncreme	ent pio	ws	N
			0.0	0.6			TOPSOIL (7")					
1	3.0	5.0	0.6		18		ORANGE BROWN SILT AND CLAY WITH SOME FINE SAND, WITH A	2	8	6	6	14
						4	LITTLE FINE GRAVEL.					
2	8.0	10.0			22	CL/ML	MOTTLED BROWN, ORANGE TO YELLOWISH BROWN SILT AND CLAY	1	2	3	3	5
						Q.	WITH A LITTLE FINE SAND (USCS: ML/CL)					
3	13.0	15.0			24	, A	MOTTLED BROWN, ORANGE BROWN, AND LIGHT GRAY CLAY/SILT	1	2	3	10	5
						∏	AND FINE SAND (USCS: CL/ML)					
4	18.0	20.0			8	Ω	BROWN TO ORANGE BROWN CLAYEY SILT AND FINE SAND, TRACE	1	2	3	2	5
						ERE	WEATHERED FINE GRAVEL.					
5	23.0	25.0			24	INTERLAYERED ML/CL AND	MOTTLED BROWN, ORANGE BROWN, AND LIGHT GRAY CLAY/SILT	2	3	7	9	10
						ËR	AND FINE SAND, TRACE UNWEATHERED FINE GRAVEL.					
6	28.0	30.0			11	Ξ	MOTTLED BROWN, ORANGE BROWN, AND LIGHT GRAY CLAY/SILT	1	1	1	1	2
				30.0			AND FINE SAND, TRACE UNWEATHERED FINE GRAVEL.					
							WET ON SPOON AT 23'.					
							WATER LEVEL THROUGH AUGERS AT 22'.					
							CAVED AT 27'.					
							WATER LEVE ON CAVE AT 22'.					
					<u> </u>							
								1				
					<u> </u>							
								1				

Notes/Comments:

Pocket Pentrometer Testing

S1: 1.5 TSF

DR: DECOMPOSED ROCK

ALL ABOVE SAMPLES ARE DR.

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD \$3-0180

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of S	Sample (ft.)	Content, %	Silts/Clays, %		Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		2	8.0	10.0	22.8	70.3	39	24	15	CL
	SB-01	3	13.0	15.0	32.0	85.4	-	-	-	_
	3D-01	5	23.0	25.0	25.6	79.5	-	-	-	-
		6	28.0	30.0	28.6	97.4	37	20	17	CL
		3	13.0	15.0	34.3	62.4	36	24	12	CL/ML
		5	23.0	25.0	37.3	79.3	44	33	11	ML
	SB-02	7	33.0	35.0	28.9	52.5	-	-	-	-
S3-0180	3D-02	9	43.0	45.0	35.3	63.2	39	25	14	CL/ML
		12	58.0	60.0	23.8	52.0	-	-	-	-
		14	68.0	70.0	25.6	56.9	-	-	-	-
		1	3.0	5.0	23.3	71.7	-	-	-	-
		2	8.0	10.0	44.6	88.0	34	24	10	ML/CL
	SB-03	3	13.0	15.0	30.5	69.5	38	24	14	CL/ML
		5	23.0	25.0	25.3	52.1	-	-	-	-
		6	28.0	30.0	28.8	59.1	-	1	-	-

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD \$3-0170

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S3-0180	/ernersville Roa	SB-02	Millbach Fm - Pinkish gray and medium gray laminated limestone and interbeds of light to medium gray finely crystalline dolomite	level upland	Millbach Fm	Interbedded limestone	1,500	40-79	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u> Very Loose	<u>N (blows)*</u> 5 or less	<u>Particle S</u>	ize Identifica	<u>tion</u>
•	6 to 10	Boulders	8 in. diame	ter or more
Loose		Cobbles	3 to 8 in. di	ameter
Medium Dense Dense	11 to 30 31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
,		Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
<u>Description Term</u>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	-, ,		,

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	<i>5 , 5</i>	

ROCK (Rock Cores)

Rock	Rock
Quality Designation	Quality <u>Descripti</u>
(RQD), %	<u>on</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

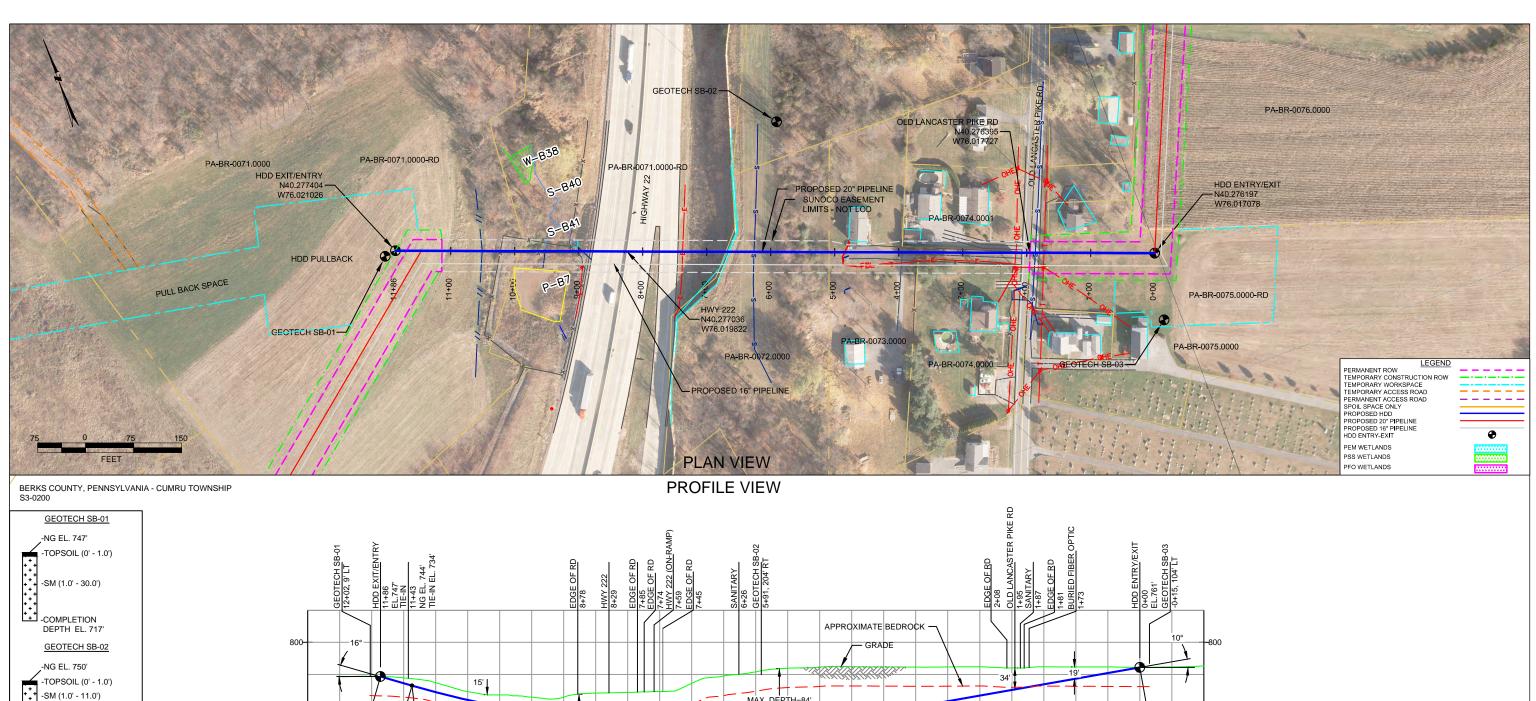
	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C _u or C _c requiren	nents for GW
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size or than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from tion smaller assified as fo	W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols
Coarse Grained Soils f material is larger tha	maller than	ands to fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (frac ed soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c required	ments for SW
N)	half of coa	n fines able fines)	SM	Silty sands, sand- silt mixtures	Determ		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils p When w _L	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	A Line:		
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50	U Line:	0.73(LL - 20) 0.9(LL - 8)	Or I
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %			, or oth
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		13/18/	MH or OH
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	blasi		Culton	
(More than	Silts ar 9	ОН	Organic clays	s of medium to high anic silts	7 4	<u> </u>	ML or OL 20 30 40 50 6	0 70 80 90 100
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL	

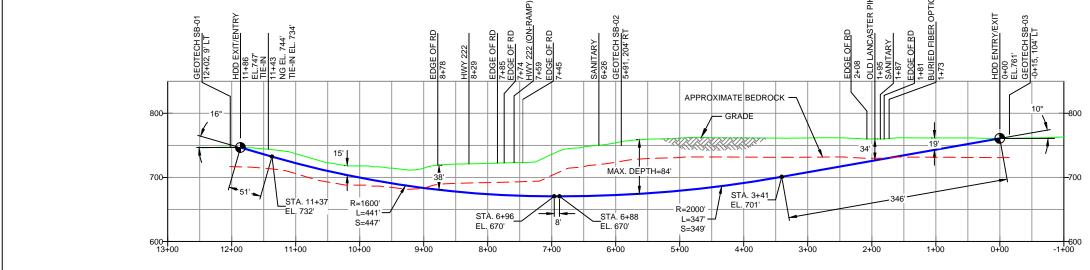
⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

HDD PA-BR-0075.0000-RD (S-B41, S-B40)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 190 feet from the western edge of Stream B41 (S-B41) and enter/exit 960 feet from the eastern edge. The horizontal directional drill will enter/exit 240 feet from the western edge of Stream B40 (S-B40) and enter/exit 910 feet from the eastern edge. The drill will also enter/exit 270 feet from the western edge of Highway 222 and enter/exit 740 feet from the eastern edge. The drill will pass 15 feet underneath S-B41, 20 feet beneath S-B42, and 45 feet below Highway 222. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrates being drilled through are fractured conglomerate of quartz and sandstone with layers of sandy clays and silts above the drill. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.





DESIGN AND CONSTRUCTION: CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON

- THIS DRAWING THIS DRAWING.

 THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.

 DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4

 CROSSING PIPE SPECIFICATION:
 HDD HORZ. LENGTH (L=):1186'
 HDD PIPE LENGTH (L=):1201'
 20" x 0.456" W.T., X-65, APISL, PSL2, ERW, BFW
 COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)

-SM/GM (11.0' - 24.0')

-FRACTURED

(24.0' - 32.0')

\-COMPLETION

DEPTH EL. 718' **GEOTECH SB-03** -NG EL. 767' -TOPSOIL (0' - 0.7')

-SM (0.7' - 30.0')

-COMPLETION DEPTH EL. 737' NOTE: REFER TO TEST BORING LOG S3-0290

CONGLOMERATE

- 5. INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0. DESIGN FACTOR 0.50).
- INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND
- STREAM CROSSINGS.

 8. CARRIER PIPE NOT ENCASED.

 9. PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

- 12. SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL

FO	OR COMPLETE SOIL MATERIAL DESCRIPTION				COATING: 14-16 MILS FB	E WITH	30-35 MIL ARO (POWERCRETE R95)						
	NOTES			REF. DRA	AWING		REVISIONS						
	1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-5.27	то Е	ES-5.28	EROSION & SEDIMENT PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	DLM	09/30/16	RMB	09/30/16	AAW 09	/30/16
	STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 15	TO S	SHEET 16	AERIAL SITE PLAN	EP1	REVISED PER PADEP COMMENTS	JTW	05/09/16	RMB	05/09/16	AAW 05	/09/16
	OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINI					EP		MRS	11/23/15	RMB	11/23/15	AAW 11	/23/15
	LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.	´				С	ADDED GEOTECH INFO	MRS	09/25/15	RMB	09/25/15	AAW 09	/25/15
4	 CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING. 					В	ISSUED FOR BID	DLM	07/31/15	RMB	07/31/15	AAW 07	/31/15
5	5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR REVIEW	КВ	04/15/15	RMB	04/15/15	AAW 04	/15/15
		DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	СНК	DATE	APP I	DATE



