

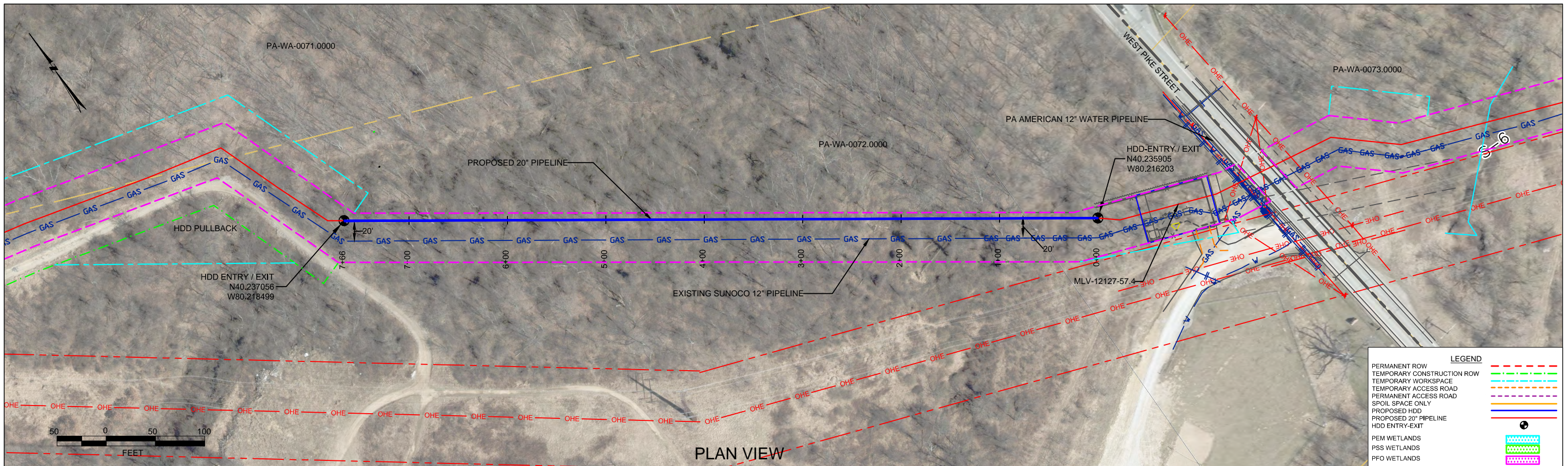
**Attachment A  
HDD Table  
Washington County**

| Drawing Name           | Drill Name                                 | County                  | Township                       | Drill Location               | Risk Assessment Level<br>(Low / Medium / High) |
|------------------------|--|-------------------------|--------------------------------|------------------------------|--|
| PA-WA-0072.0000-SR.pdf | Pike Street                                | Washington              | Chartiers                      | N: 40.237056<br>W: 80.218499 | low  |
| PA-WA-0074.0000-RR.pdf | Pittsburgh & Ohio<br>Central Railway, I-79 | Washington              | Chartiers & North<br>Strabane  | N: 40.235073<br>W: 80.213066 | low  |
| PA-WA-0102.0000-SR.pdf | Cultural Area #2                           | Washington              | North Strabane                 | N: 40.234366<br>W: 80.144241 | medium   |
| PA-WA-0103.0000-RD.pdf | Linden Creek Road                          | Washington              | North Strabane                 | N: 40.235362<br>W: 80.139373 | medium   |
| PA-WA-0106.0000-SR.pdf | Cultural Area #3                           | Washington              | North Strabane                 | N: 40.235652<br>W: 80.132392 | medium   |
| PA-WA-0111.0000-SR.pdf | Cultural Area #4                           | Washington              | North Strabane                 | N: 40.238062<br>W: 80.117729 | medium   |
| PA-WA-0119.0000-RD.pdf | Linden Road                                | Washington              | North Strabane                 | N: 40.235514<br>W: 80.103237 | low  |
| PA-WA-0119.0003-RD.pdf | Munntown Road                              | Washington              | North Strabane &<br>Nottingham | N: 40.233359<br>W: 80.100624 | low  |
| PA-WA-0127.0000-RR.pdf | Allegheny Vally RR                         | Washington              | Nottingham                     | N: 40.235766<br>W: 80.092434 | low  |
| PA-WA-0164.0000-RD.pdf | Mingo Church Road                          | Washington              | Union                          | N: 40.231411<br>W: 80.009203 | low  |
| PA-WA-0171.0000-RR.pdf | Wheeling and Lake Erie<br>RR               | Washington              | Union                          | N: 40.231135<br>W: 79.998609 | low  |
| PA-WA-0172.0000-RD.pdf | Highway 43                                 | Washington              | Union                          | N: 40.229580<br>W: 79.986851 | low  |
| PA-WA-0176.0000-RR.pdf | Norfolk Southern RR &<br>Monongahela River | Washington<br>Allegheny | Union<br>Forward               | N: 40.229918<br>W: 79.975984 | low  |

***HDD PA-WA-0072.0000-SR***

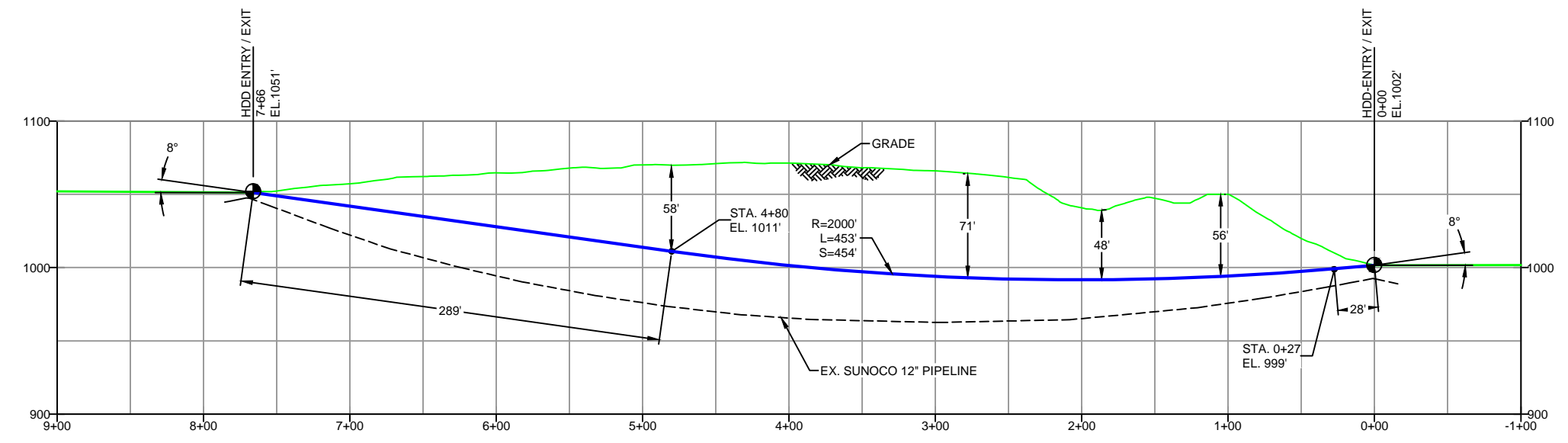
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

This drill will not cross any high risk environmental areas, such as streams or wetlands, or any transportation lines, such as roads or railroads. The 20" drill does parallel the existing ME1 12" pipeline drill. The geotechnical results from November 2014, as well as the previous drill information and other data points were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the substrate the drill will travel through is a silty clay down to 20 feet and potentially rock beyond that. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PLAN VIEW  
PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA, CHARTIERS TOWNSHIP  
SIB-0010



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L=): 766'  
HDD PIPE LENGTH (S=): 771'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |            | EROSION & SEDIMENT PLAN |     | REVISIONS                                    |  |
|--------------|------------|-------------------------|-----|--|--|
| DWG NO       | DWG NO     | DESCRIPTION             | NO. | DESCRIPTION                                  | DATE                                   |
| ES-1.13      | TO ES-1.13 | EROSION & SEDIMENT PLAN |     |  |  |
| SHEET 9      | TO SHEET 9 | AERIAL SITE PLAN        |     |  |  |
|              |            | EP2                     |     | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 | MRS 09/30/16 RMB 09/30/16 AAW 09/30/16 |
|              |            | EP1                     |     | REVISED PER PADEP COMMENTS                   | MRS 05/05/16 RMB 05/05/16 AAW 05/05/16 |
|              |            | EP                      |     |  | JTW 03/15/16 RMB 03/15/16 AAW 03/15/16 |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
PIKE STREET  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'  
DWG. NO: PA-WA-0072.0000-SR



**TETRA TECH**

240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
 302.738.7551  
 fax: 302.454.5988

**TEST BORING LOG**

|  |  |   |                        |                        |  |
|--|--|---|------------------------|------------------------|--|
| Project Name: SUNOCO MARINER EAST      |  |   | Project No.: 103IP2762 |                        |  |
| Project Location: WEST PIKE VALVE SITE |  |   | Page 1 of 1            |                        |  |
| Test Boring No.: SB-01                 |  | Dates(s) Drilled: 11/14/14              |                        | Inspector: J. COSTELLO |  |
| Drilling Contractor: HYNES             |  | Drilling Method: SPT - ASTM D1586       |                        | Driller: JUSTIN        |  |
| Surface Elevation (ft):                |  | Groundwater Depth (ft): NOT ENCOUNTERED |                        | Total Depth (ft): 21.0 |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS) | Description of Materials  | 6" Increment Blows *  |    |    | N  |  |
|------------|-------------------|------|-------------------|------|-------------|---------------|---|---|----|----|----|--|
|            | From              | To   | From              | To   |             |               |   |   |    |    |    |  |
| 1          | 0                 | 2.0  | 0.0               |      | 18          | CL            | VARI-COLORED (BROWN, BLACK, GRAY) SILTY CLAY WITH SOME FINE SAND, TRACE F-GRAVEL. STIFF. (USCS: CL).                            | 5   | 5  | 8  | 13 |  |
| 2          | 3.0               | 4.5  |                   |      | 16          |               | DARK BROWN SILTY CLAY AND FINE SAND WITH THIN APPARENT COAL SEAMS, TRACE F-GRAVEL. VERY STIFF.                                  | 5   | 10 | 13 | 23 |  |
| 3          | 6.0               | 7.5  |                   |      | 18          |               | DARK BROWN TO REDDISH BROWN (WITH MOTTLES OF GRAY) SILTY CLAY AND FINE SAND, TRACE F-GRAVEL. VERY STIFF.                        | 5   | 7  | 12 | 19 |  |
| 4          | 9.0               | 10.5 |                   |      | 14          |               | DARK BROWN TO REDDISH BORWN (WITH MOTTLES OF GRAY) SILTY CLAY, A LITTLE F- SAND, TRACE F-GRAVEL. USCS: CL. STIFF TO VERY STIFF. | 5   | 6  | 9  | 15 |  |
| 5          | 14.0              | 15.5 |                   |      | 16          |               | YELLOWISH TO GREENISH BROWN SILTY CLAY AND FINE SAND, TRACE FINE SILTSTONE GRAVEL. VERY STIFF.                                  | 5   | 6  | 10 | 16 |  |
|            |                   |      |                   | 19.5 |             |               |   |   |    |    |    |  |
| 6          | 19.0              | 20.5 | 19.5              | 21.0 | 14          |               | PARTIALLY WEATHERED LIGHT GRAY SHALE OR SILTSTONE.  | 10  | 25 | 29 | 54 |  |
|            |                   |      |                   |      |             |               |   | AUGER REFUSAL AT 21'.   |    |    |    |  |
|            |                   |      |                   |      |             |               |   | CAVED AND DRY AT 13'.   |    |    |    |  |
|            |                   |      |                   |      |             |               |   | BORING LOCATION IS LOCATED APPROXIMATELY 10' BELOW PREVIOUS NATURAL GRADE (ROUGH, VISUAL ONLY). |    |    |    |  |

Notes/Comments:

Pocket Pentrometer Testing

S1: 2.0 TSF

S3: 3.0 TSF

S4: 3.0 TSF

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.