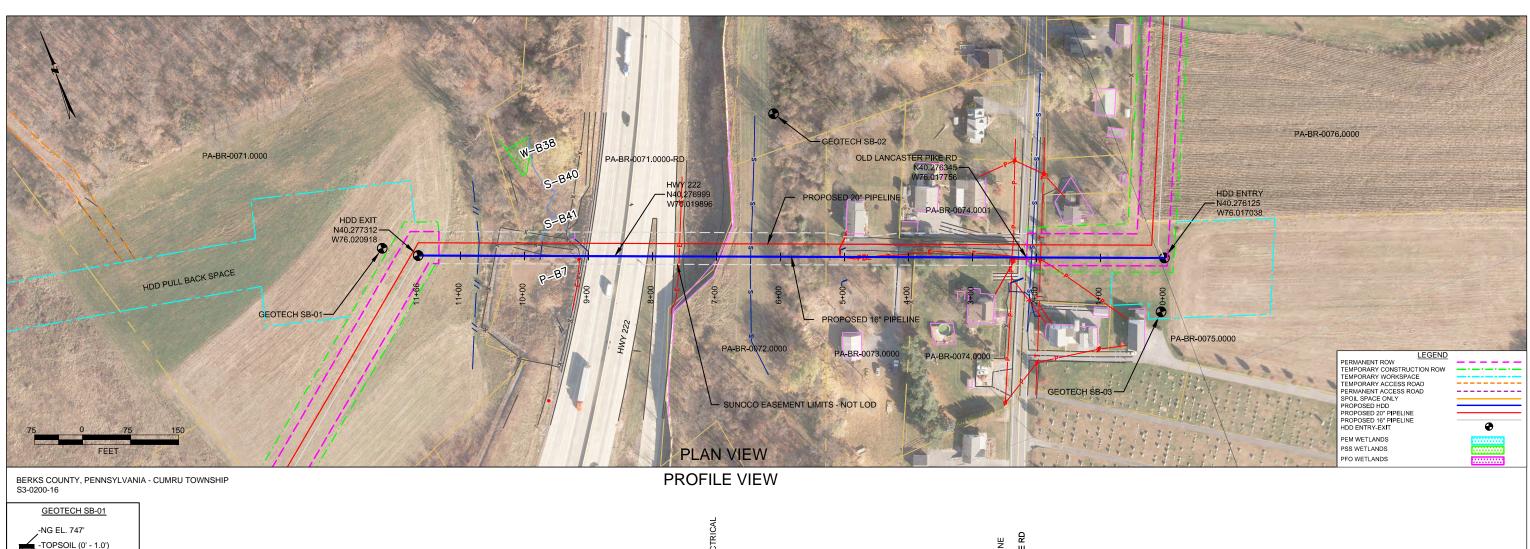
(303) 792-5911

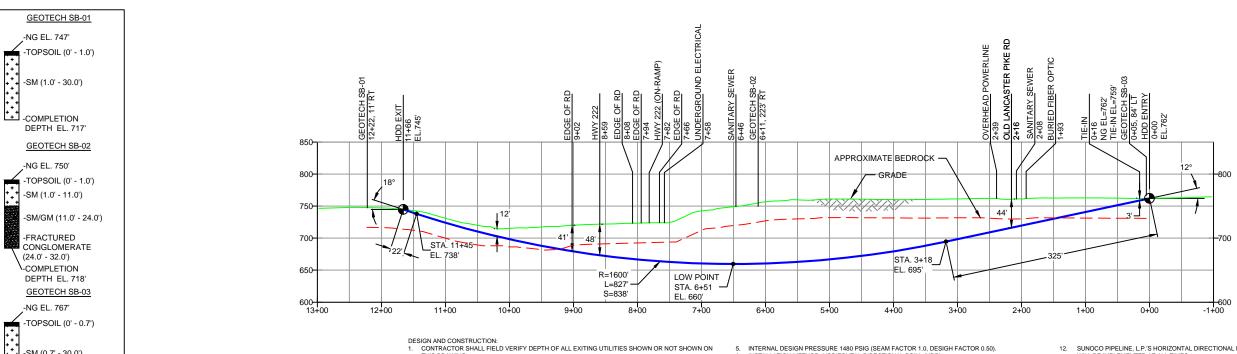
TETRA TECH ROONEY

20-INCH HORIZONTAL DIRECTIONAL DRILL HWY 222 PENNSYLVANIA PIPELINE PROJECT

SUNOCO PIPELINE, L.P.

DWG. NO: PA-BR-0075.0000-RD SCALE: 1"=150'





THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS

2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.

3. DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4

4. CROSSING PIPE SPECIFICATION:
HDD HORZ, LENGTH (L=):1186'
HDD PIPE LENGTH (S=):1186'
16" × 0.438" W.T., X-70, APISL, PSL2, ERW, BFW
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95) NOTE: REFER TO TEST BORING LOG S3-0290 FOR COMPLETE SOIL MATERIAL DESCRIPTION NOTES REF. DRAWING REVISIONS

-SM (0.7' - 30.0')

-COMPLETION DEPTH EL. 737'

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50). 12. SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN INTERNAL DESIGN PRESSURE 1400 PSIG (SERIM PACTOR IT, U) BESIGN FACTOR U.SUJ.
 INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND
 STREAM GROSSINGS. WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL
- STREAM CROSSINGS.

 8. CARRIER PIPE NOT ENCASED.

 9. PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- - **Sunoco Logistics** Partners L.P.

(303) 792-5911

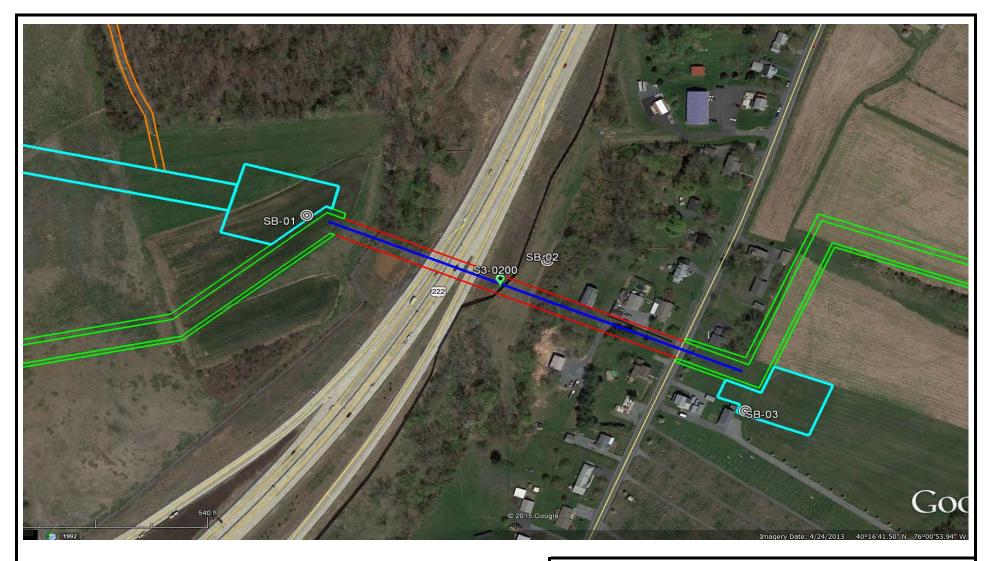
16-INCH HORIZONTAL DIRECTIONAL DRILL HWY 222 PENNSYLVANIA PIPELINE PROJECT

SUNOCO PIPELINE, L.P.

DWG. NO: PA-BR-0075.0000-RD-16 SCALE: 1"=150'

1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-5.27	то	ES-5.28	EROSION & SEDIMENT PLAN			1 1				1 1	1 1
STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 15	то	SHEET 16	AERIAL SITE PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	MRS	10/07/16	RMB	10/07/16	AAW	10/07/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE,					EP1	REVISED PER PADEP COMMENTS	JTW	05/09/16	RMB	05/09/16	AAW	05/09/16
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					EP		MRS	11/23/15	RMB	11/23/15	AAW	11/23/15
 CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING. 					В	ADDED GOETECH INFO	MRS	09/25/15	RMB	09/25/15	AAW	09/25/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR BID	MRS	08/31/15	RMB	08/31/15	AAW	08/31/15
	DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE

TETRA TECH ROONEY



LEGEND:

Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S3-0200
BERKS COUNTY, CUMRU TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA P	IPELINE PROJECT		Project No.: 1	03IP3406	
Project Location:	OLD LANCASTER PIKE, CUM	MRU, PA		Page 1 of 1		
HDD No.:	S3-0200	Dates(s) Drilled: 12-11/12-14	Inspector:	E. WATT		
Boring No.:	SB-01	Drilling Method: SPT - ASTM D1586	S. HOFFER			
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0		
Boring Location Coord	linates:	40° 16' 38.638" N	76° 1' 15.864" W			
Sample Depth (ft)) Strata Depth (ft) ≥ Strata					

Domie	Location						10 10 00:000 11					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials	6" lı	ncreme	nt Blo	WC *	N
No.	From	То	From	То	Rec (ir	(USCS)	Description of Materials	U II	iciente	пі БіО	vvS	IN
			0.0	1.0			TOPSOIL (12")					
1	3.0	5.0	1.0		17		MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	8	14	22	15	36
							GRAVEL/CONGLOMERATE MATRIX, SOME SILT.					
2	8.0	8.7			6		MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	50	50/2"			>50
							GRAVEL/CONGLOMERATE MATRIX, SOME SILT.					
3	13.0	13.9			8		MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	16	50/5"			>50
						014	GRAVEL/CONGLOMERATE MATRIX, A LITTLE SILT.					
4	18.0	18.8			5	SM	MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	50	50/3"			>50
							GRAVEL/CONGLOMERATE MATRIX, A LITTLE SILT.					
5	23.0	25.0			24		MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	3	31	48	25	79
							GRAVEL/CONGLOMERATE MATRIX, A LITTLE SILT.			 		
6	28.0	28.4			6		MAROON FINE TO COARSE SAND WITH A LITTLE FINE TO COARSE	50/5"		 		0
				30.0			GRAVEL/CONGLOMERATE MATRIX, SOME SILT.					
							AUGERED TO 30'.					
							GRINDING BEGINS 5' TO 6', THEN OFF AND ON BETWEEN					
							10 TO 28'.					
							CAVED AND DRY AT 27'.			 		
					1]						<u> </u>

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYL\	/ANIA PI	PELINE PROJECT		Project No.: 103IP3406	
Project Location:	OLD LANCASTER PI	KE, CUM	IRU, PA		Page 1 of 1	
HDD No.:	S3-0200		Dates(s) Drilled: 02-10-15	Inspector:	E. WATT	
Boring No.:	SB-02		Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER	
Drilling Contractor:	HAD DRILLING		Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	32.0	
Boring Location Coor	dinates:		40° 16' 38.348" N	76° 1' 7.530" W		
Sample Donth /f	t) Strata Depth (ft)	Stroto				

Domig	Location	ii Oooiaii	iatoo.				10 10 00:010 11					
Sample	-	Depth (ft)	-	Depth (ft)	Recov.	Strata	Description of Materials	6" l	ncreme	ent Blo	ws *	N
No.	From	То	From 0.0	To 0.7	8 -	(USCS)	TOPSOIL (8")				T	
4	2.0	F 0		0.7	17		· '	19	26	21	42	47
1	3.0	5.0	0.7		17		MAROON FINE TO COARSE SAND WITH A LITTLE SILT, AND WITH	19	26	21	42	47
		0.7				SM	SOME FINE TO COARSE GRAVEL.		50/01	-	-	
2	8.0	8.7			6		REDDISH BROWN MEDIUM COARSE SAND WITH SOME CONGLOMERAT	34	50/2"		-	>50
				11.0			GRAVEL, AND A LITTLE SILT.					
							AUGER REFUSAL AT 10'. OFFSET 8' NORTH AND CONTINUOUSLY					
							AUGERED TO NEXT SAMPLE INTERVAL.					
3	12.0	13.7	11.0		8		REDDISH BROWN MEDIUM TO COARSE SAND WITH SOME	10	50/3"			>50
3	13.0	13.7	11.0		0			10	50/3		-	>50
						SM/	CONGLOMERATE GRAVEL, TRACE ROCK FRAGMENTS, LITTLE SILT.				-	
4	18.0	18.3			5	GM	REDDISH BROWN MEDIUM TO COARSE SAND WITH SOME	50/4"				>50
							CONGLOMERATE GRAVEL, TRACE ROCK FRAGMENTS, LITTLE SILT.				<u> </u>	
5	23.0	23.1		24.0	24		REDDISH BROWN CONGLOMERATE GRAVEL.	50/1"			<u> </u>	>50
							AUGER REFUSAL AT 24'.	-				
							AUGLINICI USAL AT 24.					
							ROCK CORING					
RUN 1	24.0	27.0	24.0	27.0	21	GLO I.	REDDISH BROWN INTENSELY FRACTURED CONGLOMERATE.	TCR: 5	8%, SCF	R: 8%, F	₹QD: 0%	6
RUN 1	27.0	32.0	27.0	32.0	22	CONGLO M.	REDDISH BROWN INTENSELY FRACTURED CONGLOMERATE.	TCR: 3	7%, SCF	R: 17%,	RQD: 0	%
							CAVED AND DRY AT 23'.	-	_			
												<u> </u>
								<u> </u>				
]								 				
								<u> </u>				

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

N: Number of blows to drive spoon from 6" to 18" interval.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI	PELINE PROJECT		Project No.: 103IP3406
Project Location:	OLD LANCASTER PIKE, CUM	IRU, PA		Page 1 of 1
HDD No.:	S3-0200	Dates(s) Drilled: 12-11-14	Inspector:	E. WATT
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0
Boring Location Coordi	nates:	40° 16' 33.303" N	76° 1' 1.736" W	

Boring	Location	n Coordin	iates:				40° 16′ 33.303″ N					
Sample	-	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials	6" I	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Re	(USCS)	·					
			0.0	0.7			TOPSOIL (8")				ļ	
1	3.0	5.0	0.7		10		MAROON FINE SAND AND SILT, TRACE CONGLOMERATE MATRIX.	2	9	9	12	18
2	8.0	10.0			11		MAROON FINE TO MEDIUM SAND WITH SOME SILT, TRACE FINE	1	5	15	20	20
							TO COARSE GRAVEL.					
3	13.0	14.5			15		MAROON FINE TO MEDIUM SAND AND SILT, WITH A LITTLE FINE	312	35	35		70
1						014	TO COARSE GRAVEL. (USCS: SM).					
4	18.0	19.4			15	SM	MAROON FINE TO MEDIUM SAND AND SILT, WITH A LITTLE FINE	7	8	50/5"		>50
·							TO COARSE GRAVEL.					
5	23.0	23.8			7		MAROON FINE TO MEDIUM SAND AND SILT, WITH A LITTLE FINE	20	50/3"			>50
							TO COARSE GRAVEL.					
6	28.0	28.7			5		MAROON FINE TO MEDIUM SAND WITH SOME SILT, WITH A LITTLE FINE	30	50/2"			>50
				30.0			TO COARSE GRAVEL.					
							AUGERED TO 30'.					
							CAVED AND DRY AT 20'.					
							SAVES AND SIAT ALL EST					
- 												
												-
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										<u> </u>	<u> </u>	