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# TEST BORING LOG

|  |  |   |                        |                        |  |
|--|--|---|------------------------|------------------------|--|
| Project Name: SUNOCO MARINER EAST      |  |   | Project No.: 103IP2762 |                        |  |
| Project Location: WEST PIKE VALVE SITE |  |   | Page 1 of 1            |                        |  |
| Test Boring No.: SB-02                 |  | Dates(s) Drilled: 11/14/14              |                        | Inspector: J. COSTELLO |  |
| Drilling Contractor: HYNES             |  | Drilling Method: SPT - ASTM D1586       |                        | Driller: JUSTIN        |  |
| Surface Elevation (ft):                |  | Groundwater Depth (ft): NOT ENCOUNTERED |                        | Total Depth (ft): 20.0 |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS) | Description of Materials  | 6" Increment Blows * |    |    | N  |
|------------|-------------------|------|-------------------|------|-------------|---------------|---|----------------------|----|----|----|
|            | From              | To   | From              | To   |             |               |   |                      |    |    |    |
| 1          | 0                 | 2.0  | 0.0               |      | 18          | CL            | VARI-COLORED (BROWN, BLACK, GRAY) SILTY CLAY WITH SOME FINE SAND, TRACE F-GRAVEL (USCS: CL). STIFF. | 3                    | 5  | 7  | 12 |
| 2          | 3.0               | 4.5  |                   | 5.5  | 16          |               | LIGHT GRAY TO BROWN SILTY CLAY, WITH A TRACE OF FINE SAND, TRACE F-GRAVEL (USCS: CL). VERY STIFF.   | 3                    | 7  | 10 | 17 |
| 3          | 6.0               | 7.5  | 5.5               |      | 14          | SC/CL         | YELLOWISH BROWN FINE SAND AND SILTY CLAY WITH A LITTLE SILTSTONE F-GRAVEL. MEDIUM DENSE.            | 4                    | 9  | 15 | 24 |
| 4          | 9.0               | 10.5 | 10.0              |      | 16          |               | GRAY TO YELLOWISH BROWN SILTY CLAY AND FINE SAND WITH SOME FINE TO COARSE SHALE GRAVEL. HARD.       | 6                    | 19 | 22 | 41 |
| 5          | 14.0              | 15.5 |                   | 17.0 | 16          | CL            | REDDISH BROWN SILTY CLAY WITH A LITTLE FINE SAND AND GRAVEL (USCS: CL). HARD.                       | 15                   | 24 | 26 | 50 |
| 6          | 19.0              | 19.4 | 17.0              | 20.0 | 4           |               | PARTIALLY WEATHERED REDDISH BROWN SHALE.  | 50/5"                |    |    |    |
|            |                   |      |                   |      |             |               | AUGER REFUSAL AT 20'.   |                      |    |    |    |
|            |                   |      |                   |      |             |               | CAVED AND DRY AT 9'.  |                      |    |    |    |

**Notes/Comments:**

Pocket Pentrometer Testing

S1: 2.75 TSF

S2: 3 TSF

S5: >4 TSF

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.

**LABORATORY TESTING SUMMARY  
SUNOCO MARINER EAST  
VARIOUS VALVE SITES**

| Valve Site/<br>Soil Boring<br>No. | Sample<br>No. | Depth of Sample (ft.) |      | Water<br>Content, %<br>(ASTM D2216) | Percent<br>Silts/Clays, %<br>(ASTM D1140) | Atterburg Limits (ASTM D4318) |                     |                        | USCS<br>Classif.<br>(ASTM D2487) |
|-----------------------------------|---------------|-----------------------|------|-------------------------------------|---|-------------------------------|---------------------|------------------------|----------------------------------|
|                                   |               | From                  | To   |                                     |   | Liquid<br>Limit, %            | Plastic<br>Limit, % | Plasticity<br>Index, % |                                  |
| WEST PIKE/SB-01                   | 1             | 0.0                   | 2.0  | 18.2                                | 72.7                                      | 40                            | 23                  | 17                     | CL                               |
|                                   | 2             | 3.0                   | 4.5  | 14.5                                | 59.4                                      | -                             | -                   | -                      | -                                |
|                                   | 3             | 6.0                   | 7.5  | 17.0                                | 60.8                                      | -                             | -                   | -                      | -                                |
|                                   | 4             | 9.0                   | 10.5 | 23.3                                | 83.5                                      | 37                            | 23                  | 14                     | CL                               |
|                                   | 5             | 14.0                  | 15.5 | 18.3                                | 62.3                                      | -                             | -                   | -                      | -                                |
|                                   | 6             | 19.0                  | 20.5 | 12.4                                | 51.3                                      | -                             | -                   | -                      | -                                |
| WEST PIKE/SB-02                   | 1             | 0.0                   | 2.0  | 19.8                                | 70.0                                      | -                             | -                   | -                      | -                                |
|                                   | 2             | 3.0                   | 4.5  | 16.8                                | 97.9                                      | 38                            | 23                  | 15                     | CL                               |
|                                   | 3             | 6.0                   | 7.5  | 10.1                                | 49.0                                      | -                             | -                   | -                      | -                                |
|                                   | 4             | 9.0                   | 10.5 | 8.5                                 | 61.5                                      | -                             | -                   | -                      | -                                |
|                                   | 5             | 14.0                  | 15.5 | 8.3                                 | 89.9                                      | 39                            | 22                  | 17                     | CL                               |
|                                   | 6             | 19.0                  | 19.4 | 9.6                                 | 85.4                                      | -                             | -                   | -                      | -                                |

**Notes:**

- 1) Sample depths based on feet below grade at time of exploration.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more  |
| Cobbles   | 3 to 8 in. diameter   |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve<br>Fine (F) ¾ in. to No. 4 sieve  |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium No. 10 to No. 40 sieve<br>(M) (2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

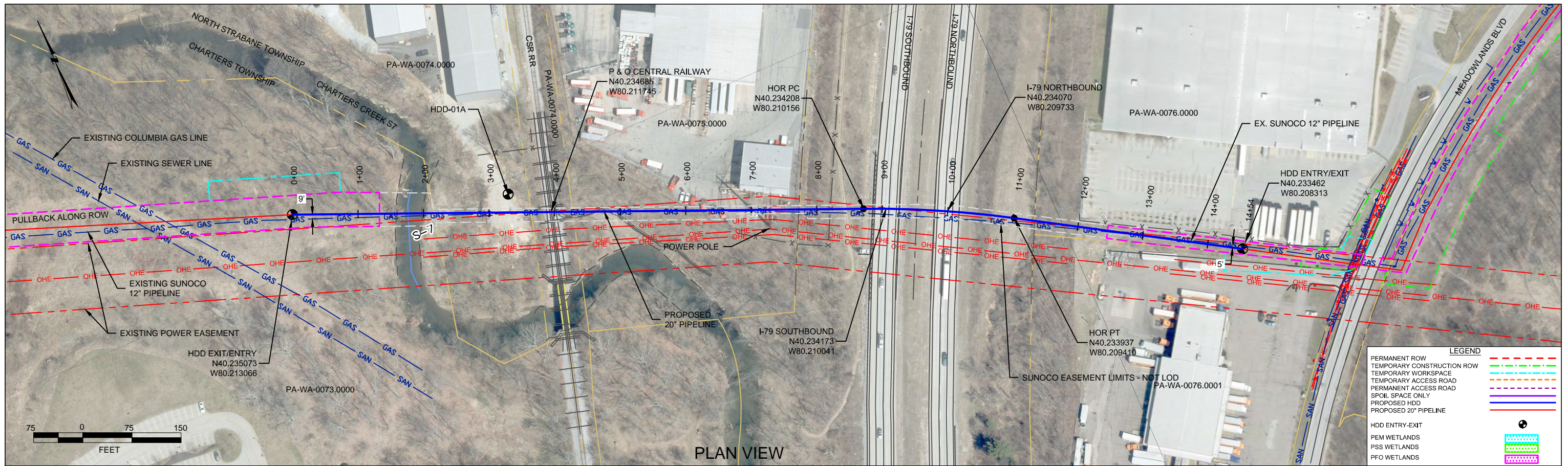
| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |  |   |  |
|---|--|---|---|---|--|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW  |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |  |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4<br><br>Atterberg limits above A line with $I_p$ greater than 7<br><br>Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols |   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       |  |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)       | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW  |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |  |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4<br><br>Atterberg limits above A line with $I_p$ greater than 7<br><br>Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols |   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                |  |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |  |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions   | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                     | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |  |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |  |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |  |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                  | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |  |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |  |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |  |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |  |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0074.0000-RR (S7)***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

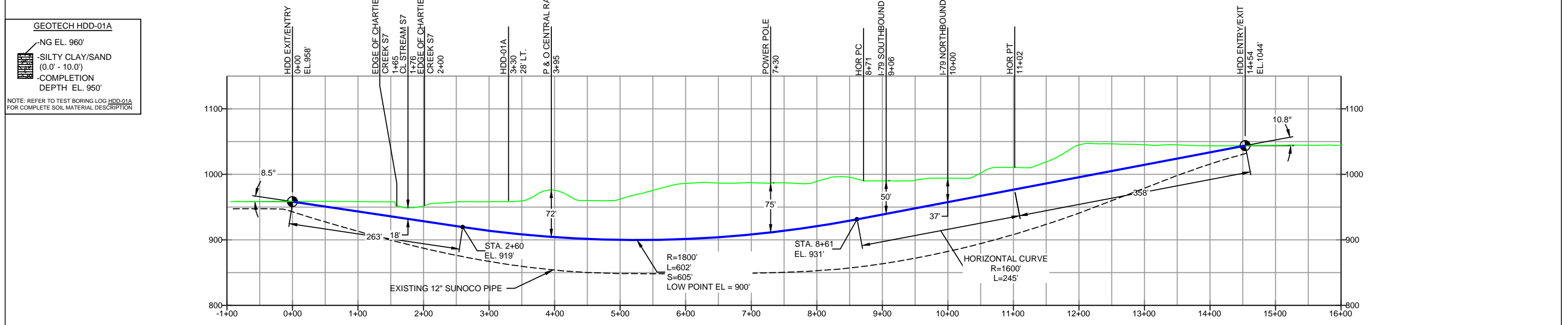
The drill will enter/exit 150 feet from the edge of the western most boundary of Chartiers Creek (S7) and enter/exit 1,200 feet from the eastern most boundary after crossing under Interstate 79. The drill will pass 15 feet under the western most boundary of the creek and 25 feet under the eastern most boundary. The 20" drill will parallel the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate that will be passed through below 12 feet is estimated to be rock. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PLAN VIEW

WASHINGTON COUNTY, PENNSYLVANIA - CHARTIERS/NORTH STRABANE TOWNSHIPS S1B-0020

PROFILE VIEW



**GEOTECH HDD-01A**  
 -NG EL. 960'  
 -SILTY CLAY/SAND (0.0' - 10.0')  
 -COMPLETION DEPTH EL. 950'  
 NOTE: REFER TO TEST BORING LOG HDD-01A FOR COMPLETE SOIL MATERIAL DESCRIPTION

- DESIGN AND CONSTRUCTION:**
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
 HDD HORZ. LENGTH (L): 1454'  
 HDD PIPE LENGTH (S): 1471'  
 20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
 COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - PIPELINE AND CROSSING TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LAST APPROVED AMERICAN RAILWAY ENGINEERING AND MAINTENANCE OF WAY ASSOCIATION SPECIFICATIONS FOR PIPELINES CONVEYING FLAMMABLE AND NON-FLAMMABLE SUBSTANCES.
  - BLASTING NOT PERMITTED.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

**REF. DRAWING**

|         |    |         |                         |
|---------|----|---------|-------------------------|
| ES-1.14 | TO | ES-1.15 | EROSION & SEDIMENT PLAN |
| SHEET 8 | TO | SHEET 9 | AERIAL SITE PLAN        |

**REVISIONS**

| NO. | DESCRIPTION                                  | BY  | DATE     | CHK | DATE     | APP | DATE     |
|-----|--|-----|----------|-----|----------|-----|----------|
| EP2 | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 | MRS | 09/30/16 | RMB | 09/30/16 | AAW | 09/30/16 |
| EP1 | REVISED PER PADEP COMMENTS                   | MRS | 05/17/16 | RMB | 05/17/16 | AAW | 05/17/16 |
| EP  |  | DLM | 03/15/16 | RMB | 03/15/16 | AAW | 03/15/16 |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
 (303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
 PITTSBURGH & OHIO CENTRAL RAILWAY, I-79  
 PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=150'    DWG. NO: PA-WA-0074.0000-RR



Source: Topo data from USGS D.L.G. Roads from DelDOT

S:\US-Projects\intra\community\RT - Rooney (RE)\GIS\Boring Map HDD-01A\_01A2\_01B.mxd