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD \$3-0200

	Test				Water	Percent	Atterburg	Limits (AS	STM D4318)	USCS
HDD	Boring	Sample	Depth of	Sample (ft.)	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	To	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		2	8.0	8.7	8.5	24.0	-	-	-	-
	SB-01	3	13.0	13.9	5.8	16.8	-	-	-	-
	36-01	5	23.0	25.0	7.3	19.3	-	-	-	-
		6	28.0	28.4	8.7	31.9	-	-	-	-
		1	3.0	5.0	6.3	19.3	-	-	-	-
S3-0200	SB-02	2	8.0	8.7	5.7	12.6	-	-	-	-
33-0200		3	13.0	13.7	9.6	20.2	-	-	-	-
		1	3.0	5.0	11.6	47.1	-	-	-	-
		2	8.0	10.0	8.4	28.2	-	-	-	-
	SB-03	3	13.0	14.5	8.4	40.6	33	25	8	SM
		5	23.0	23.8	8.0	38.7	-	-	-	-
		6	28.0	28.7	4.5	26.8	-	•	-	-

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0200

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S3-0200	Hwy 222 - Lancaster Pike	SB-02	Hammer Creek Conglomerate - very coarse quartz conglomerate having abundant pebbles and cobbles of gray quartzite.	level-rolling upland	Conglomerate	quartz conglomerate; reddish brown cross-bedded sandstone	2,580	10-60	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0200

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
S3-0200	SB-2	1	24	27	58	8	0	24	27	Slight to moderate	Conglomerate	Massive	Light Red	Heavily fractured in bottom half of core; fractures ranging from 0° to 65°, Avg. 25°
33-0200	36-2	2	27	32	37	17	0	27	32	Slight to moderate	Conglomerate	Massive	_	Fractures ranging from 0° to 30°, Avg. 9°

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u> Very Loose	<u>N (blows)*</u> 5 or less	<u>Particle S</u>	ize Identifica	<u>tion</u>
•	6 to 10	Boulders	8 in. diame	ter or more
Loose		Cobbles	3 to 8 in. di	ameter
Medium Dense Dense	11 to 30 31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
,		Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
<u>Description Term</u>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	-, ,		,

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	<i>5 , 5</i>	

ROCK (Rock Cores)

Rock	Rock
Quality Designation	Quality <u>Descripti</u>
(RQD), %	<u>on</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

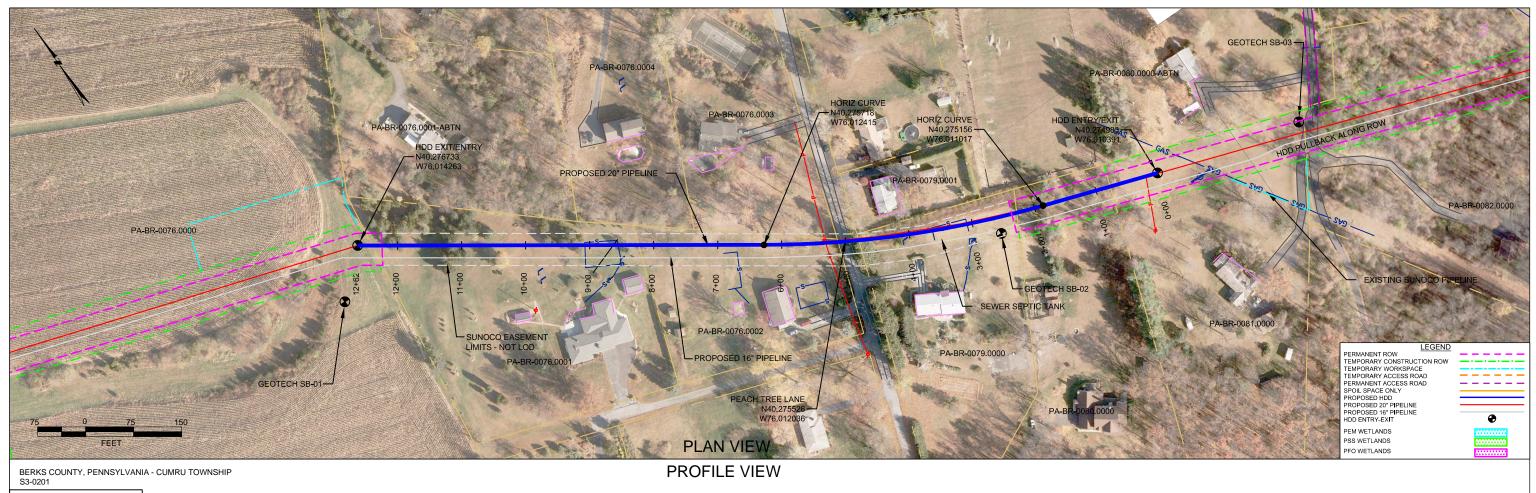
	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C _u or C _c requiren	nents for GW
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size or than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from tion smaller assified as fo	W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols
Coarse Grained Soils f material is larger tha	maller than	ands to fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (frac ed soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c required	ments for SW
N)	half of coa	n fines able fines)	SM	Silty sands, sand- silt mixtures	Determ		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils p When w _L	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	A Line:		
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50	U Line:	0.73(LL - 20) 0.9(LL - 8)	Or I
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %			, or oth
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		13/18/	MH or OH
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	blasi		Culton	
(More than	Silts ar 9	ОН	Organic clays	s of medium to high anic silts	7 4	<u> </u>	ML or OL 20 30 40 50 6	0 70 80 90 100
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL	

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

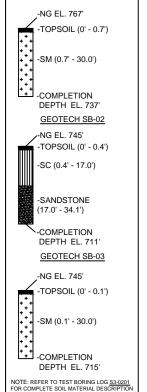
HDD PA-BR-0079.0000-RD

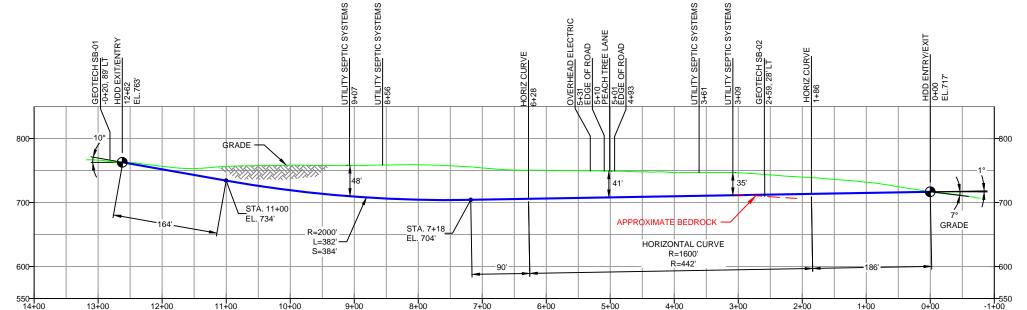
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 830 feet from the western edge of Peach Tree Lane and enter/exit 670 feet from the eastern edge. The drill will pass 63 feet below the road. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrates being drilled through are sandstones with layers of clays and silty sands above the rock. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.









- DESIGN AND CONSTRUCTION:

 1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON
- THIS DRAWING.

 THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.

 DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4

 CROSSING PIPE SPECIFICATION:
 HDD HORZ. LENGTH (L=):1262'
 HDD PIPE LENGTH (S=):1266'
 20" X 0.456" W.T., X-65, APISL, PSL2, ERW, BFW
 COATING: 14.16 MIS SEPS WITH 30.35 MIL ARO (POWERCRETE R95)

- 5. INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50).
- INTERNAL DESIGN PRESSURE 1400 PSIG (SERIM PACTOR IT, U) BESIGN FACTOR U.SUJ.
 INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND
 STREAM GROSSINGS.

- STREAM CROSSINGS.

 8. CARRIER PIPE NOT ENCASED.

 9. PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- 12. SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.

 13. SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL

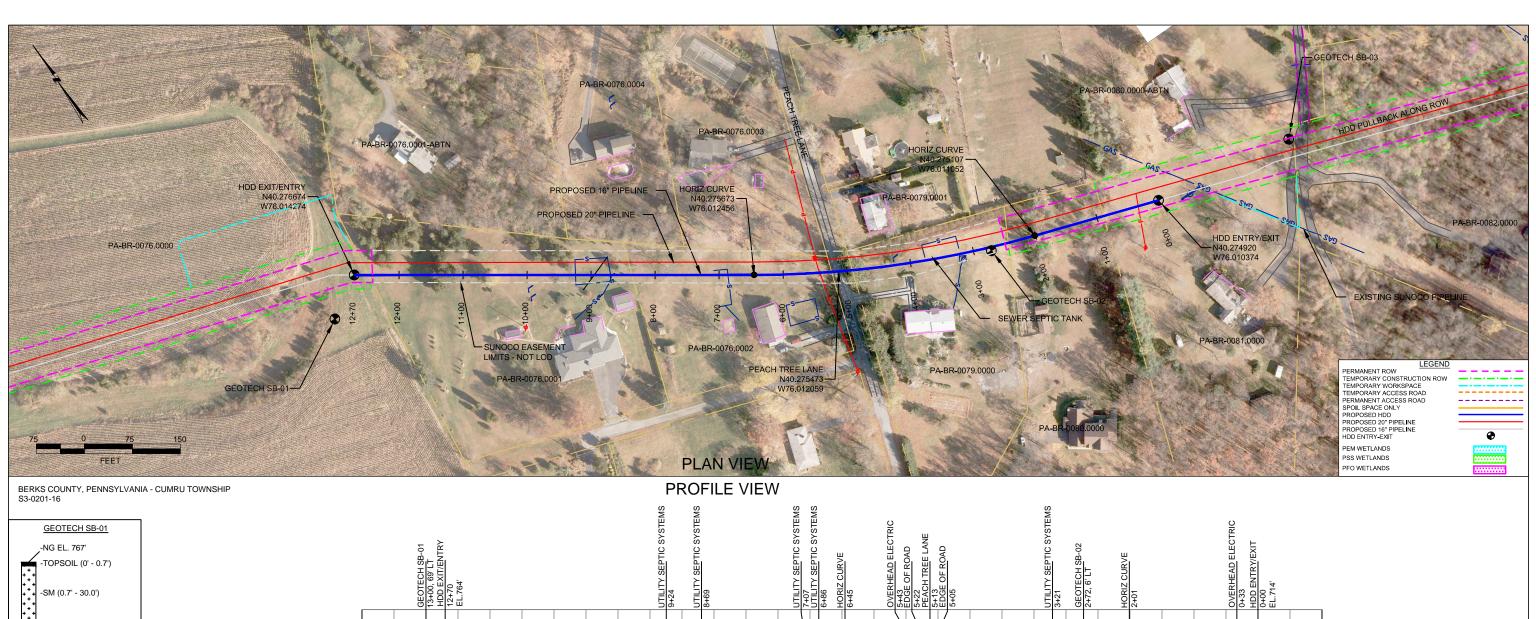
					20 AUSG W.I., AND AFIGE FOLL ELW, B.W. COATING: 14-16 MILS F8E WITH 30-35 MIL ARO (POWERCRETE R95)							
NOTES		REF. DRAWING REVISIONS										
ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-5.28	TO ES-5.2	29	EROSION & SEDIMENT PLAN	EP1	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	DLM	09/30/16	RMB	09/30/16	AAW 0	9/30/16
STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 16	TO SHEE	T 16	AERIAL SITE PLAN	EP1	REVISED PER PADEP COMMENTS	JTW	05/09/16	RMB	05/09/16	AAW 0	5/09/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE.					EP		MRS	11/23/15	RMB	11/23/15	AAW 1	1/23/15
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					MB	ISSUED FOR BID/DESIGN CHANGE	DLM	08/22/15	RMB	08/22/15	AAW 0	8/22/15
4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.					В	ISSUED FOR BID	DLM	07/31/15	RMB	07/31/15	AAW 0	7/31/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR REVIEW	КВ	04/15/15	RMB	04/15/15	AAW 0	4/15/15
	DWG NO	DW	G NO	DESCRIPTION	NO	DESCRIPTION	BY	DATE	CHK	DATE	ΔPP	DATE

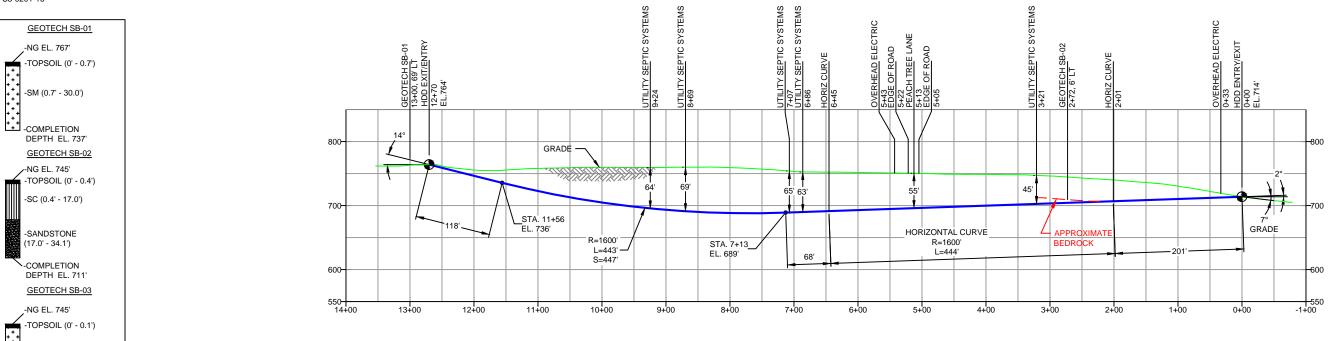


TETRA TECH ROONEY (303) 792-5911

SUNOCO PIPELINE, L.P. 20-INCH HORIZONTAL DIRECTIONAL DRILL PEACH TREE LANE PENNSYLVANIA PIPELINE PROJECT

DWG. NO: PA-BR-0079.0000-RD SCALE: 1"=150'





-SM (0.1' - 30.0')

DEPTH EL. 715'

-COMPLETION

- DESIGN AND CONSTRUCTION:

 1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.