**Figure**  
**Boring Locations HDD-01A/01A2/01B**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

**Tt** Tetra Tech, Inc.  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
 www.tetrattech.com

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**TETRA TECH**  
 240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
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 fax: 302.454.5000

# TEST BORING LOG

|   |                                   |   |                                       |  |  |
|---|-----------------------------------|---|---------------------------------------|--|--|
| Project Name: SUNOCO MARINER EAST       |                                   |   | Project No.: 103IP2762                |  |  |
| Project Location: WASHINGTON COUNTY, PA |                                   |   | Page 1 of 1                           |  |  |
| Test Boring No.: HDD-01A                | Dates(s) Drilled: 06/10/13        |   | Inspector: E. WATT                    |  |  |
| Drilling Contractor: CONNELLY           | Drilling Method: SPT - ASTM D1586 |   | Driller: T. REDMAN                    |  |  |
| Surface Elevation (ft):                 |                                   | Groundwater Depth (ft): Not Encountered | Total Depth (ft): VARIOUS, SEE BELOW. |  |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |    | Recov. (in) | Strata (USCS) | Description of Materials  | 6" Increment Blows * |    |   | N  |
|------------|-------------------|------|-------------------|----|-------------|---------------|---|----------------------|----|---|----|
|            | From              | To   | From              | To |             |               |   |                      |    |   |    |
| 1          | 3.5               | 5.0  |                   |    | 6           |               | GRAY AND BROWN FINE TO MEDIUM SAND WITH SOME SILTY CLAY, TRACE FINE GRAVEL, TRACES OF WOOD (FILL)                                 | 5                    | 4  | 4 | 8  |
| 2          | 8.5               | 8.2  |                   |    | <1"         |               | NO RECOVERY, GRAVEL IN SPOON TIP.   | 50/2"                |    |   |    |
|            |                   |      |                   |    |             |               | AUGER REFUSAL AT 9.0'. OFFSET BORING 8' TO THE NORTH.   |                      |    |   |    |
| 3          | 12.0              | 12.1 |                   |    | 0           |               | CONTINUOUS DRILLING, AUGER REFUSAL AT 12'. NO RECOVERY. OFFSET BORING 13' TO THE WEST.  | 50/1"                |    |   |    |
| 4          | 13.5              | 15.0 |                   |    | 8           |               | CONTINUOUS DRILLING, AUGER REFUSAL AT 13.5'. GRAY GRAVEL AND SAND WITH SOME SILT AND CLAY (FILL). OFFSET BORING 53' TO THE NORTH. | 14                   | 14 | 7 | 21 |
|            |                   |      |                   |    |             |               | CONTINUOUS DRILLING, AUGER REFUSAL AT 10'.  |                      |    |   |    |
|            |                   |      |                   |    |             |               | ABOVE BORINGS PERFORMED IN MOUNDED HISTORIC FILL  |                      |    |   |    |
|            |                   |      |                   |    |             |               | FILL AREA; INDICATED BY STEEP SIDE SLOPES CONTAINING MISCELLANEOUS SOIL FILL, CONCRETE RUBBLE, AND MISC. DEBRIS.                  |                      |    |   |    |
|            |                   |      |                   |    |             |               | WILL PERFORM AN ADDITIONAL BORING TO THE NORTH, AT LOCATION BEYOND HISTORIC FILL AREA.  |                      |    |   |    |

Notes/Comments:  
Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.





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# TEST BORING LOG

|   |          |                         |                        |                   |          |
|---|----------|-------------------------|------------------------|-------------------|----------|
| Project Name: SUNOCO MARINER EAST       |          |                         | Project No.: 103IP2762 |                   |          |
| Project Location: WASHINGTON COUNTY, PA |          |                         | Page 1 of 1            |                   |          |
| Test Boring No.:                        | HDD-01B  | Dates(s) Drilled:       | 09/10/13               | Inspector:        | E. WATT  |
| Drilling Contractor:                    | CONNELLY | Drilling Method:        | SPT - ASTM D1586       | Driller:          | K. KERSH |
| Surface Elevation (ft):                 |          | Groundwater Depth (ft): |                        | Total Depth (ft): | 57.0     |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS)       | Description of Materials                              | 6" Increment Blows * |    |    | N  |
|------------|-------------------|------|-------------------|------|-------------|---------------------|---|----------------------|----|----|----|
|            | From              | To   | From              | To   |             |                     |   |                      |    |    |    |
| 1          | 3.5               | 5.0  | 0.0               |      | 18          | SUSPECTED FILL (SM) | BROWN FINE TO MEDIUM SAND, SILT, AND GRAVEL.          | 7                    | 13 | 20 | 33 |
|            |                   |      |                   |      |             |                     | SUSPECTED HISTORICAL FILL)                            |                      |    |    |    |
| 2          | 8.5               | 10.0 |                   |      | 17          |                     | SIMILAR TO ABOVE.                                     | 8                    | 11 | 16 | 27 |
| 3          | 12.0              | 12.0 |                   | 12.0 | 0           | SUSPECTED FILL??    | NO RECOVERY   | 50/0"                |    |    |    |
|            |                   |      |                   |      |             |                     | AUGER REFUSAL AT 12'. MADE SEVERAL FAILED ATTEMPTS TO |                      |    |    |    |
|            |                   |      |                   |      |             |                     | PENETRATE DEEPER THAN 12'.                            |                      |    |    |    |
|            |                   |      |                   |      |             |                     | ROCK CORING   |                      |    |    |    |
| RUN 1      | 12.0              | 17.0 | 12.0              |      |             | SUSPECTED FILL??    | 78% RECOVERY, 58% RQD. SUSPECTED HISTORICAL FILL:     |                      |    |    |    |
|            |                   |      |                   |      |             |                     | GRAY SANDSTONE, MICA, BROWN SILT, GRAVEL.             |                      |    |    |    |
| RUN 2      | 17.0              | 22.0 |                   |      |             |                     | 40% RECOVERY, 0% RQD. SUSPECTED HISTORICAL FILL:      |                      |    |    |    |
|            |                   |      |                   |      |             |                     | SANDSTONE, BROWN AND GRAY SILT, GRAVEL/ROCK.          |                      |    |    |    |
| RUN 3      | 22.0              | 27.0 |                   |      |             | SUSPECTED FILL??    | 33% RECOVERY, 0% RQD. SUSPECTED HISTORICAL FILL:      |                      |    |    |    |
|            |                   |      |                   |      |             |                     | SOIL, GRAVEL, ROCK.                                   |                      |    |    |    |
| RUN 4      | 27.0              | 32.0 |                   | 32.0 |             | SUSPECTED FILL??    | 60% RECOVERY, 0% RQD. SUSPECTED HISTORICAL FILL:      |                      |    |    |    |
|            |                   |      |                   |      |             |                     | SANDSTONE GRAVEL WITH MICA.                           |                      |    |    |    |
| RUN 5      | 32.0              | 37.0 | 32.0              |      |             | VIRGIN SOILS??      | 50% RECOVERY, 0% RQD. SUSPECTED HISTORICAL FILL:      |                      |    |    |    |
|            |                   |      |                   |      |             |                     | BROWN AND GRAY SILT, SAND, GRAVEL, ROCK.              |                      |    |    |    |
| RUN 6      | 37.0              | 42.0 |                   | 43.0 |             | VIRGIN SOILS??      | 22% RECOVERY, 0% RQD. SUSPECTED HISTORICAL FILL:      |                      |    |    |    |
|            |                   |      |                   |      |             |                     | BROWN AND GRAY SILT, SAND, GRAVEL, ROCK.              |                      |    |    |    |
| RUN 7      | 42.0              | 47.0 | 43.0              |      |             | ROCK                | 78% RECOVERY, 43% RQD. GRAY TO DARK GRAY              |                      |    |    |    |
|            |                   |      |                   |      |             |                     | SILTSTONE.  |                      |    |    |    |
| RUN 8      | 47.0              | 52.0 |                   |      |             |                     | 100% RECOVERY, 34% RQD. GRAY TO DARK GRAY SILTSTONE.  |                      |    |    |    |
| RUN 9      | 52.0              | 57.0 |                   | 57.0 |             | ROCK                | 87% RECOVERY, 45% RQD. GRAY TO DARK GRAY SILTSTONE.   |                      |    |    |    |

Notes/Comments:

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more  |
| Cobbles   | 3 to 8 in. diameter   |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve<br>Fine (F) ¾ in. to No. 4 sieve  |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium No. 10 to No. 40 sieve<br>(M) (2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8- 22                   |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

| Major Divisions   |   | Group Symbols  | Typical Descriptions  | Laboratory Classifications   |   |   |  |  |
|---|---|--|---|--|---|---|--|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve)                       | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size)            | Clean gravel<br>(Little or no fines)   | GW<br>Well-graded gravels, gravel-sand mixtures, little or no fines | Determine Percentage of sand and gravel from grain size curve.<br>Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |   |  |  |
|   |   | GP<br>Poorly graded gravels, gravel-sand mixtures, little or no fines            | Not meeting $C_u$ or $C_c$ requirements for GW                      |  |   |   |  |  |
|   |   | Gravel with fines<br>(Appreciable amount of fines)                               | GM<br>Silty gravels, gravel-sand-silt mixtures                      |  | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |  |
|   |   |  | GC<br>Clayey gravels, gravel-sand-clay mixtures                     |  | Atterberg limits above A line with $I_p$ greater than 7   |   |  |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)                  | Clean sands<br>(Little or no fines)  | SW<br>Well graded sands, gravelly sands, little or no fines         |  | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |   |  |  |
|   |   |  | SP<br>Poorly graded sands, gravelly sands, little or no fines       |  | Not meeting $C_u$ or $C_c$ requirements for SW  |   |  |  |
|   |   | Sands with fines<br>(Appreciable amount of fines)                                | SM<br>Silty sands, sand-silt mixtures                               |  | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |  |
|   |   |  | SC<br>Clayey sands, sand-clay mixtures                              |  | Atterberg limits above A line with $I_p$ greater than 7   |   |  |  |
|   |   |  |   |  |   | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |  |
|   |   | Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve) | Silt and clays<br>(Liquid limit less than 50)                       |  | ML<br>Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |  |  |
| CL<br>Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays |   |  |   |  |   |   |  |  |
| OL<br>Organic silts and organic silty clays of low plasticity   |   |  |   |  |   |   |  |  |
| Silt and Clays (Liquid limit greater than 50)   | MH<br>Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts |  |   |  |   |   |  |  |
|   | CH<br>Inorganic clays of high plasticity, fat clays                                       |  |   |  |   |   |  |  |
|   | OH<br>Organic clays of medium to high plasticity, organic silts                           |  |   |  |   |   |  |  |
| Highly organic soils  | Pt<br>Peat and other highly organic soils   |  |   |  |   |   |  |  |

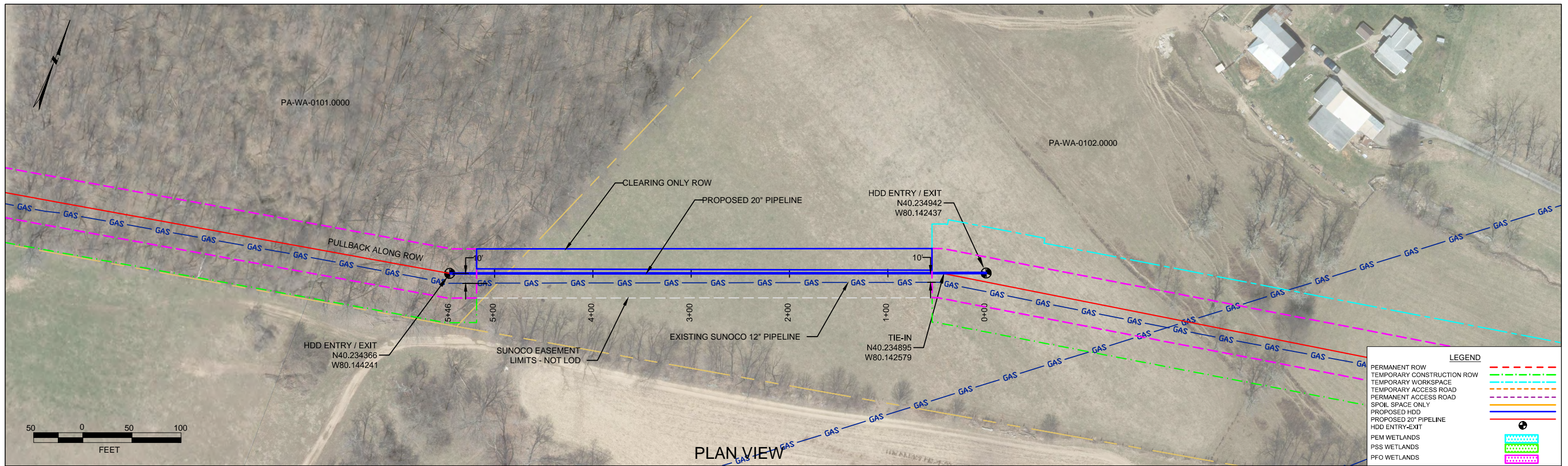
(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0102.0000-SR***

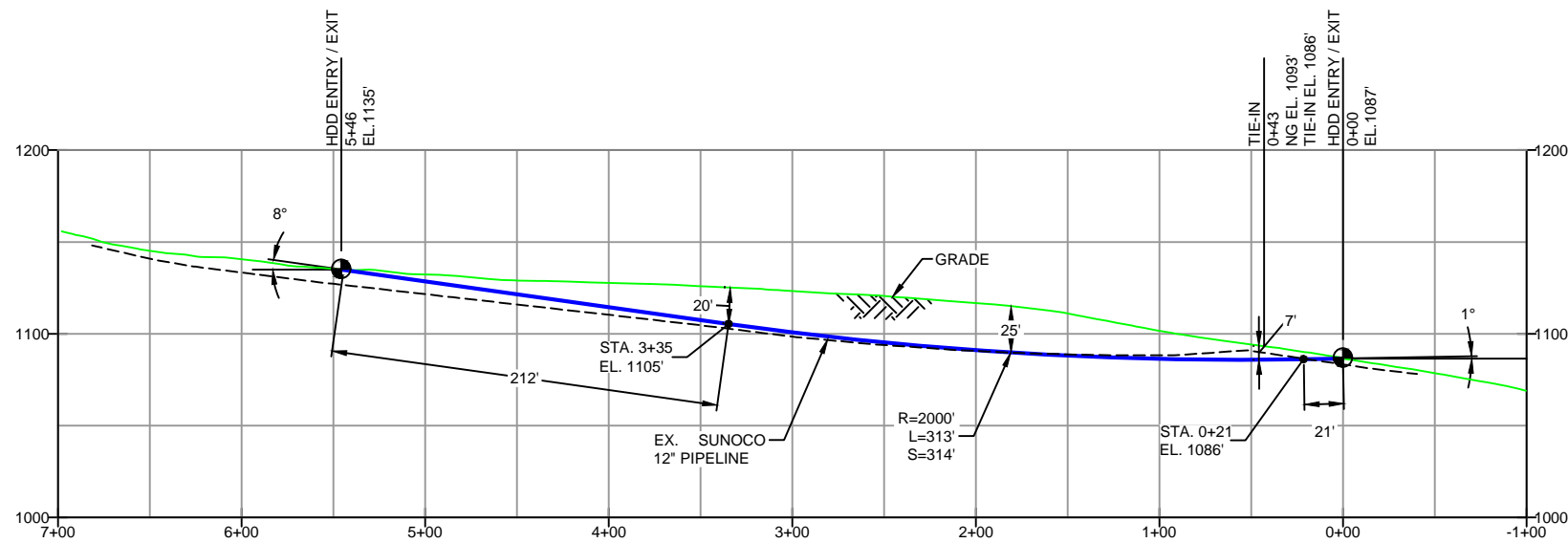
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be medium. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The horizontal directional drill is designed to prevent open cutting on a hillside. There are no crossings of roads, water bodies, or wetlands, nor are any of these features close to the drill location. The drill parallels the existing ME1 12" pipeline drill. No geotechnical boring was completed at this location.

There is no current geotechnical data on which to determine soil types and potential loss of drilling fluid causing the risk to be higher than locations with good drilling soils. However, there are also no major environmental areas of impact (wetlands, waterways, etc.) so the environmental risk for inadvertent returns is medium.



PROFILE VIEW



DESIGN AND CONSTRUCTION:

- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
- THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
- DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L)=546'  
HDD PIPE LENGTH (S)=547'  
20" x 0.456" W.T., X-65, APISL, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
- INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
- PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
- CARRIER PIPE NOT ENCASED
- PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER
- CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
- SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
- SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |        | REVISIONS   |  |
|--------------|--------|-------------|--|
| ES-1.28      | TO     | ES-1.28     | EROSION & SEDIMENT PLAN                      |
| SHEET 17     | TO     | SHEET 17    | AERIAL SITE PLAN                             |
|              |        | EP2         | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |        | EP1         | REVISED PER PADEP COMMENTS                   |
|              |        | EP          |  |
| DWG NO       | DWG NO | DESCRIPTION | NO.  |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
CULTURAL AREA #2  
PENNSYLVANIA PIPELINE PROJECT

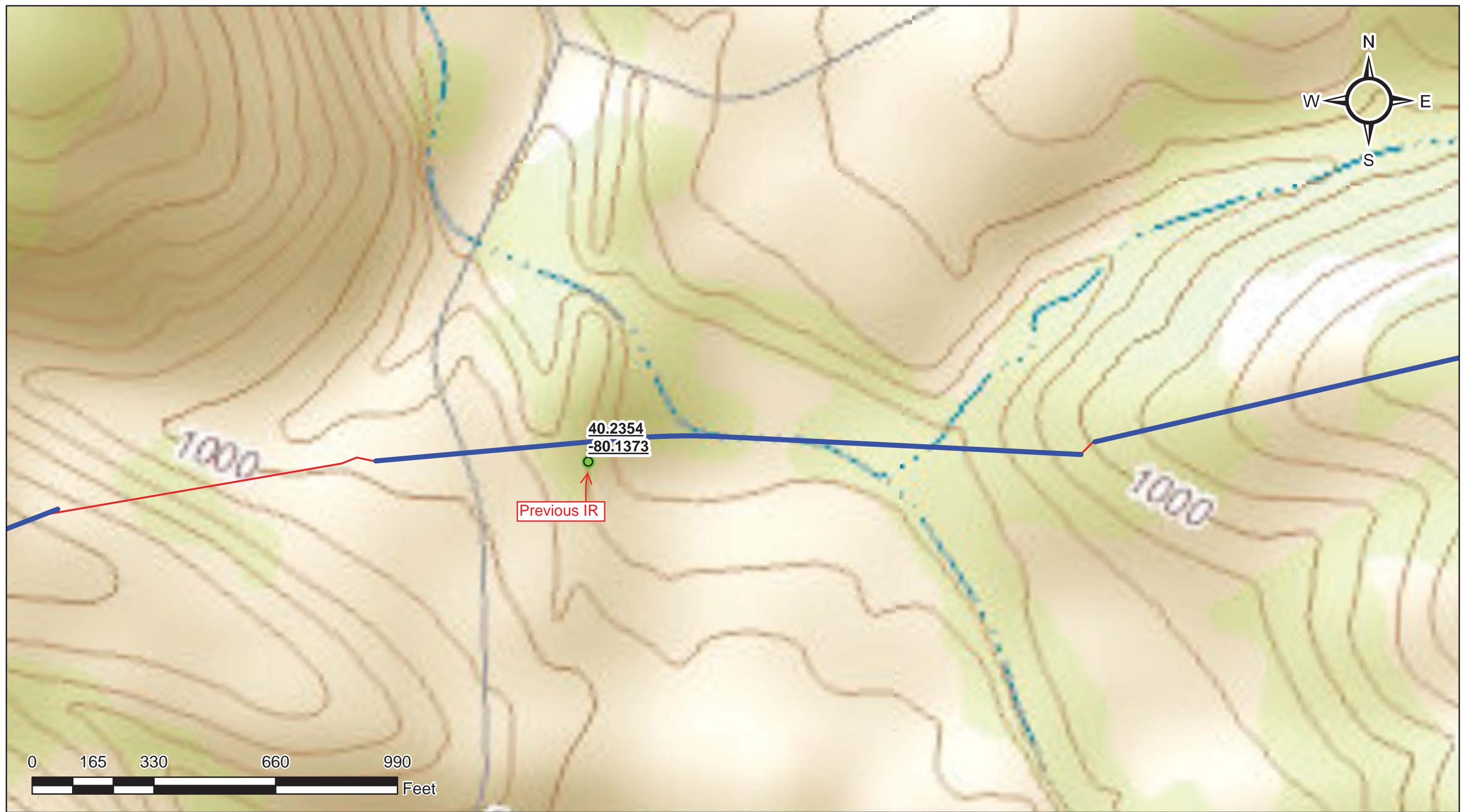
SCALE: 1"=100'    DWG. NO: PA-WA-0102.0000-SR

***HDD PA-WA-0103.0000-RD (S16, S250)***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be medium. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 890 feet from the western edge of Linden Creek (S16) and enter/exit 607 feet from the eastern edge. The horizontal directional drill will enter/exit 1,530 feet from the western edge of Stream 250 (S250) and enter/exit 380 feet from the eastern edge. From Linden Road the drill will enter/exit 226 feet from the western edge and enter/exit 1,660 feet from the eastern edge. The drill will cross 56 feet below Linden Creek Road, 41 feet below Linden Creek, and 25 feet below S250. The 20" drill will closely follow the existing ME1 12" pipeline drill, which had an inadvertent return.

A geotechnical boring was not completed for this drill location for either the ME1 project or the current ME2 project. The lack of a geotechnical report, and a waterway require an increase in the risk assessment for potential environmental impacts of inadvertent returns. The depth of crossing at the road and the creek are such that the potential for inadvertent returns, and their impacts, is mitigated to an extent. A previous drill in this location resulted in 700 gallons of inadvertent returns to the surface. Sandbags and a containment box were built to prevent the spread of the returns and no aquatic resources were impacted. The environmental risk for inadvertent returns associated with this drill is a medium. As such it is recommended that additional inspection in the area surrounding the drill be in place to monitor for potential inadvertent returns. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.



**Legend**

- HDD Path
- Proposed Centerline

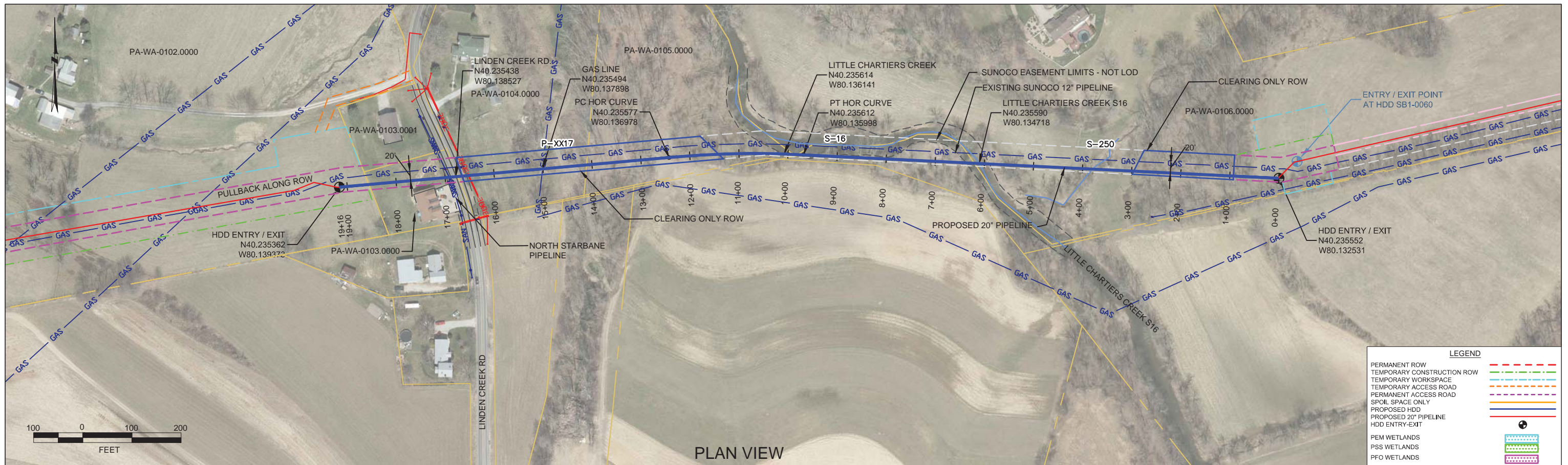
| DRAWN | CHECKED | APPROVED | DATE     | REV NO. | DESCRIPTION       |
|-------|---------|----------|----------|---------|-------------------|
| AW    | RB      | RB       | 11/21/16 | A       | ISSUED FOR REVIEW |