 2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED DIDELING.
- PIPELINE.
 DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- CROSSING PIPE SPECIFICATION:
 HDD HORZ. LENGTH (L=):1270'
 HDD PIPE LENGTH (S=):1278'
 16" x 0.438" W.T., X-70, API5L, PSL2, ERW, BFW

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50).
 INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND
 STREAM CROSSINGS.
 CARRIER PIPE NOT ENCASED.
 PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR

- WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNDOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

SCALE: 1"=150'

SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

NOTE: REFER TO TEST BORING LOG \$3-0201				16" x 0.438" W.T., X-70, A		L2, ERW, BFW	LINE FIX	JULUI LU	IXI WED	NVIAL LOK	ACCES	3 KOAD /
FOR COMPLETE SOIL MATERIAL DESCRIPTION				COATING: 14-16 MILS FE	BE WITH	30-35 MIL ARO (POWERCRETE R95)						
NOTES			REF. DR	AWING		REVISIONS						
ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-5.28	то	ES-5.29	EROSION & SEDIMENT PLAN								1
STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 16	то	SHEET 16	AERIAL SITE PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	MRS	10/07/16	RMB	10/07/16	AAW	10/07/16
OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE					EP1	REVISED PER PADEP COMMENTS	JTW	05/09/16	RMB	05/09/16	AAW	05/09/16
LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.	·				EP		DLM	11/23/15	RMB	11/23/15	AAW	11/23/15
4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.					В	ADDED GEOTECH INFO	MRS	09/25/15	RMB	09/25/15	AAW	09/25/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					А	ISSUED FOR BID	MRS	08/31/15	RMB	08/31/15	AAW	08/31/15
	DWG NO	Т	DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	СНК	DATE	APP	DATE



16-INCH HORIZONTAL DIRECTIONAL DRILL PEACH TREE LANE

SUNOCO PIPELINE, L.P.

TETRA (303) 7	TECH 92-5911	ROONEY

PENNSYLVANIA PIPELINE PROJECT

DWG. NO: PA-BR-0079.0000-RD-16



LEGEND:

© Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S3-0201 PEACH TREE LANE
BERKS COUNTY, CUMRU TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project I	Name:	SUNOCO PENN	SYLVA	NIA PI	PELINE PROJECT		Projec	: No.: 103IP3406	
Project I	Location:	OLD LANCASTE	R PIKE	, REA	DING, PA		Page 1	of 1	
HDD No).:	S3-0201			Dates(s) Drilled: 05-08-15	Inspector:	E. WA	ГТ	
Boring N	No.:	SB-01			Drilling Method: SPT - ASTM D1586	Driller:	S. HO	FER	
Drilling (Contractor:	HAD DRILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0		
Boring L	ocation Coordin	nates:			40° 16' 35.656" N	76° 0' 52.158" W			

	То	Recov. (in)		Description of Materials					
0.0		Ľ.	(USCS)	Description of Materials 6" Increment Blows		vs	N		
	0.7			TOPSOIL (8")					
0.7		16		REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE	2	4	4	8	8
				FINE TO COARSE GRAVEL. (USCS: SM)					
)		10		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH	15	21	25	35	46
				A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).					
1		13		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH	4	35	50/5"		>50
			CNA	A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).					
ļ.		15	SIVI	REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE	8	25	50/5"		>50
				FINE GRAVEL (TRACE CONGLOMERATE MATRIX (USCS: SM).					
)		24		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT	13	27	30	40	57
				A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).					
)		24		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT	5	4	6	18	10
30	30.0			A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).					
				DRY AND CAVED AT 23'.					
	0 4 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 0	4 13 4 15 0 24	4 13 4 15 0 24 0 24 30.0	REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX (USCS: SM). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT	REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX (USCS: SM). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).	REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH 15 21 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH 4 35 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE 8 25 FINE GRAVEL (TRACE CONGLOMERATE MATRIX (USCS: SM). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT 13 27 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT 5 4 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).	10	REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH 15 21 25 35 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND WITH 4 35 50/5" A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND AND SILT, WITH A LITTLE 8 25 50/5" FINE GRAVEL (TRACE CONGLOMERATE MATRIX (USCS: SM). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT 13 27 30 40 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX). REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND WIT 5 4 6 18 A LITTLE FINE GRAVEL (TRACE CONGLOMERATE MATRIX).

Notes/Comments:

Pocket Pentrometer Testing

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENN	SYLVA	NIA PI	PELINE PROJECT		Project	No.: 103IP3406	
Project Location:	PEACH TREE LA	ANE, R	EADIN	G, PA		Page 1	of 1	
HDD No.:	S3-0201			Dates(s) Drilled: 05-07-15	Inspector:	E. WA	ГТ	
Boring No.:	SB-02			Drilling Method: SPT - ASTM D1586	Driller:	S. HO	FER	
Drilling Contractor:	HAD DRILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	34.1		
Boring Location Coordi	nates:			40° 16' 30.595" N	76° 0' 40.609" W	,		
								1

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" lı	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Re	(USCS)	·					<u> </u>
			0.0	0.4			TOPSOIL (5")					
1	3.0	5.0	0.4				DARK BROWN FINE TO MEDIUM SAND WITH SOME SILTY CLAY, LITTLE	4	7	6	5	13
							FINE GRAVEL.					
2	8.0	10.0				SC	REDDISH BROWN FINE SAND AND SILTY CLAY (DECOMPOSED	4	15	10	12	25
						30	SILTSTONE) (USCS: SC), MICACEOUS.					
3	13.0	14.9					REDDISH BROWN FINE SAND AND SILTY CLAY (DECOMPOSED	8	16	15	50/5"	31
				17.0			SILTSTONE) MICACEOUS.					
4	18.0	18.5	17.0			ED	PARTIALLY WEATHERED SANDSTONE.	50/6"				>50
						ËR						
5	23.0	23.5				¥ .	PARTIALLY WEATHERED SANDSTONE.	50/6"				>50
						.Y WEA						
6	28.0	28.0				거꼾	NO RECOVERY	50/0"				>50
						TIAI						
7	33.5	34.1		34.1		PARTIALLY WEATHERED ROCK	PARTIALLY WEATHERED CONGLOMERATE.	17	50/1"			>50
-						ш						
							AUGER REFUSAL AT 33.5'.					
							AGGERTE GOVE AT 66.5.					
							DRY AND CAVED AT 32.5'.					
							DRT AND CAVED AT 32.5.		-	-		
									-	-	-	
									<u> </u>	<u> </u>		
									L	L		

Notes/Comments:

Pocket Pentrometer Testing

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVA	NIA PIPELINE PROJECT		Project No.: 103IP3406	
Project Location:	45 SCARLETT LANE, R	EADING, PA		Page 1 of 1	
HDD No.:	S3-0201	Dates(s) Drilled: 10-08-15	J. COSTELLO		
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	E. ODGEN	
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0	
Boring Location Coor	dinates:	40°16'29.31"N	40°16'29.31"N 76° 0'34.43"W		

-		n Oooran					10 10 20:01 14															
Sample					Strata Depth (ft)		Strata Depth (ft)		Strata Depth (ft)		(ft) Strata Depth (ft)		le Depth (ft) Strata Depth (ft)		Recov.	Strata	Description of Materials	6" I	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Re	(USCS)	·					ļ										
			0.0	0.1			TOPSOIL (TRACE)			<u> </u>												
1	3.0	5.0	0.1		20		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, INTER	5	27	21	40	48										
							LAYERED WITH PARTIALLY WEATHERED ROCK SEAMS.			<u></u>												
2	8.0	10.0			16		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND	4	14	10	12	24										
							WITH A LITTLE F-C SANDSTONE/CONGLOMERATE GRAVEL. (SM)															
3	13.0	15.0			16		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND	1	6	12	15	18										
						CNA	WITH TRACE F-C SANDSTONE/CONGLOMERATE GRAVEL.															
4	18.0	20.0			18	SM	REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND	5	21	41	40	62										
							WITH SOME F-C SANDSTONE/CONGLOMERATE GRAVEL.															
5	23.0	25.0			20		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, AND	6	15	21	31	36										
							WITH TRACE FINE SANDSTONE/CONGLOMERATE GRAVEL.															
6	28.0	29.0			18		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, AND	6	50/6"			>50										
				30.0			WITH A LITTLE F-C SANDSTONE/CONGLOMERATE GRAVEL.															
							AUGERED TO 30'.															
							CAVED AND DRY AT 24'.															
							GROUNDWATER NOT ENCOUNTERED.															
									1													
									-													
									-													
									_													
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									<u> </u>	<u> </u>	<u> </u>											

Notes/Comments:

Pocket Pentrometer Testing

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0201 PEACH TREE LANE

	Test				Water	Percent	Atterburg Limits (ASTM D4318)			USCS
HDD	Boring	Sample	Depth of Sample (ft.)					Plastic	Plasticity	Classif.
No.	No.	No.	From	To	(ASTM D2216)	_	Limit, %	Limit, %	,	(ASTM D2487)
INO.	INO.				,			-		
		1	3.0	5.0	13.7	47.1	33	25	7	SM
		2	8.0	10.0	6.8	36.0	-	-	-	-
	SB-01	3	13.0	14.4	7.5	25.2	-	-	-	-
	SD-01	4	18.0	19.4	11.8	46.8	32	25	7	SM
		5	23.0	25.0	6.9	18.6	-	-	-	-
		6	28.0	30.0	8.2	18.8	-	-	-	-
		1	3.0	5.0	11.8	31.9	-	-	-	-
S3-0201	SB-02	2	8.0	10.0	9.5	47.8	28	20	8	SC
33-0201		4	18.0	18.5	4.2	27.7	-	-	-	-
		5	23.0	23.5	6.2	41.2		-	-	-
		7	33.5	34.1	5.9	40.8	ı	-	-	-
		2	8.0	10.0	15.4	37.0	NL	NP	NV	SM
		3	13.0	15.0	6.4	19.4	-	-	-	-
	SB-03	4	18.0	20.0	5.3	16.1	-	-	-	-
		5	23.0	25.0	4.4	10.6	1	-	-	-
		6	28.0	29.0	7.7	25.9	ı	-	-	-

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD \$3-0201

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
		SB-01	Hammer Creek Fm - reddish-brown, coarse grained sandstone with interbeds of red shale and quartz pebble conglomerate			limestone with interbedded crystalline dolomite	9,360		
\$3-0201	Peachtree Lane	SB-02	Hammer Creek Conglomerate - very coarse quartz conglomerate having abundant pebbles and cobbles of gray quartzite.	level rolling upland	Hammer Creek Conglomerate	quartz conglomerate;	2,580	generally 10-30 but some shallow outcrops present	Yields generally 5-30 range with some high yield outlier wells
		SB-03	Hammer Creek Conglomerate - very coarse quartz conglomerate having abundant pebbles and cobbles of gray quartzite.			reddish brown cross-bedded <u>sandstone</u>			

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u> Very Loose	<u>N (blows)*</u> 5 or less	<u>Particle Si</u>	Particle Size Identification				
•	6 to 10	Boulders	8 in. diamet	ter or more			
Loose		Cobbles	3 to 8 in. di	ameter			
Medium Dense Dense	11 to 30 31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve			
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve			
,		Sand	Coarse (C)	No. 4 to No. 10 sieve			
				(4.75mm-2.00mm)			
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve			
<u>Description Term</u>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)			
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve			
Little	11 - 20			(0.425 – 0.074mm)			
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)			
And	36 - 50	-, ,		,			

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock				
Quality Designation	Quality Descripti				
(RQD), %	<u>on</u>				
0-25	Very Poor				
25-50	Poor				
50-75	Fair				
75-90	Good				
90-100	Excellent				