PREPARED BY:  
 TETRA (303) 79

Sunoco Pipeline L.P.  
 PENNSYLVANIA PIPELINE PROJECT  
 INADVERTENT RETURN ASSESSMENTS

|                |                 |
|----------------|-----------------|
| DRAWN: AW      | CHECKED: RB     |
| DATE: 11/21/16 | SCALE: AS SHOWN |

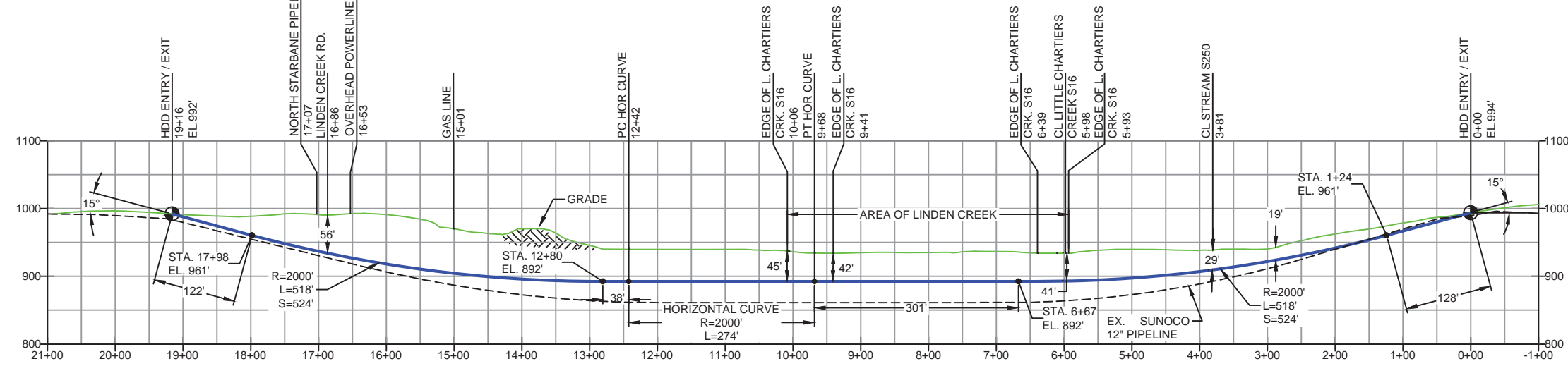
**PA-WA-0103-RD-HDD**



PLAN VIEW

WASHINGTON COUNTY, PENNSYLVANIA, STRABANE TOWNSHIP  
S1B-0050

PROFILE VIEW



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L): 1916'  
HDD PIPE LENGTH (S): 1911'  
20" x 0.456" W.T., X-65, APISL, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

| NOTES |  |
|-------|--|
| 1.    | ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83  |
| 2.    | STATIONING IS BASED ON HORIZONTAL DISTANCES.   |
| 3.    | ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN. |
| 4.    | CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.  |
| 5.    | SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.  |

| REF. DRAWING |             | REVISIONS               |  |
|--------------|-------------|-------------------------|--|
| ES-1.20      | TO ES-1.30  | EROSION & SEDIMENT PLAN |  |
| SHEET 17     | TO SHEET 17 | AERIAL SITE PLAN        |  |
|              |             | EP2                     | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |             | EP1                     | REVISED PER PADEP COMMENTS                   |
|              |             | EP                      |  |
| DWG NO       | DWG NO      | DESCRIPTION             | NO.  |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
LINDEN CREEK ROAD  
PENNSYLVANIA PIPELINE PROJECT

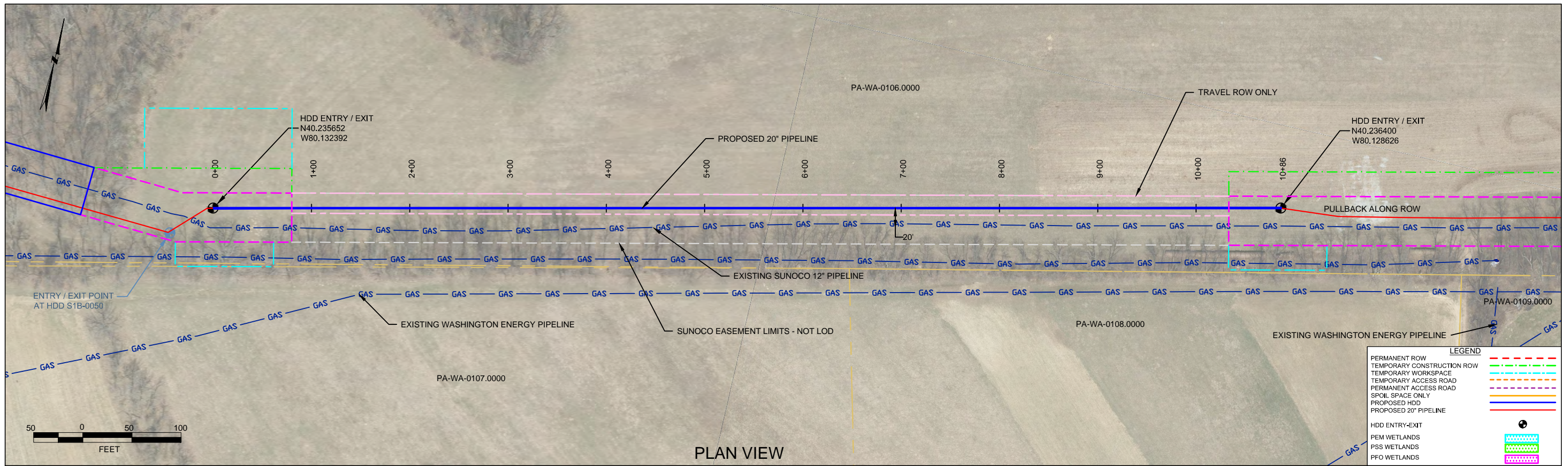
SCALE: 1"=200'    DWG. NO: PA-WA-0103.0000-RD

***HDD PA-WA-0106.0000-SR***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be medium. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The horizontal directional drill is designed to prevent open cutting on a hillside. There are no crossings of roads, water bodies, or wetlands, nor are any of these features close to the drill location. The drill parallels the existing ME1 12" pipeline drill. No geotechnical boring was completed at this location.

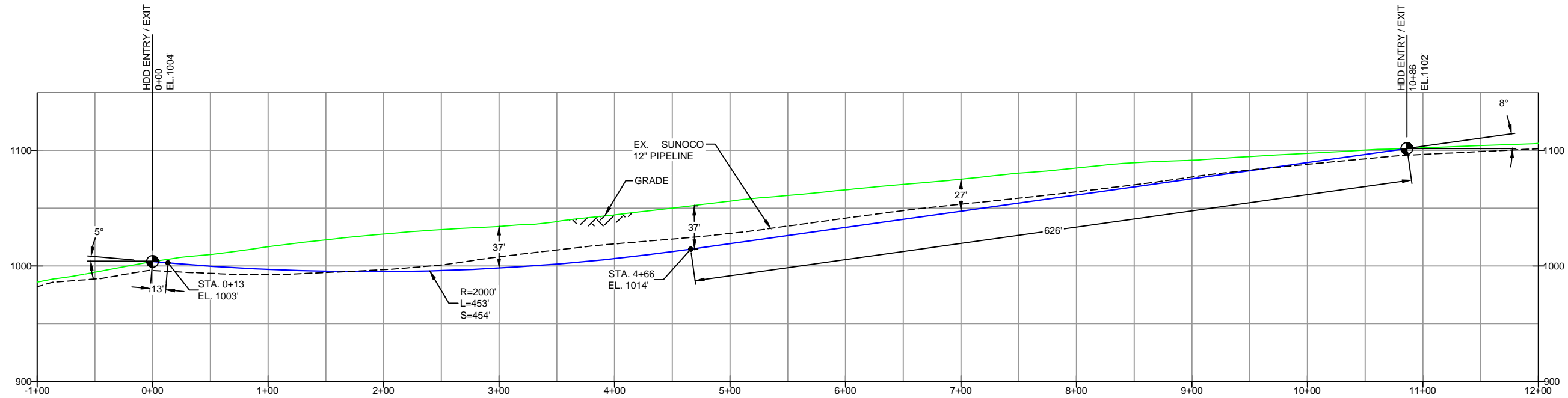
There is no current geotechnical data on which to determine soil types and potential loss of drilling fluid causing the risk to be higher than locations with good drilling soils. However, there are also no major environmental areas of impact (wetlands, waterways, etc.) so the environmental risk for inadvertent returns is medium. As such it is recommended that additional inspection in the area surrounding the drill be in place to monitor for potential inadvertent returns.



PLAN VIEW

WASHINGTON COUNTY, PENNSYLVANIA - NORTH STRABANE TOWNSHIP  
S1B-0060

PROFILE VIEW



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L-): 1086'  
HDD PIPE LENGTH (S-): 1093'  
20" x 0.456" W.T., X-65, APISL, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |    | EROSION & SEDIMENT PLAN |             | AERIAL SITE PLAN                             |             |
|--------------|----|-------------------------|-------------|--|-------------|
| ES-1.30      | TO | ES-1.31                 |             |  |             |
| SHEET 19     | TO | SHEET 19                |             |  |             |
|              |    |                         | EP2         | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |             |
|              |    |                         | EP1         | REVISED PER PADEP COMMENTS                   |             |
|              |    |                         | EP          |  |             |
| DWG NO       | TO | DWG NO                  | DESCRIPTION | NO.  | DESCRIPTION |

**REVISIONS**

| BY  | DATE     | CHK | DATE     | APP | DATE     |
|-----|----------|-----|----------|-----|----------|
| MRS | 09/30/16 | RMB | 09/30/16 | AAW | 09/30/16 |
| MRS | 05/17/16 | RMB | 05/17/16 | AAW | 05/17/16 |
| JTW | 03/15/16 | RMB | 03/15/16 | AAW | 03/15/16 |

(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
CULTURAL AREA #3  
PENNSYLVANIA PIPELINE PROJECT

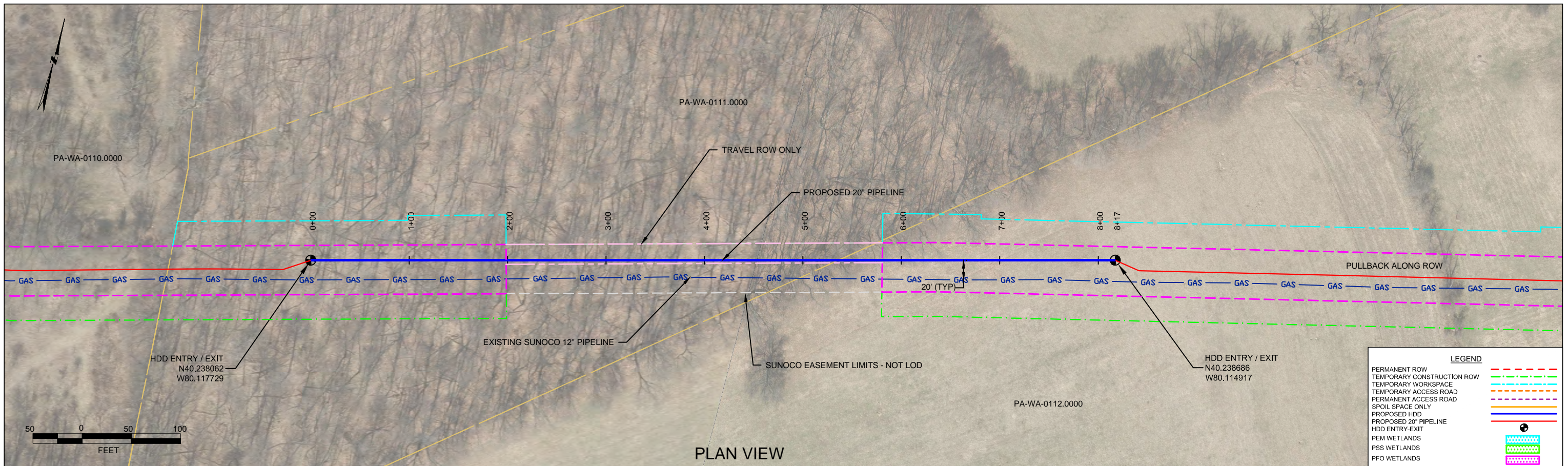
SCALE: 1"=100'    DWG. NO: PA-WA-0106.0000-SR