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

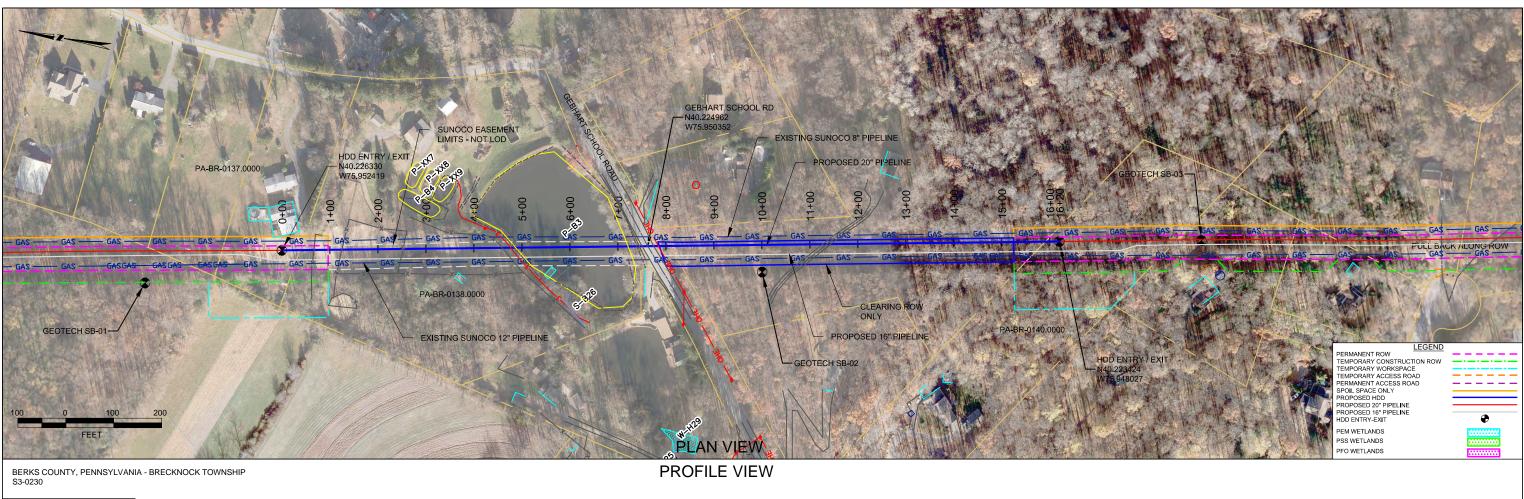
	Major Divisions			Typical Descriptions			Laboratory Classification	ons		
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}} \text{ greater than 4: } C_{c=\frac{(D_{30})2}{D_{10} \text{ x } D_{60}} \text{ between 1 and 3}$			
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	Gw, GP, Sw, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C _u or C _c requiren	nents for GW		
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size (than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are		
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from tion smaller assified as fo	W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols		
Coarse Grained Soils f material is larger tha	maller than	ands io fines)	sw	Well graded sands, gravely sands, little or no fines	2 2 6	Less than 5 percent G More than 12 percent G 5 to 12 percent B	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{L}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3		
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines			Not meeting C_u or C_c require	ments for SW		
N)		No. n fines able fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched		
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols		
Major	Divisions	Group Symbols	Туріса	Typical Descriptions		lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.		
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	O A Line:				
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	5(U Line:	1 1	Or I		
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or		
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and Clays (Liquid limit greater than 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH		
Fin half of mat		СН	Inorganic clar	ys of high plasticity,	Plasi		Character			
(More than	Silts ar 9	ОН	Organic clays	s of medium to high anic silts	7		ML or OL	0 70 80 90 100		
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL			

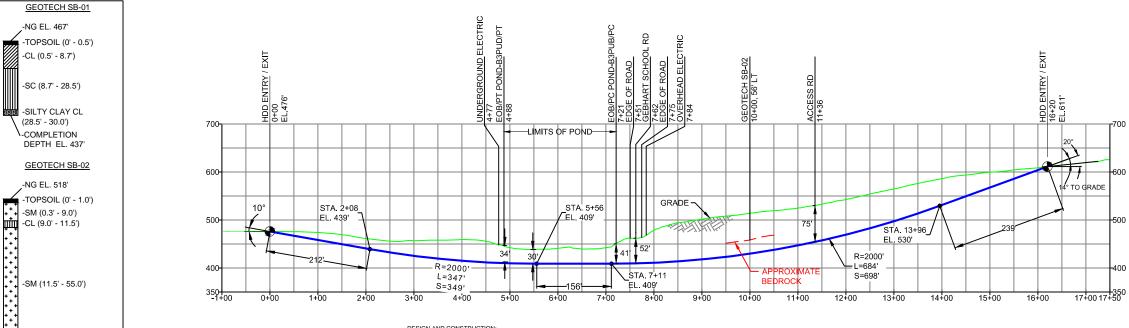
⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

HDD PA-BR-0138.0001-RD (PuB-B3)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 510 feet from the western edge of Pond B3 (PuB-B3) and enter/exit 1,050 feet from the eastern edge. The drill will pass 30 feet below PuB-B3. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrates being drilled through are siltstone, silty clays, and fine sands. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.





GEOTECH SB-03 -NG EL. 643' -TOPSOIL (0' - 0.1') -CL (0.1' - 16.5') -CL/GC (16.5' - 25.0') -COMPLETION DEPTH EL. 618' NOTE: REFER TO TEST BORING LOG S3-0230 FOR COMPLETE SOIL MATERIAL DESCRIPTION

-SILT STONE (55.0' - 57.5') -CL (57.5' - 61.0')

-COMPLETION DEPTH EL. 450'

-SILT STONE (61.0' - 68.0')

- DESIGN AND CONSTRUCTION:

 1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.

 2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED DIPPLINE.
- HAN 10 FEE I AS MEASURED FROM THE OUTSIDE EDGE OF PIPELINE.
 DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4 CROSSING PIPE SPECIFICATION: HDD HORZ. LENGTH (L=):1620' HDD PIPE LENGTH (S=):1654'
 - 20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW

- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGH FACTOR 0.50).
 INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
- STREAM CROSSINGS.
 CARRIER PIPE NOT ENCASED.
 PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR
- WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNDOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

					TIDD TILL LENGTH (0=).		TI. SEE SONOCO FENNSTEVANIA FIFEI	LIINE FF	COLC I LO	KI WED	WAL LOK	100LG	3 KOKD /
NOTE: REFER TO TEST BORING LOG S3-0230					20" x 0.456" W.T., X-65, A	NPI5L, PS	SL2, ERW, BFW						
FOR COMPLETE SOIL MATERIAL DESCRIPTION					COATING: 14-16 MILS FE	BE WITH	30-35 MIL ARO (POWERCRETE R95)						
	NOTES		F	REF. DR.	AWING		REVISIONS						
	COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83		TO ES	S-5.47	EROSION & SEDIMENT PLAN	EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	DLM	09/30/16	RMB	09/30/16	AAW	09/30/16
	ATIONING IS BASED ON HORIZONTAL DISTANCES. ONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION		TO SH	HEET 27	AERIAL SITE PLAN	EP1	REVISED PER PADEP COMMENTS	JTW	05/09/16	RMB	05/09/16	AAW	05/09/16
	ROUNEY ENGINEERING, INC. AND SUNGED PIPELINE, IP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNGCO PIPELIN					EP		MRS	11/23/15	RMB	11/23/15	AAW	11/23/15
LP, FOR ANY DAMAGES RESULTING FRO	OM ERRORS OR OMISSIONS THEREIN.					С	ADDED GEOTECH INFO	MRS	09/28/15	RMB	09/28/15	AAW	09/28/15
CONTRACTOR IS RESPONSIBLE FOR LC DIGGING.	I. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO					В	ISSUED FOR BID	MRS	07/31/15	RMB	07/31/15	AAW	07/31/15
5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.						Α	ISSUED FOR REVIEW	KB	04/15/15	RMB	04/15/15	AAW	04/15/15
		DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	CHK	DATE	APP	DATE



Sunoco Logistics Partners L.P.

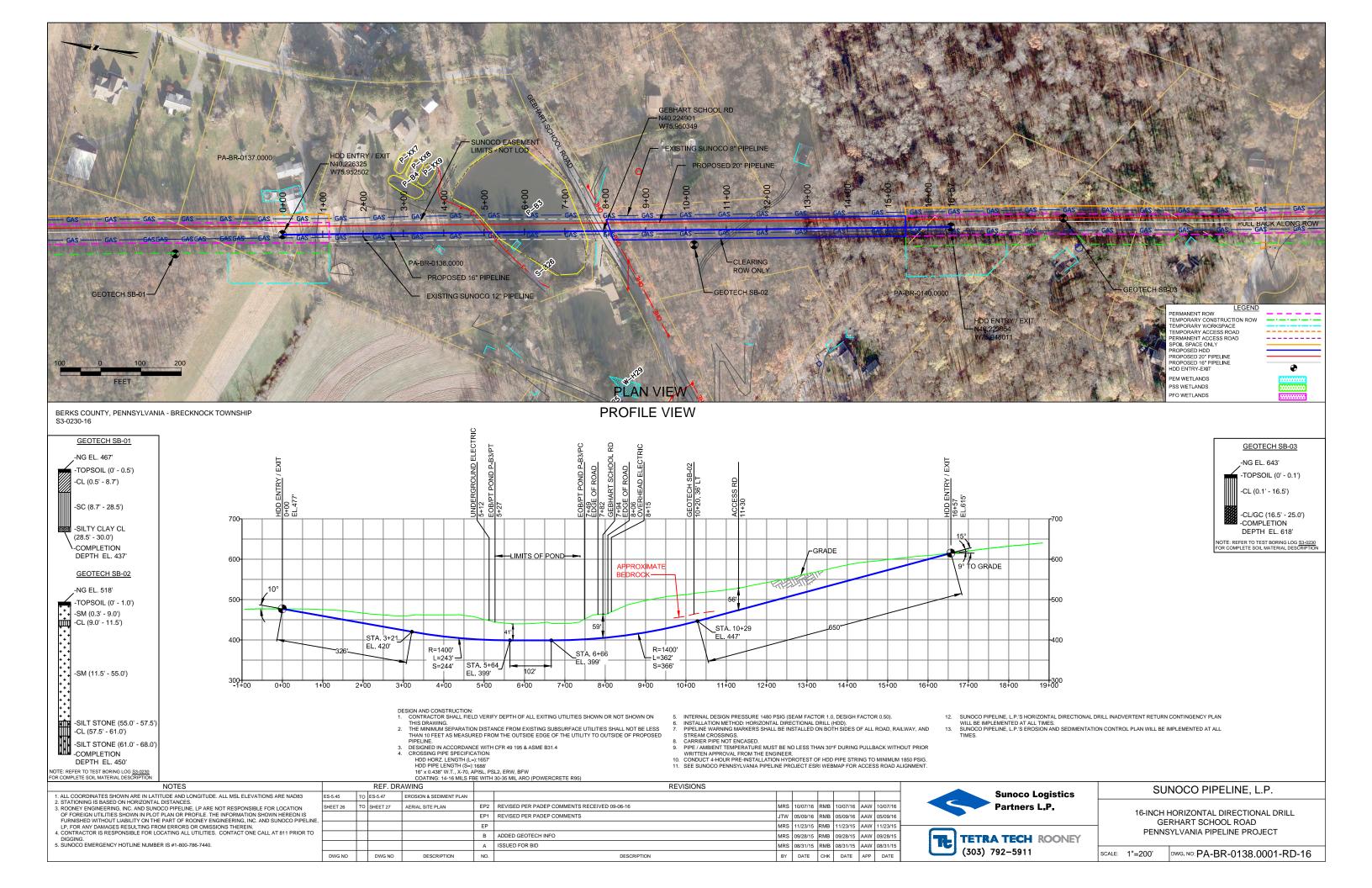
20-INCH HORIZONTAL DIRECTIONAL DRILL GERHART SCHOOL ROAD

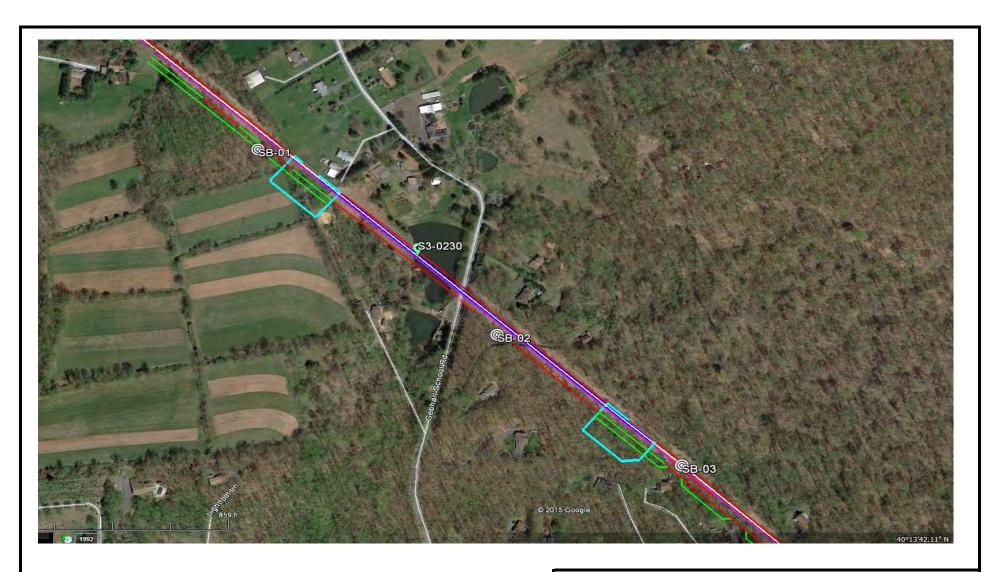
SUNOCO PIPELINE, L.P.

TETRA TECH ROONEY (303) 792-5911

PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=200' DWG. NO: PA-BR-0138.0001-RD





LEGEND:

© Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S3-2300
BERKS COUNTY, BRECKNOCK TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENN	SYLV	ANIA P	Project No.: 103IP3406				
Project Location:	GEBHART SCH	OOL F	ROAD			Page 1	of 1	
HDD No.:	S3-0230			Dates(s) Drilled: 02-08-15	Inspector:	E. WA	ТТ	
Boring No.:	SB-01			Drilling Method: SPT - ASTM D1586	Driller:	S. HOF	FER	
Drilling Contractor:	HAD DRILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0		
Boring Location Coor	dinates:			40° 13' 36.153" N	75° 57' 11.979" \	W		
- Cample Depth /	(t) Strata Donth (ft)		Ctroto					

Domig	Location	1 Oooran	iatoo.				10 10 00.100 11					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials I 6" Increment Blov		ws *	N		
No.	From	То	From	То	Re.	(USCS)			V3			
			0.0	0.5			TOPSOIL (6")				<u></u>	
1	3.0	5.0	0.5		13	CL	REDDISH BROWN SILTY CLAY WITH SOME FINE SAND, TRACE	1	2	3	5	5
				8.7		CL	COARSE GRAVEL. (USCS: CL).				1	
2	8.0	10.0	8.7		24		REDDISH BROWN FINE SAND WITH SOME SILTY CLAY, TRACE	5	10	10	18	20
							MICA.					
3	13.0	15.0			24		REDDISH BROWN FINE SAND AND SILTY CLAY, WITH A LITTLE	2	18	16	17	34
						-00	UNWEATHERED SILTSTONE/SHALE ROCK FRAGMENTS.					
4	18.0	20.0			18	SC	REDDISH BROWN FINE SAND, WITH INTERLAYERED SILTY CLAY	3	7	10	24	17
							LENSES. (USCS: SC).					
5	23.0	25.0			18		REDDISH BROWN MICACEOUS FINE TO MEDDIUM SAND WITH A	5	12	16	16	28
				28.5			LITTLE SILTY CLAY.	1				
6	28.0	29.3	28.5		15		REDDISH BROWN SILTY CLAY, TRACE WEATHERED	2	25	50/3"		>50
				30.0		CL	SILTSTONE/SHALE. (USCS: CL)				<u> </u>	
							AUGERED TO 30'.	+				
								_				
							CAVED AND DRY AT 29'.					
								+				
								+				
								+				
								+-	-	-		├
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								4—				<u> </u>
											ì	

Notes/Comments:

Pocket Pentrometer Testing

S1: 1.0 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Namar					_			_				
mame:		SUNOC	O PENN	SYLVA	NIA PI		•		3IP34	06		
Location	า:	GEBHAI	RT SCH	OOL R	DAC	P	age 1 of 2	2				
D.:		S3-0230)			()	. WATT					
No.:		SB-02						₹				
			ILLING				8.0					
		1	\4l- /f4\		0	40° 13' 27.913" N 75° 56' 59.361" W						T -
From	Jeptn (tt)	From	Peptn (π)	Recov (in)		Description of Materials	6	" Incr	reme	nt Blo	ws *	N
		0.0	0.3			TOPSOIL (3")						
3.0	5.0	0.3		23	SM	REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT,	3		9	9	8	18
			9.0			A LITTLE FINE GRAVEL.						
8.0	10.0	9.0		10	CI	REDDISH BROWN SILTY CLAY, TRACE FINE SAND.	4		3	4	4	7
			11.5		Ċ.	(USCS: CL).						
13.0	13.7	11.5		7		REDDISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, TRAC	E 25	5 50	0/2"			>50
						FINE UNWEATHERED GRAVEL.						
18.0	18.6			9		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, WI	ΓH 20) 50	0/3"			>50
						A LITTLE FINE TO COARSE GRAVEL.						
23.0	23.3			4		LIGHT REDDISH BROWN FINE TO MEDIUM SAND, TRACE SILT, SOI	ME 50/	4"				>50
						FINE TO COARSE GRAVEL AND CONGLOMERATE.						
28.0	28.7			8		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, SO	ME 30	5 5	0/2"			>50
						FINE TO COARSE GRAVEL AND CONGLOMERATE.						
38.0	40.0			19	SM	REDDISH BROWN MEDIUM TO COARSE SAND WITH SOME SILT, TI	RACE 36	6 4	42	16	20	58
						FINE GRAVEL.						
43.0	43.3			3		REDDISH BROWN FINE SAND AND WEATHERED ROCK FRAGMEN	rs. 50/	3"				>50
48.0	48.4			4		REDDISH BROWN FINE SAND AND WEATHERED ROCK FRAGMEN	rs, 50/	5"				>50
						SOME SILT. (USCS: SM).						
53.0	53.5			6		REDDISH BROWN FINE SAND AND WEATHERED ROCK FRAGMEN	rs, 50/	6"				>50
			55.0			SOME SILT.						
						AUGER REFUSAL AT 55". BEGIN ROCK CORING:						
55.0	57.5	55.0	57.5	17		REDDISH BROWN SILTSTONE	TCR	: 57%	SCR	2: 57%	ROD: 3	0%
00.0	07.0	00.0	07.0	''		NESSION SICONO SICONO		07 70	,, 00.1	01 70,		T
						CORE HOLE FILLED WITH MATERIAL, PULLED BARREL AND SWITC	CHED					
						BACK TO AUGER (SEE NEXT PAGE).						
						, , , ,						
		1										
	Location	Location: Do: No.: Contractor: Location Coordi Sample Depth (ft) From To 3.0 5.0 8.0 10.0 13.0 13.7 18.0 18.6 23.0 23.3 28.0 28.7 38.0 40.0 43.0 43.3 48.0 48.4	Sand Sand	Sand Sand	Sand Sand	Sand Sand	Dates Date	Description GEBHART SCHOOL ROAD	Date Contractor Sebhart School Road Dates (s) Drilled: 02-25-15 Inspector: E. Wathrick Short Sho	Dates(s) Drilled: 02-25-15 Inspector: E. WATT	Description Sebhart School Road Description of Materials Descriptio	Sacration Sebata Sacration Sacration Sacratical Sacration Sacratical Sacratical

Notes/Comments:

Pocket Pentrometer Testing

S2 (9.5'): 3.25 TSF S11: > 4 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

			Tax: 302.45	4.5988									
Projec	t Name:		SUNOC	O PENN	ISYLV <i>A</i>		Project No.:	103IP3	3406				
Projec	t Locatio	n:	GEBHA	RT SCH	OOL R	OAD			Page 2 of 2			-	
HDD N	No.:		S3-0230)			Dates(s) Drilled: 02-26-15	Inspector:	E. WATT				
Boring	No.:		SB-02				Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER				
	g Contrac		HAD DF	RILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	68.0				
Boring	Location	n Coordi	1				40° 13' 27.913" N	75° 56' 59.361" \	N				
Sample	Sample	Depth (ft)	Strata [Depth (ft)	Recov. (in)	Strata	Description of Materi	als	6"	Increm	nent Blov	ws *	N
No.	From	То	From	То	Re	(USCS)							<u> </u>
11	F7 F	E0.0	E7 E		15		DEDDICH DDOMNI CII TV CLAV AND EINE CAN	ID TRACE OILTOTO	ONE 20	20	FO/4"		. 50
11	57.5	58.8	57.5		15	CL	REDDISH BROWN SILTY CLAY AND FINE SAN		ONE 28	29	50/4"		>50
				61.0			FRAGMENTS. (DECOMPOSED SILTSTONE).	(USCS: CL).					
12	64.0	64.3	61.0	65.0	3	当	REDDISH BROWN WEATHERED SILTSTONE.		50/3	3"			>50
RUN 2	65.0	68.0	65.0		30	SILTSTONE	HEAVILY WEATHERED REDDISH BROWN SIL	TSTONE.	TCR:	83%, SC	CR: 42%,	RQD: 1	9%
						LS	SLIGHT TO MODERATELY WEATHERED SILT	STONE.					
				68.0		lls	HEAVILY WEATHERED REDDISH BROWN SIL	TSTONE.					
							AUGER REFUSAL AT 65', BEGAN ROCK CORI	NG AT 65'.					
											+		
							COULD NOT RESET CORE BARREL TO CONT	INUE ROCK CORIN	lG				
							DUE TO MATERIAL CAVE WITHIN BOREHOLE	·.					
											+		
							WATER LEVEL NOT ENCOUNTERED WITHIN	OVERBURDEN.			+		
							CAVED AND DRY AT 26'.						
										+	+		
							CORE TESTING RESULTS (RUN 2, DEPTH 66.	7)'):			+		
							COMPRESSIVE STRENGTH: 3,390 PSI						
							UNIT WEIGHT: 157.1 PCF			+	+		
										+	+		
										_	+		
													<u> </u>
		-			+	1				+	+		

Notes/Comments:

Pocket Pentrometer Testing

S2 (9.5'): 3.25 TSF S11: > 4 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI	PELINE PROJECT		Project No.: 103IP3406
Project Location:	GEBHART SCHOOL ROAD			Page 1 of 1
HDD No.:	S3-0230	Dates(s) Drilled: 02-11-15	Inspector:	E. WATT
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	25.0
Boring Location Coord	inates:	40° 13' 22.412" N	75° 56' 49.955" V	V

builing	LUCATIO	ii Coolali	เลเธร.			40 13 22.412 IN 73 30 49.933 W						
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials I 6" Incremen		ent Blov	NC *	N	
No.	From	То	From	То	Rec	(USCS)	5)		ioreini	J. 11 DIO	*3	14
			0.0	0.1			TOPSOIL (1")					
1	3.0	5.0	0.1		14		REDDISH BROWN SILTY CLAY AND FINE SAND, TRACE FINE		5	8	13	13
							GRAVEL.					
2	8.0	10.0			24	01	REDDISH BROWN SILTY CLAY AND FINE SAND, MICACEOUS,	8	13	23	35	36
						CL	TRACE FINE GRAVEL.					
3	13.0	15.0			24		REDDISH BROWN SILTY CLAY. (USCS: CL)	4	11	18	17	29
				16.5								
4	18.0	19.5	16.5		18		REDDISH BROWN SILTY CLAY WITH SOME FINE MICACEOUS	3	31	50/6"		>50
						CL/	SAND, WITH SOME FINE GRAVEL.					
5	23.0	24.5			18	GC	REDDISH BROWN SILTY CLAY WITH SOME FINE MICACEOUS	10	41	50/6"		>50
				25.0			SAND, WITH SOME FINE GRAVEL. (USCS: CL)					
							AUGER REFUSAL AT 25'.					
							CAVED AND DRY AT 24'.					
						<u> </u>						
				1		1			Щ			1

Notes/Comments:

Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD \$3-0230

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of Sample (ft.)		Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	5.0	19.6	73.5	34	19	15	CL
		2	8.0	10.0	6.6	28.6	-		-	-
	SB-01	3	13.0	15.0	10.3	47.8	-	-	-	-
	36-01	4	18.0	20.0	12.8	41.0	30	20	10	SC
		5	23.0	25.0	9.2	17.6	-	-	-	-
		6	28.0	29.3	13.9	95.7	36	21	15	CL
		2	8.0	10.0	21.0	97.7	42	24	18	CL
		3	13.0	13.7	7.6	35.5	-	-	-	-
S3-0230	SB-02	6	28.0	28.7	5.7	18.8	-	-	-	-
	36-02	7	38.0	40.0	12.3	27.9	-	-	-	-
		9	48.0	48.4	10.6	35.0	29	24	5	SM
		11	57.5	58.8	14.0	63.2	42	19	23	CL
		1	3.0	5.0	12.9	52.8	-	-	-	-
		2	8.0	10.0	15.6	61.1	-	-	-	-
	SB-03	3	13.0	15.0	14.9	99.0	35	20	15	CL
		4	18.0	19.5	8.8	72.4	-	-	-	-
		5	23.0	24.5	10.0	74.0	34	19	15	CL

Rock Core Testing Results										
Boring	Core	Approximate	Compressive	Unit						
No.	Run	Depth (ft)	Strength (psi)	Weight (pcf)						
SB-02	2	66.7	3,390	157.1						

Notes:

1) Sample depths based on feet below grade at time of exploration.