***HDD PA-WA-0111.0000-SR***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be medium. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

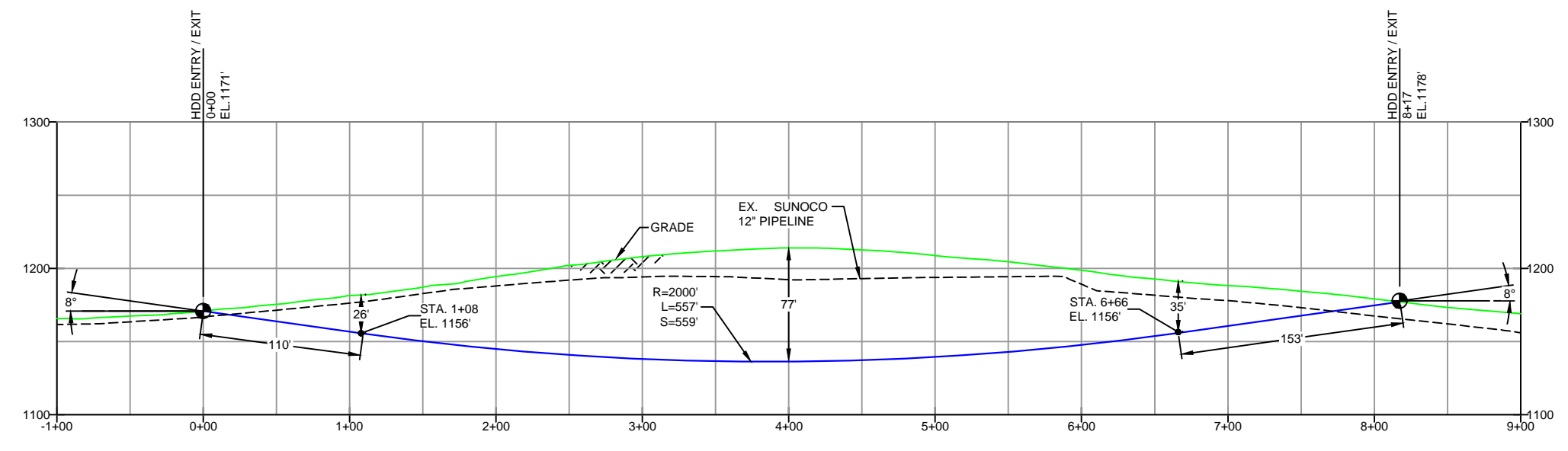
The horizontal directional drill is designed to prevent open cutting a hillside. There are no crossings of roads, water bodies, or wetlands, nor are any of these features close to the drill location. The drill parallels the existing ME1 12" pipeline drill. No geotechnical boring was completed at this location.

There is no current geotechnical data on which to determine soil types and potential loss of drilling fluid causing the risk to be higher than locations with good drilling soils. However, there are also no major environmental areas of impact (wetlands, waterways, etc.) so the environmental risk for inadvertent returns is medium. As such it is recommended that additional inspection in the area surrounding the drill be in place to monitor for potential inadvertent returns.



PLAN VIEW  
PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA - NORTH STRABANE TOWNSHIP  
S1B-0070



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L=): 818'  
HDD PIPE LENGTH (S=): 822'  
20" x 0.456" W.T., X-70, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

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- STATIONING IS BASED ON HORIZONTAL DISTANCES.
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- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

**REF. DRAWING**

| ES-1.33  | TO | ES-1.33  | EROSION & SEDIMENT PLAN |
|----------|----|----------|-------------------------|
| SHEET 20 | TO | SHEET 20 | AERIAL SITE PLAN        |

**REVISIONS**

| NO. | DESCRIPTION                                  | BY  | DATE     | CHK | DATE     | APP | DATE     |
|-----|--|-----|----------|-----|----------|-----|----------|
| EP2 | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 | MRS | 09/30/16 | RMB | 09/30/16 | AAW | 09/30/16 |
| EP1 | REVISED PER PADEP COMMENTS                   | MRS | 05/17/16 | RMB | 05/17/16 | AAW | 05/17/16 |
| EP  |  | JTW | 03/15/16 | RMB | 03/15/16 | AAW | 03/15/16 |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

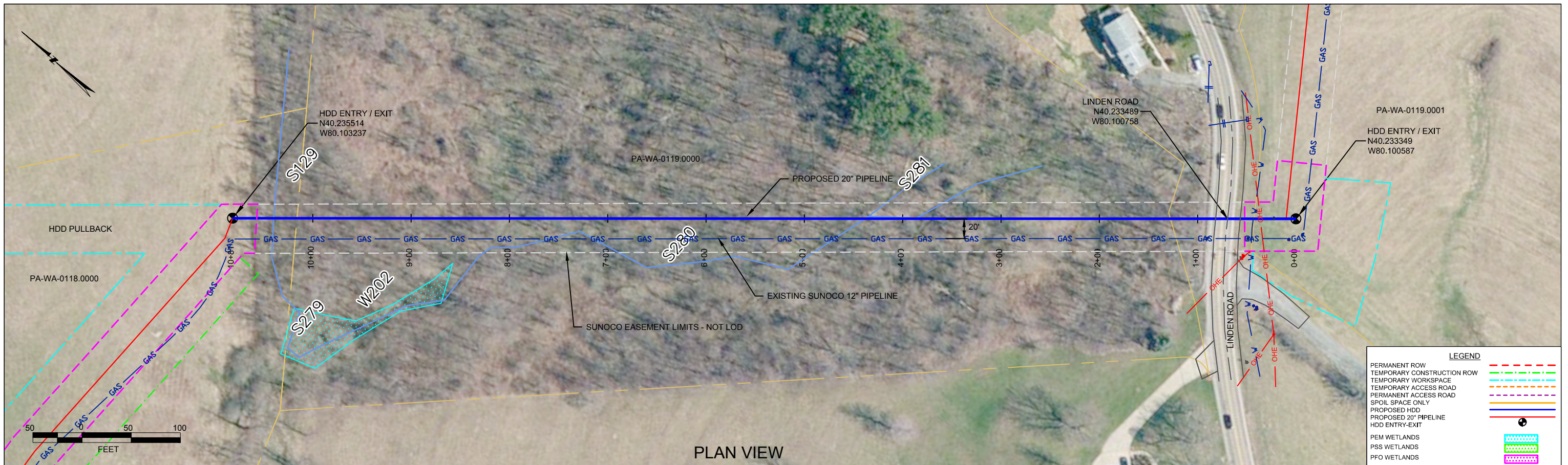
20-INCH HORIZONTAL DIRECTIONAL DRILL  
CULTURAL AREA #4  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'  
DWG. NO: PA-WA-0111.0000-SR

***HDD PA-WA-0119.0000-RD (S129, S279, S280, S281, W202)***

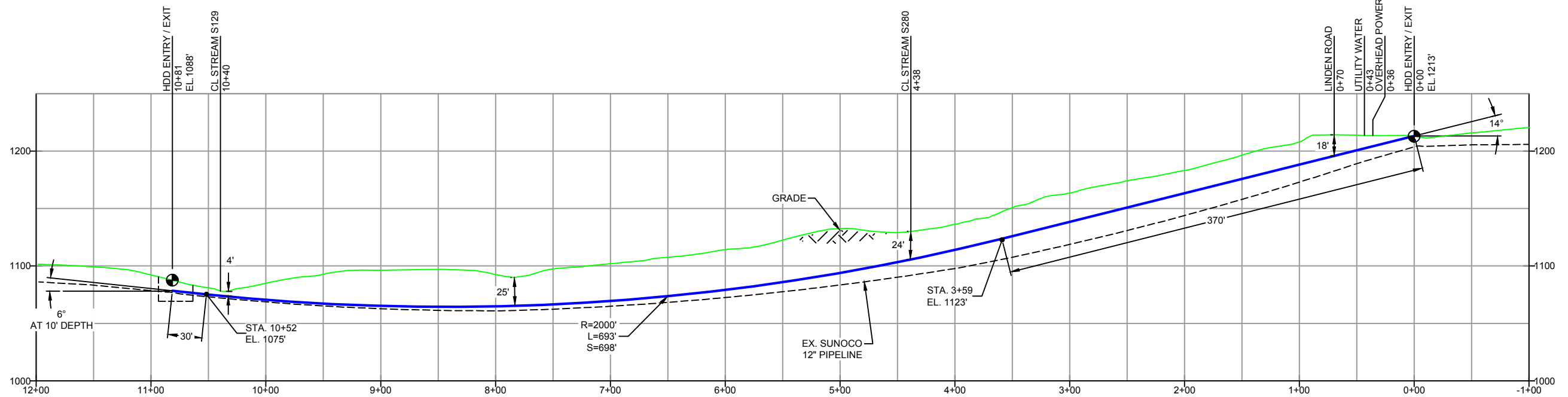
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 30 feet from the western edge of a Stream 129 (S129) and enter/exit 1000 feet from the eastern edge. The horizontal directional drill will enter/exit 650 feet from the western edge of streams 280 and 281 (S280, S281) and enter/exit 430 feet from the eastern edge. The drill will cross below S129 at 4 feet and S280/S281 at 25 feet. The 20" drill will parallel the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate at both crossings (S129 and S280/S281) is estimated to be rock. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PLAN VIEW  
PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA - NORTH STRABANE TOWNSHIP  
S1B-0080



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L=): 1081'  
HDD PIPE LENGTH (S=): 1098'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

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- STATIONING IS BASED ON HORIZONTAL DISTANCES.
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- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |        | EROSION & SEDIMENT PLAN |  |
|--------------|--------|-------------------------|--|
| ES-1.35      | TO     | ES-1.35                 | EROSION & SEDIMENT PLAN                      |
| SHEET 22     | TO     | SHEET 22                | AERIAL SITE PLAN                             |
|              |        | EP2                     | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |        | EP1                     | REVISED PER PADEP COMMENTS                   |
|              |        | EP                      |  |
| DWG NO       | DWG NO | DESCRIPTION             | NO.  |