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0230

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
		SB-01	Hammer Creek Fm - reddish-brown, coarse grained sandstone with interbeds of red shale and quartz pebble conglomerate		Hammer Creek Fm.	sandstone with quartz pebble conglomerate	9,360	13-70	
S3-0230	Gebhart School Rd.	SB-02	Hammer Creek Conglomerate - very coarse quartz conglomerate having	Rolling hills	Hammer Creek	quartz conglomerate; reddish brown	2,580	14-70	
		SB-03	abundant pebbles and cobbles of gray quartzite.		Conglomerate	cross-bedded sandstone			

 $\underline{\textit{Note}}: \textit{Source of well log data-http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm.} \ \textit{All other sources as referenced in comments section.}$

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0230

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
		1	55	57.5	57	57	30	55	57.5	Slight	Siltstone	Massive	RAU	Fractures ranging from 0° to 35°, Avg. 15°
								65	66	Heavily	Siltstone	Approx. 1'	Red	Heavily fractured and appears as though fines washed out during drilling
S3-0230	SB-2	2	65	38	83	42	19	66	67	Slight to moderate	Siltstone	Approx. 1'	Red	Fractures (very few) ranging from 0° to 5°, Avg. 2.5°; infilling of fractures with minerals, probably quartz
								67	68	Heavily	Siltstone	Approx. 1'	Red	Heavily fractured and weathered, very broken up and soft

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u> Very Loose	<u>N (blows)*</u> 5 or less	<u>Particle S</u>	ize Identifica	<u>tion</u>
•	6 to 10	Boulders	8 in. diame	ter or more
Loose		Cobbles	3 to 8 in. di	ameter
Medium Dense Dense	11 to 30 31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
,		Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
<u>Description Term</u>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	-, ,		,

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	<i>5 , 5</i>	

ROCK (Rock Cores)

Rock	Rock
Quality Designation	Quality <u>Descripti</u>
(RQD), %	<u>on</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

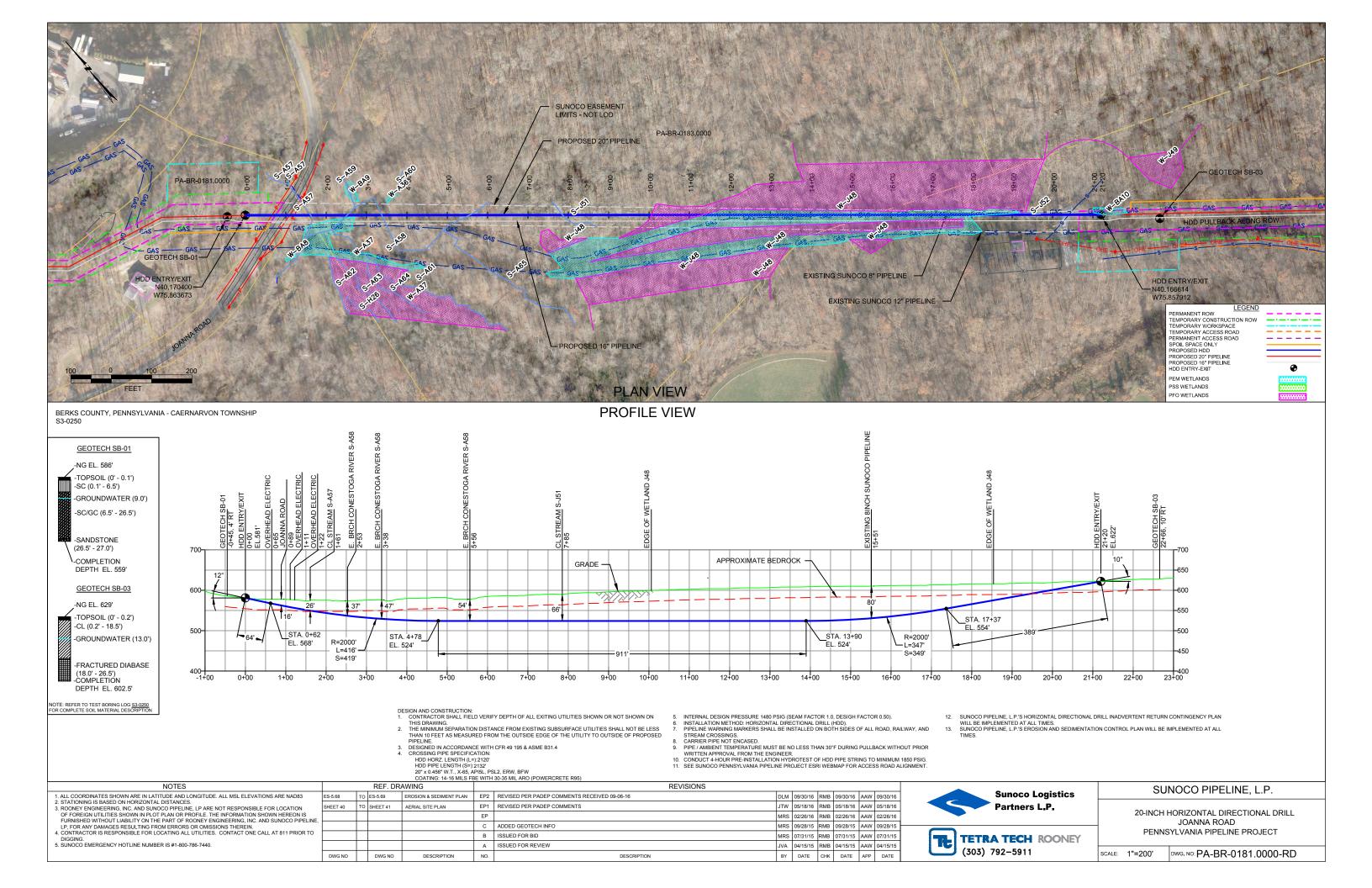
	Major Divisi	ons	Group Symbols	Typical Descriptions			Laboratory Classification	ons
	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines		nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
(6)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	curve. 00 sieve),	GW, GP, SW, SP GM. GC, SM, SC Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C _u or C _c requiren	nents for GW
o. 200 sieve	Gra n half of co than No. 4	Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	grain size or than No. 2	/, SP , SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with I p between 4 and 7 are
d Soils ger than No	More tha	Gravel v (Appre amount	GC	Clayey gravels, gravel-sand-clay mixtures	gravel from tion smaller assified as fo	W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols
Coarse Grained Soils f material is larger tha	maller than	ands io fines)	sw	Well graded sands, gravely sands, little or no fines	of sand and of fines (frac ed soils are cla		$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{D_{10}}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c required	ments for SW
N)	half of coa	n fines able fines)	SM	Silty sands, sand- silt mixtures	Determ		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched
	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols
Major	Divisions	Group Symbols	Туріса	Descriptions	For soils p When w _L	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.
	ıys han 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	A Line:		
200 sieve)	Silts and clays Jimit less than 50)	CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	50	U Line:	0.73(LL - 20) 0.9(LL - 8)	Or I
is r than No.	Silt (Liquid li	OL	Organic silts clays of low	and organic silty plasticity	% (PI), %			, or oth
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	iquid limit 50)	мн		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		13/18/	MH or OH
Fin half of mat	Silts and Clays (Liquid limit greater than 50)	СН	Inorganic clar	ys of high plasticity,	blasi		Culton	
(More than	Silts ar 9	ОН	Organic clays	s of medium to high anic silts	7 4	<u> </u>	ML or OL 20 30 40 50 6	0 70 80 90 100
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL	

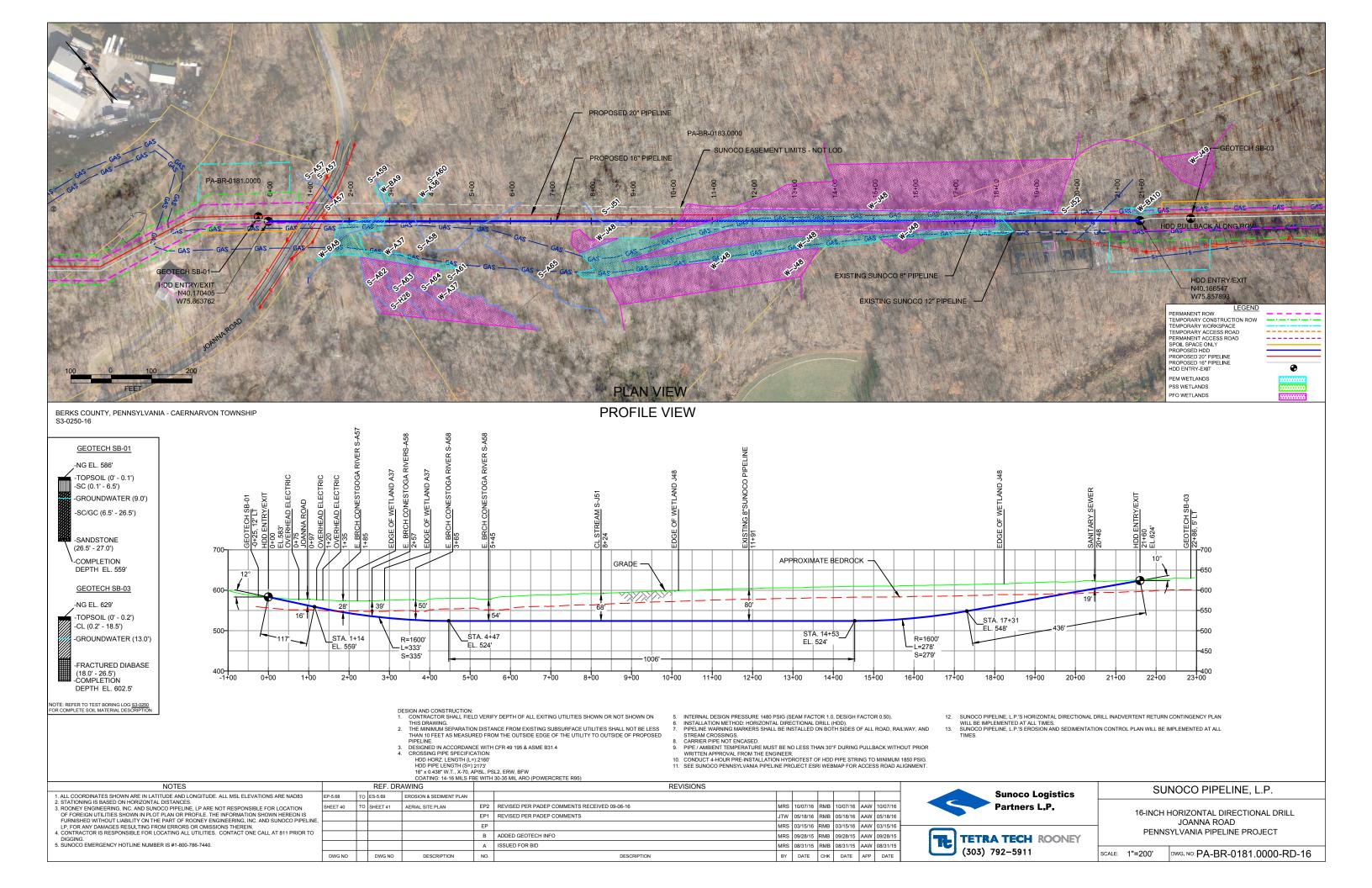
⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

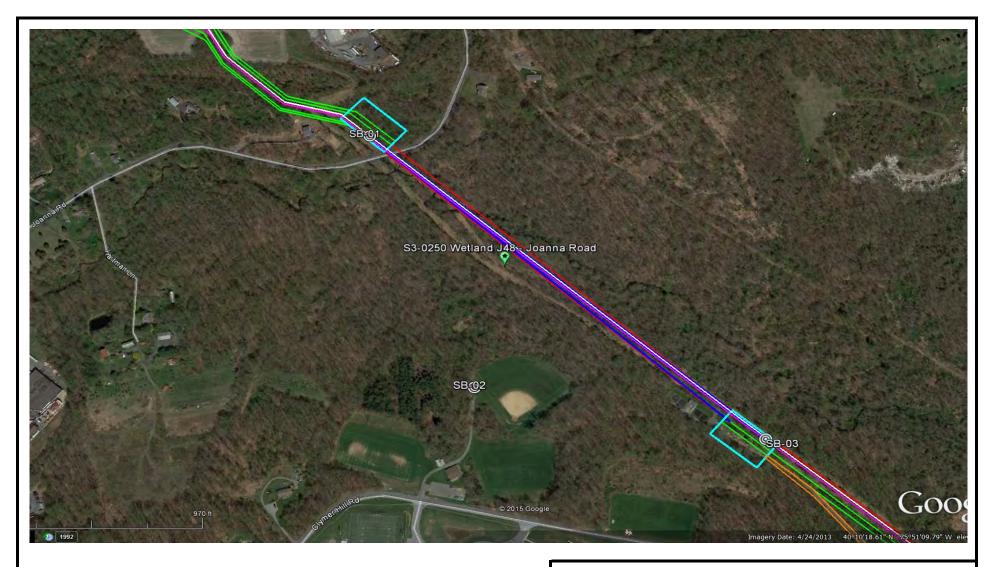
HDD PA-BR-0181.0000-RD (S-A57, S-A58, S-A59, S-A61, PFO-J48, PEM-J48, PEM-BA10)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 180 feet from the western edge of Stream A57 (S-A57) and enter/exit 1,940 feet from the eastern edge. The horizontal directional drill will enter/exit 270 feet from the western edge of Stream A59 (S-A59) and enter/exit 1,900 feet from the eastern edge. The western edge of Stream A58 (S-A58) is 360 feet from the drill's western entrance/exit while the stream's eastern edge is 1,810 feet from the drill's eastern entrance/exit. The drill will enter/exit 570 feet from the western edge of Stream A61 (S-A61) and enter/exit 1600 feet from the eastern edge. The drill will then pass below Stream J51 (S-J51) whose western edge is 800 feet from the drill's entrance/exit and whose eastern edge is 1,360 feet from the drill's eastern entrance exit. The drill will enter/exit 1,020 feet from the western edge of Forested and Grassy Wetland J48 (PFO-J48 and PEM-J48) and enter/exit 220 feet from the eastern edge of the wetland. The eastern entrance/exit of the drill will be through Grassy Wetland BA10 (PEM-BA10). With the exception of PEM-BA10 the drill will pass at least 25 feet below each water feature, and at a maximum will be 75 feet below wetland J48. The geotechnical results, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report primary substrates being drilled through are a diabase rock bed with silty clays layers above. Based on the geotechnical report and the drill profile minimal inadvertent returns are expected.







LEGEND:

© Geotechnical Soil Boring (SB) Locations



TETRATECH

GEOTECHNICAL BORING LOCATIONS
HDD S3-0250 WETLAND J48 - JOANNA ROAD
BERKS COUNTY, CAERNARVON TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI	PELINE PROJECT		Project No.: 103IP3406
Project Location:	JOANNA ROAD, MORGANTO	WN, PA		Page 1 of 1
HDD No.:	S3-0250	Dates(s) Drilled: 03-15-15	Inspector:	E. WATT
Boring No.:	SB-01	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 9.0	Total Depth (ft):	27.0
Boring Location Coordi	nates:	40° 10' 13.705" N	75° 51' 49.628" W	I

Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" lr	ncreme	nt Blo	.we *	N
No.	From	То	From	То	Rec	(USCS)	Description of Materials	0 11	iciente	THE DIC	ws	14
			0.0	0.1			TOPSOIL (1")					
1	3.0	5.0	0.1		20	SC	REDDISH BROWN FINE TO MEDIUM SAND AND SILTY CLAY, TRACE	1	7	9	13	16
				6.5		30	FINE TO COARSE GRAVEL (USCS: SC).					
2	8.0	10.0	6.5		10		REDDISH BROWN FINE TO MEDIUM SAND WITH A LITTLE SILTY CLAY,	2	6	12	10	18
							AND FINE TO COARSE SUBANGULAR GRAVEL.					
3	13.0	14.9			19	SC	DARK MAROON FINE TO MEDIUM SAND AND SILTY CLAY, WITH A	6	37	37	50/5"	74
						AND	LITTLE FINE TO COARSE GRAVEL.					
4	18.0	18.1			<1	SC/	MAROON GRAVEL PIECES.	50/1"				>50
						GC						
5	23.0	23.8			8		DARK MAROON FINE TO COARSE GRAVEL AND FINE TO COARSE	10	50/4"			>50
				26.5			GRAVEL, AND SILTY CLAY. (USCS: SC)					
6	26.5	27.0	26.5	27.0	4		GRAY PARTIALLY WEATHERED SANDSTONE.	50/6"				>50
							AUGER REFUSAL AT 26.5'.					
							WET ON SPOON AT 8'.					
							WATER LEVEL THROUGH AUGERS AT 10'.					
							CAVED AT 20'. WATER LEVEL ON CAVE AT 9'.					
												-
											-	-

Notes/Comments:

Pocket Pentrometer Testing

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI	PELINE PROJECT		Project	No.: 103IP3406	
Project Location:	CLYMER HILL ROAD, ELVER	SON, PA		Page 1	of 1	
HDD No.:	S3-0250	Dates(s) Drilled: 03-09-15	Inspector:	E. WAT	Т	
Boring No.:	SB-02	Drilling Method: SPT - ASTM D1586	Driller:	S. HOF	FER	
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 8.0	Total Depth (ft):	16.0		
Boring Location Coording	nates:	40° 10' 1.241" N	75° 51' 43.350" W	V		
	a a					

Domis	Location						10 10 1.211 17					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	٥	Strata	Description of Materials	6" 1.	aorom	ent Blo	*	N
No.	From	То	From	То	Recov. (in)	(USCS)	Description of Materials	0 11	icreme	ent Bio	WS	N
			0.0	0.2			TOPSOIL (2")					
1	3.0	5.0	0.2		19	٥.	MOTTLED BROWN SILTY CLAY AND FINE SAND.	2	8	7	7	15
				6.5		CL						
2	8.0	10.0	6.5		10		DECOMPOSED ROCK WEATHERED TO A GREENISH GRAY TO BROWN	1	5	14	6	19
				14.8		SC	CLAYEY FINE TO MEDIUM SAND.					
3	13.0	15.0	14.8		24	٥.	GRAY AND GREENISH GRAY DECOMPOSED ROCK WEATHERED TO A	1	3	8	10	11
				16.0		CL	SILTY CLAY AND FINE TO MEDIUM SAND. (USCS: CL).					
4	16.0	16.0	16.0	16.0	<1		REFUSAL MATERIAL APPEARS TO BE GRANITE or BASALT	50/0"				>50
							AUGER REFUSAL AT 16'.					
							WET ON SPOON AT 8'.					
							NO WATER LEVEL THROUGH AUGERS.					
							CAVED AT 11', WATER LEVEL ON CAVE AT 8'.					
							STARTED GRINDING AT 15.5'					
							AUGERS WERE TOO HIGH OUT OF GROUND TO BE ABLE TO SET					
							CORE BARREL.					
	<u> </u>	L .									Щ	<u> </u>

Notes/Comments:

Pocket Pentrometer Testing

5': 0.75 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

N: Number of blows to drive spoon from 6" to 18" interval.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.



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TEST BORING LOG

Project Name:	SUNOCO PENNSYLVA	ANIA PIPELINE PROJECT		Project No.: 103IP3406
Project Location:	CLYMER HILL ROAD,	ELVERSON, PA		Page 1 of 1
HDD No.:	S3-0250	Dates(s) Drilled: 05-19-15	Inspector:	E. WATT
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 13.0	Total Depth (ft):	26.5
Boring Location Coor	dinates:	40° 9' 58.805"" N	75° 51' 27.075""	W

Ŭ												
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov. (in)	Strata	Description of Materials	6" 1	ncreme	nt Dio	WC *	Z
No.	From	То	From	То	Rec (ir	(USCS)	Description of Materials	0 1	пстеппе	пі Біс	ws	IN
			0.0	0.2			TOPSOIL (2")					
1	3.0	5.0	0.2		18		MOTTLED BROWN, ORANGE BROWN, GRAY SILTY CLAY AND	1	4	6	10	10
							FINE SAND.					
2	8.0	10.0			12		MOTTLED BROWN, ORANGE BROWN, GRAY SILTY CLAY AND	5	5	5	8	10
							FINE SAND.					
3	13.0	15.0			24	CL	ORANGE BROWN SILTY CLAY AND FINE SAND.	1	2	2	2	4
							(USCS: CL).					
4	18.0	18.9			10		MOTTLED BROWN, LIGHT BROWN AND WHITE SILTY CLAY AND	1	50/5"			>50
				18.5			FINE SAND.					
			18.5	18.9			GRAY TO DARK GRAY PARTIALLY WEATHERED DIABASE.					
							AUGER REFUSAL AT 18.5'. AUGERS WERE TOO SKEWED TO CORE,					
							SO OFF-SET 6' AND AUGERED TO REFUSAL AT 18'. BEGIN CORING.					
							ROCK CORING					
RUN 1	18.0	21.5	18.0	19.2	42		MODERATELY FRACTURED DIABASE.	TCR: 1	100%, SC	R: 93%	, RQD:	74%
			19.2	21.5		X	SLIGHTLY FRACTURED DIABASE.					
RUN 2	21.5	26.5	21.5	22.4	48	ROCK	INTENSELY FRACTURED DIABASE, SOME OLIVINE DEPOSITS.	TCR: 8	30%, SCF	R: 60%,	RQD: 5	5%
			22.4	25.5			UNFRACTURED DIABASE WITH OLIVINE DEPOSITS					
			25.5	26.5			FRACTURE, RODS DROPPED QUICKLY					
								1				
							CORE TESTING RESULTS (RUN 1, DEPTH 21'):					
							COMPRESSIVE STRENGTH: 1,510 PSI					
							UNIT WEIGHT: 187.3 PCF					
								1	1			
				1	1							

Notes/Comments:

Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

^{*} Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0250 WETLAND J48 - JOANNA ROAD

	Test				Water	Percent	Atterburg	Limits (AS	TM D4318)	USCS
HDD	Boring	Sample	Depth of S	Sample (ft.)	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	То	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	5.0	11.8	43.4	33	21	12	SC
		2	8.0	10.0	13.4	16.1	-	-	-	-
	SB-01	3	13.0	14.9	12.0	37.6	-	-	-	-
	3D-01	4	18.0	18.1	5.1	6.7	-	-	-	-
		5	23.0	23.8	18.0	42.9	34	21	13	SC
		6	26.5	27.0	10.5	24.5	-	-	-	-
S3-0250		1	3.0	5.0	21.0	57.1	-	-	-	-
	SB-02	2	8.0	10.0	30.8	37.9	-	-	-	-
		3	13.0	15.0	41.2	54.4	41	21	20	CL
		1	3.0	5.0	22.6	63.1	-	-	-	-
	CD 03	2	8.0	10.0	20.9	59.2	-	-	-	-
	SB-03	3	13.0	15.0	66.2	55.8	36	20	16	CL
		4	18.0	18.9	53.5	52.3	-	-	-	-

		Rock Core Te	esting Results	
Boring	Core	Approximate	Compressive	Unit
No.	Run	Depth (ft)	Strength (psi)	Weight (pcf)
SB-03	1	21.0	1,510	187.3

Notes:

1) Sample depths based on feet below grade at time of exploration.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0250 WETLAND J48 - JOANNA ROAD

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
		1	18	21.5	100	93	74							
S3-0250	SB-03	2	21.5	26.5	80	60	55	18	26.5	Slight	Diabase	Massive	Gray	Fractures ranging from 30° to 70°, Avg. 45°

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0250 WETLAND J48 - JOANNA ROAD

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
		SB-01	Stockton Formation - Light-gray to buff, coarse-grained, arkosic sandstone; includes reddish-brown to grayish-purple sandstone, siltstone, and mudstone.		Stockton Fm	primarily sandstone with siltstone and mudstone		35-53	
S3-0250	Wetland J48 - Joanna Rd.	SB-02 and SB-	Diabase - occurs primarily as dikes and sheets and forms a complex igneous network that extensively intrudes sedimentary rocks in the Gettysburg and Newark basins.	Gently- moderately sloping lowlands	Diabase	Ophitic texture, an important variety of basalt texture where pyroxene (or occasionally olivine) forms larger crystals and typically contains numerous crystals of plagioclase		13	

 $\underline{\textit{Note}}: \textit{Source of well log data-http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm.} \ \textit{All other sources as referenced in comments section.}$

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u> Very Loose	<u>N (blows)*</u> 5 or less	<u>Particle Si</u>	Particle Size Identification		
•	6 to 10	Boulders	8 in. diameter or more		
Loose		Cobbles	3 to 8 in. diameter		
Medium Dense Dense	11 to 30 31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve	
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve	
,		Sand	Coarse (C)	No. 4 to No. 10 sieve	
				(4.75mm-2.00mm)	
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve	
<u>Description Term</u>	<u>Percent</u>		(M)	(2.00mm – 0.425mm)	
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve	
Little	11 - 20			(0.425 – 0.074mm)	
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)	
And	36 - 50	-, ,		,	

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock			
Quality Designation	Quality <u>Descripti</u>			
(RQD), %	<u>on</u>			
0-25	Very Poor			
25-50	Poor			
50-75	Fair			
75-90	Good			
90-100	Excellent			

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions			Group Symbols	Typical Descriptions	Laboratory Classifications						
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	n is larger	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	nbols ⁽¹⁾		$C_{u=\frac{D_{60}}{D_{10}}} \ \text{greater than 4:} C_{c=\frac{(D_{30})2}{D_{10} \ \text{x} \ D_{60}}} \ \text{between 1 and 3}$ Not meeting C_u or C_c requirements for GW				
	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean (Little or	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM. GC, SM, SC 5 to 12 percent Borderline cases requiring dual symbols ⁽¹⁾						
		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures		v, SP I, SC ases requiri	Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with ! p between 4 and 7 are			
			GC	Clayey gravels, gravel-sand-clay mixtures		W, GP, SW M. GC, SM orderline ca	Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols			
Coarse Grained Soils f material is larger tha	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	ands io fines)	sw	Well graded sands, gravely sands, little or no fines			$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=} = \frac{(D_{30})2}{D_{10} \times D_{60}}$ between 1 and 3				
C ore than half of		Clean sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	ine Percentage on Percentage coarse-grain	Less than 5 percent More than 12 percent 5 to 12 percent	Not meeting C_u or C_c require	ments for SW			
N)		fines able fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched			
		(More than	(More than	(More than	(More than	Sands with fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7
Major	Divisions	Group Symbols	Typical Descriptions		For soils p When w _{l.}	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.			
	Silts and clays (Liquid limit less than 50)	ML	sands, rock fi	s and very fine lour, silty or clayey r clayey silts with iy	60	O A Line:					
200 sieve)		CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	5(U Line:	1 1	Or I			
is r than No.		OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or			
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and Clays (Liquid limit greater than 50)	мн		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH			
		СН	Inorganic clar	ys of high plasticity,	Plasi		Character				
(More than		ОН	Organic clays	s of medium to high anic silts	7		ML or OL	0 70 80 90 100			
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL				

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.