**REVISIONS**

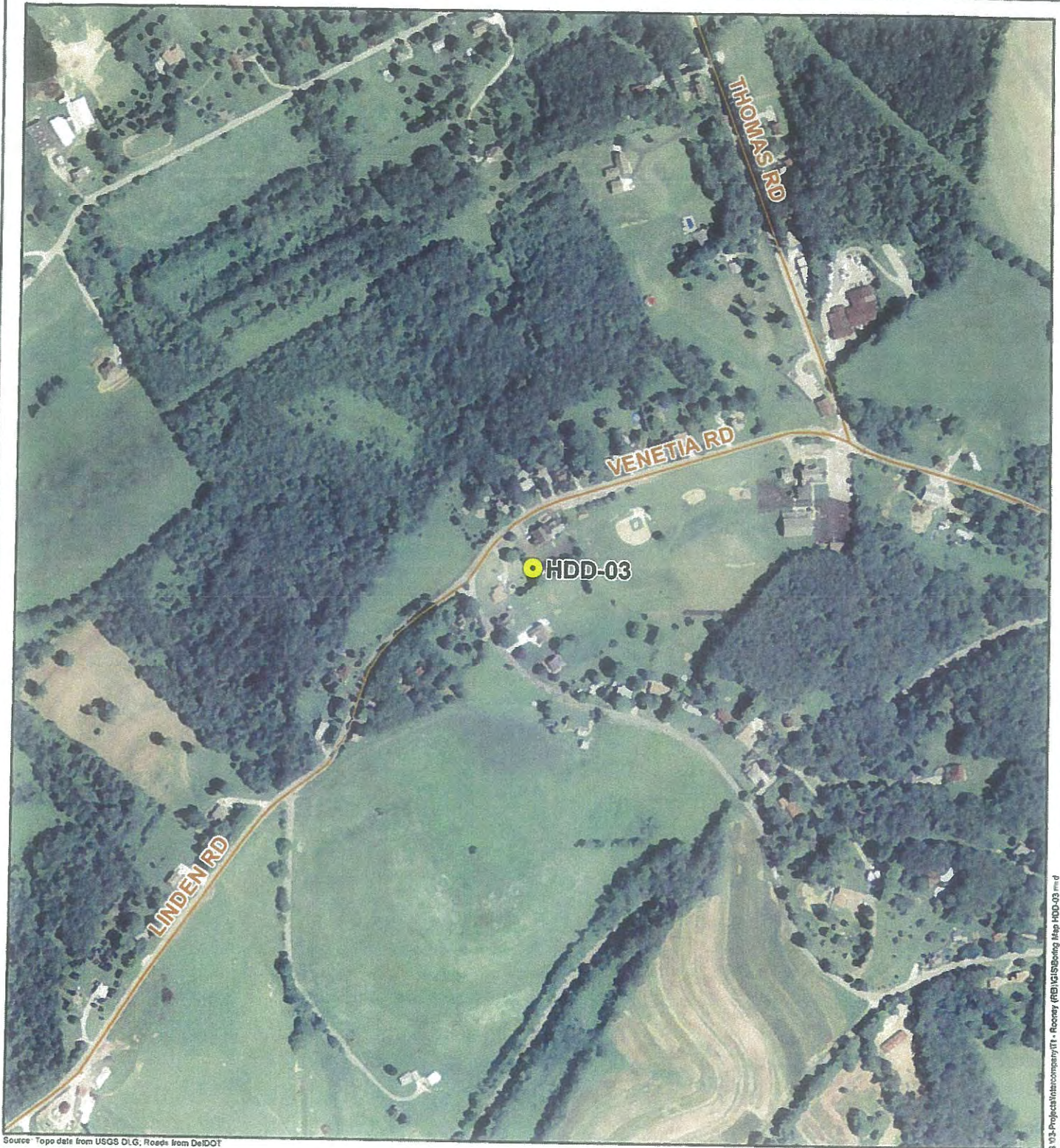
| BY  | DATE     | CHK | DATE     | APP | DATE     |
|-----|----------|-----|----------|-----|----------|
| MRS | 09/30/16 | RMB | 09/30/16 | AAW | 09/30/16 |
| MRS | 05/17/16 | RMB | 05/17/16 | AAW | 05/17/16 |
| DLM | 03/15/16 | RMB | 03/15/16 | AAW | 03/15/16 |



**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
LINDEN ROAD  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'    DWG. NO: PA-WA-0119.0000-RD

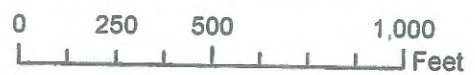


Source: Topo data from USGS DLG, Roads from DelDOT

S:\03-Projects\InfoCompany\IT - Rooney (R)\GIS\Boring Map HDD-03.mxd



**Figure**  
**Boring Location HDD-03**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

**Tt** Tetra Tech, Inc.  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
 www.tetrattech.com

This map is provided by Tetra Tech solely for display and reference purposes and is subject to change without notice. No claims, either real or assumed, as to the absolute accuracy or precision of any data contained herein are made by Tetra Tech, nor will Tetra Tech be held responsible for any use of this document for purposes other than which it was intended.





**TETRA TECH**  
 240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
 302.739.7551  
 fax: 302.454.5000

# TEST BORING LOG

Project Name: SUNOCO MARINER EAST Project No.: 103IP2762

Project Location: WASHINGTON COUNTY, PA Page 1 of 1

Test Boring No.: HDD-03 CORING Dates(s) Drilled: 09/10/13 Inspector: E. WATT

Drilling Contractor: CONNELLY Drilling Method: SPT - ASTM D1586 Driller: K. KERSH

Surface Elevation (ft): Groundwater Depth (ft): Not Encountered Total Depth (ft): 37.0

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (ft) | Strata (USCS) | Description of Materials  | 6" Increment Blows * |  |  | N |
|------------|-------------------|------|-------------------|------|-------------|---------------|---|----------------------|--|--|---|
|            | From              | To   | From              | To   |             |               |   |                      |  |  |   |
|            |                   |      | 0.0               | 27.0 |             |               | CONTINUOUS AUGERING. SEE BORING LOG HDD-03.                                   |                      |  |  |   |
|            |                   |      |                   |      |             |               | AUGER REFUSAL AT 27.0'.   |                      |  |  |   |
|            |                   |      |                   |      |             |               | <u>ROCK CORING</u>  |                      |  |  |   |
| RUN 1      | 27.0              | 32.0 | 27.0              |      |             | ROCK          | 97% RECOVERY, 87% RQD: INTERBEDED GRAY TO DARK GRAY SANDSTONE AND SILTSTONE.  |                      |  |  |   |
| RUN 2      | 32.0              | 37.0 |                   | 37.0 |             |               | 100% RECOVERY, 65% RQD: INTERBEDED GRAY TO DARK GRAY SANDSTONE AND SILTSTONE. |                      |  |  |   |
|            |                   |      |                   |      |             |               | GROUNDWATER NOT ENCOUNTERED WITHIN OVERBURDEN.                                |                      |  |  |   |

Notes/Comments:

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split- spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more                                  |
| Cobbles   | 3 to 8 in. diameter                                     |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve                         |
|           | Fine (F) ¾ in. to No. 4 sieve                           |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)     |
|           | Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm) |
|           | Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm)   |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)                    |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

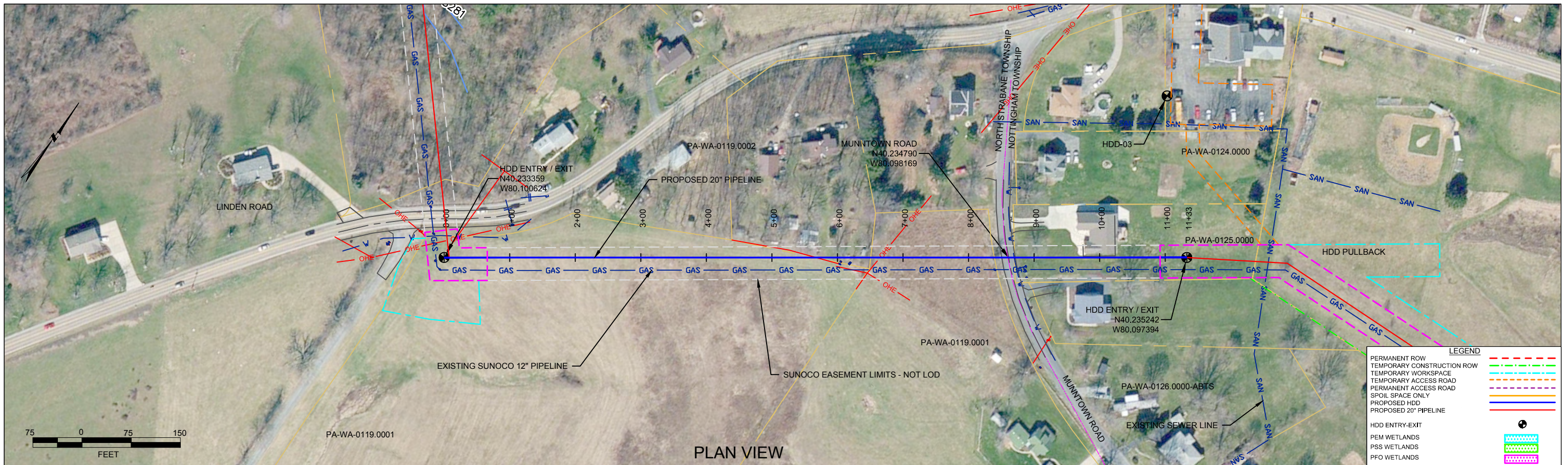
| Major Divisions   |   | Group Symbols  | Typical Descriptions  | Laboratory Classifications   |   |   |  |  |
|---|---|--|---|--|---|---|--|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve)                       | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size)            | Clean gravel<br>(Little or no fines)   | GW<br>Well-graded gravels, gravel-sand mixtures, little or no fines | Determine Percentage of sand and gravel from grain size curve.<br>Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |   |  |  |
|   |   | GP<br>Poorly graded gravels, gravel-sand mixtures, little or no fines            | Not meeting $C_u$ or $C_c$ requirements for GW                      |  |   |   |  |  |
|   |   | Gravel with fines<br>(Appreciable amount of fines)                               | GM<br>Silty gravels, gravel-sand-silt mixtures                      |  | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |  |
|   |   |  | GC<br>Clayey gravels, gravel-sand-clay mixtures                     |  | Atterberg limits above A line with $I_p$ greater than 7   |   |  |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)                  | Clean sands<br>(Little or no fines)  | SW<br>Well graded sands, gravelly sands, little or no fines         |  | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |   |  |  |
|   |   |  | SP<br>Poorly graded sands, gravelly sands, little or no fines       |  | Not meeting $C_u$ or $C_c$ requirements for SW  |   |  |  |
|   |   | Sands with fines<br>(Appreciable amount of fines)                                | SM<br>Silty sands, sand-silt mixtures                               |  | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |  |
|   |   |  | SC<br>Clayey sands, sand-clay mixtures                              |  | Atterberg limits above A line with $I_p$ greater than 7   |   |  |  |
|   |   |  |   |  |   | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |  |
|   |   | Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve) | Silt and clays<br>(Liquid limit less than 50)                       |  | ML<br>Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |  |  |
| CL<br>Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays |   |  |   |  |   |   |  |  |
| OL<br>Organic silts and organic silty clays of low plasticity   |   |  |   |  |   |   |  |  |
| Silt and Clays (Liquid limit greater than 50)   | MH<br>Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts |  |   |  |   |   |  |  |
|   | CH<br>Inorganic clays of high plasticity, fat clays                                       |  |   |  |   |   |  |  |
|   | OH<br>Organic clays of medium to high plasticity, organic silts                           |  |   |  |   |   |  |  |
| Highly organic soils  | Pt<br>Peat and other highly organic soils   |  |   |  |   |   |  |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0119.0003-RD***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 860 feet from the southern edge of Munntown Road and will enter/exit 260 feet from the northern edge. There are no known water crossings or wetlands associated with this drill that parallels the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate for the drill is rock while the substrate closest to the surface is clay. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PLAN VIEW

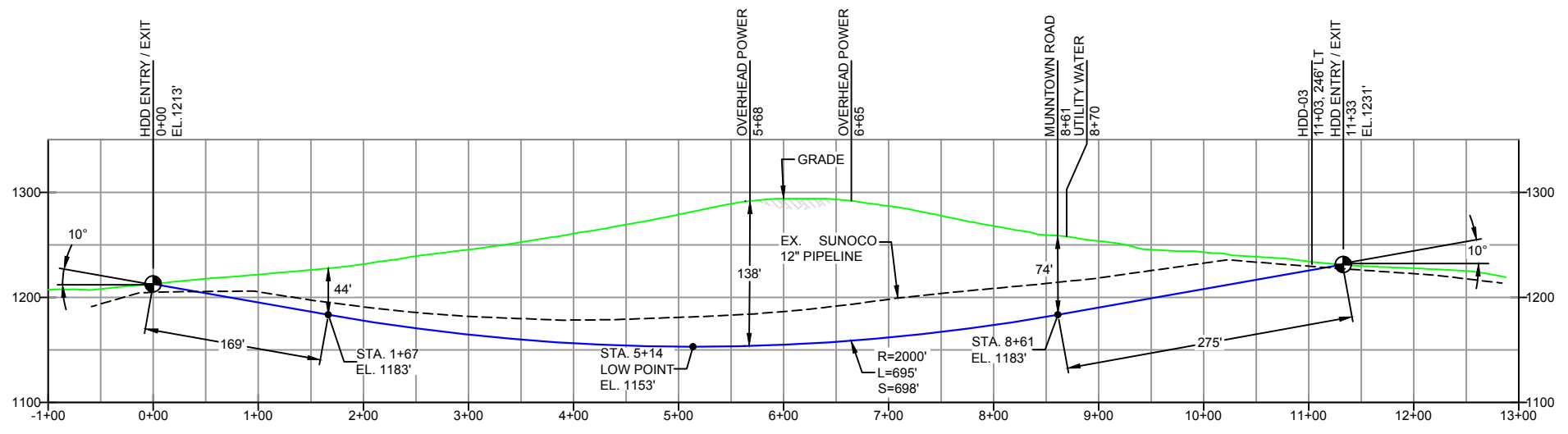
PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA - NORTH STRABANE/NOTTINGHAM TOWNSHIPS  
S1B-0090

**GEOTECH HDD-03**

- NG EL. 1232'
- CL (0.0' - 7.5')
- SILTY CLAY (7.5' - 11.0')
- SM (11.0' - 19.0')
- SILT STONE (19.0' - 23.3')
- COMPLETION DEPTH EL. 1210'

NOTE: REFER TO TEST BORING LOG HDD-03 FOR COMPLETE SOIL MATERIAL DESCRIPTION



- DESIGN AND CONSTRUCTION:**
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L)=1133'  
HDD PIPE LENGTH (S)=1142'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |             | REVISIONS               |  |             |          |      |          |      |          |      |
|--------------|-------------|-------------------------|--|-------------|----------|------|----------|------|----------|------|
| ES-1.35      | TO ES-1.35  | EROSION & SEDIMENT PLAN |  |             |          |      |          |      |          |      |
| SHEET 22     | TO SHEET 22 | AERIAL SITE PLAN        |  |             |          |      |          |      |          |      |
|              |             | EP2                     | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 | MRS         | 09/30/16 | RMB  | 09/30/16 | AAW  | 09/30/16 |      |
|              |             | EP1                     | REVISED PER PADEP COMMENTS                   | MRS         | 05/06/16 | RMB  | 05/06/16 | AAW  | 05/06/16 |      |
|              |             | EP                      |  | JTW         | 03/15/16 | RMB  | 03/15/16 | AAW  | 03/15/16 |      |
| DWG NO       | DWG NO      | DESCRIPTION             | NO.  | DESCRIPTION | BY       | DATE | CHK      | DATE | APP      | DATE |

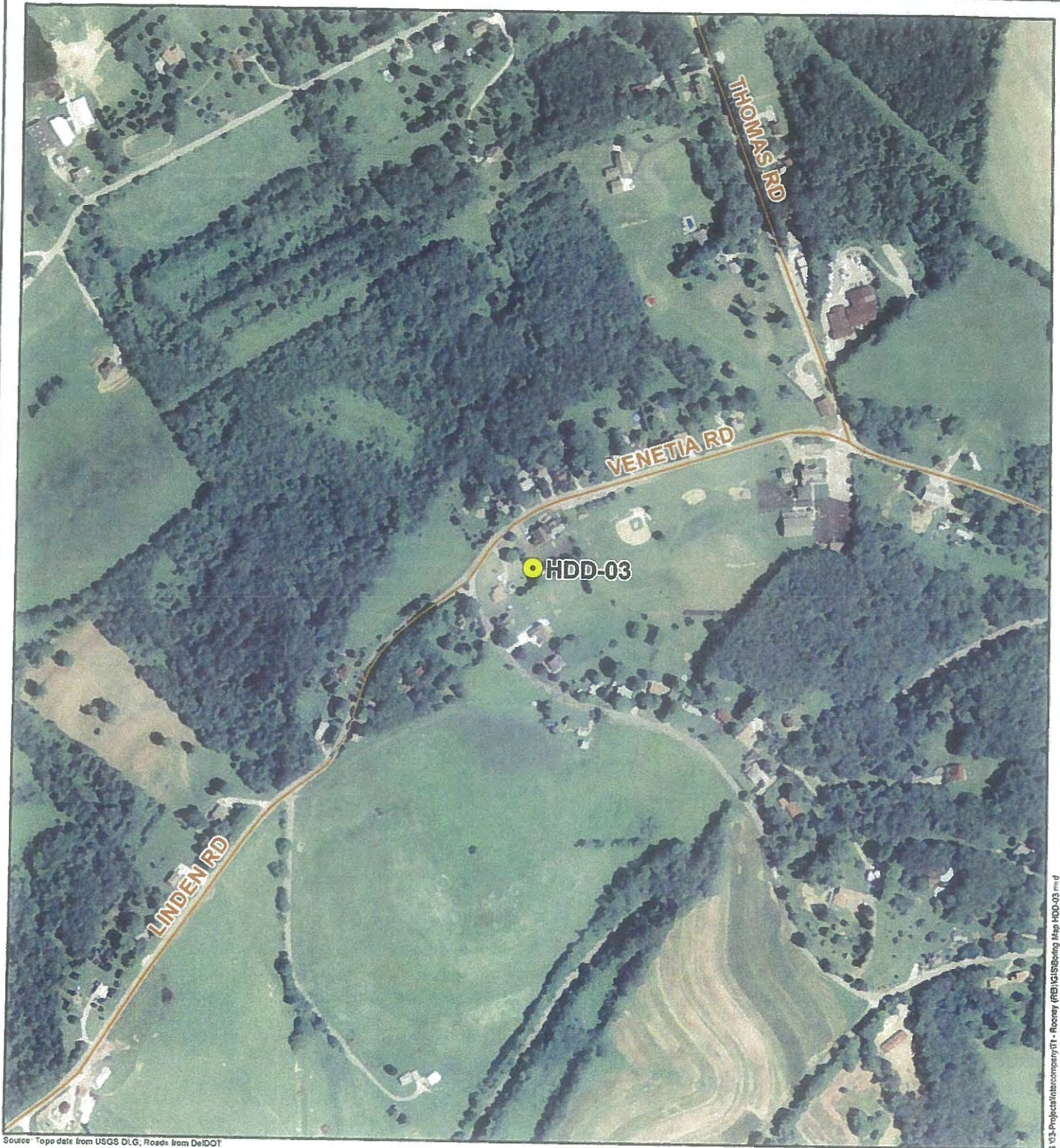
**Sunoco Logistics  
Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
MUNNTOWN ROAD  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=150'    DWG. NO: PA-WA-0119.0003-RD

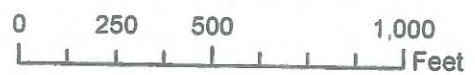


Source: Topo data from USGS DLG, Roads from DelDOT

S:\03-Projects\InfoCompany\IT - Rooney (R)\GIS\Boring Map HDD-03.mxd



**Figure**  
**Boring Location HDD-03**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

**Tt** Tetra Tech, Inc.  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
 www.tetrattech.com

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# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more                                  |
| Cobbles   | 3 to 8 in. diameter                                     |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve                         |
|           | Fine (F) ¾ in. to No. 4 sieve                           |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)     |
|           | Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm) |
|           | Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm)   |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)                    |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

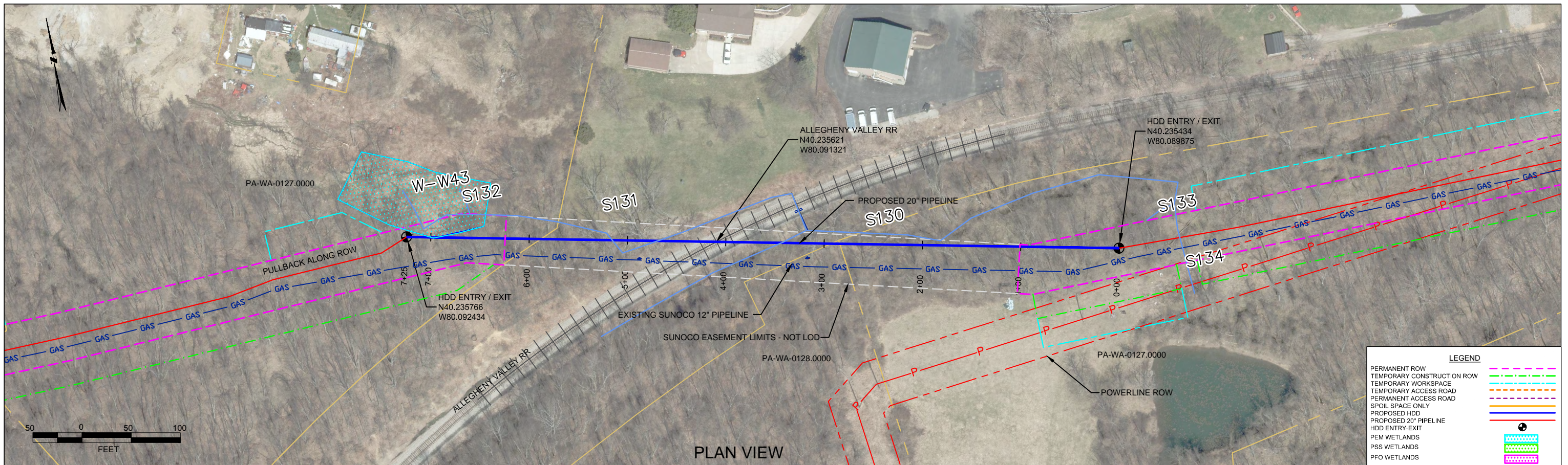
| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |   |   |  |
|---|--|---|---|---|---|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>More than half of coarse fraction is larger than No. 4 sieve size | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |   |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)     | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |   |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |   |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions  | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                   | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |   |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |   |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |   |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |   |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |   |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |   |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |   |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0127.0000-RD (W43, S131, S130)***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

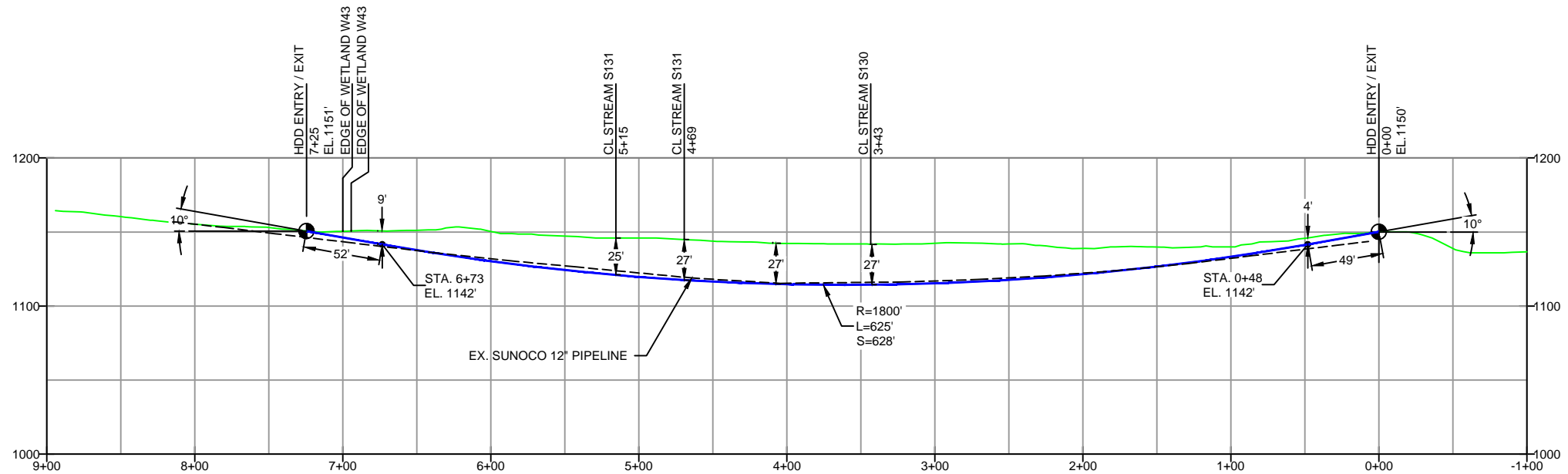
The drill will enter/exit 30 feet from the western edge of the grassy wetland (W43) and enter/exit 700 feet from the eastern edge with a depth of 5 feet. The horizontal directional drill will enter/exit 200 feet from the western edge and enter/exit 520 feet from the eastern edge and crossing at a depth of 25 feet from of the first crossing of Stream 131 (S131). For second crossing of S131 the drill will enter/exit 250 feet from the western edge and enter/exit 470 feet from the eastern edge of the drill and crossing at a depth of about 25 feet. The drill will enter/exit 375 feet from the western edge of Stream 130 (S130) and 345 feet from the eastern edge with a crossing depth of about 27 feet. The 20" drill will parallel the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate at all crossings is estimated to be a sandy clay. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PLAN VIEW

PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA - NOTTINGHAM TOWNSHIP  
S1B-0100



- DESIGN AND CONSTRUCTION:**
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L=): 725'  
HDD PIPE LENGTH (S=): 729'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, FBW  
COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - PIPELINE AND CROSSING TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LAST APPROVED AMERICAN RAILWAY ENGINEERING AND MAINTENANCE OF WAY ASSOCIATION SPECIFICATIONS FOR PIPELINES CONVEYING FLAMMABLE AND NON-FLAMMABLE SUBSTANCES.
  - BLASTING NOT PERMITTED.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |        | REVISIONS   |  |
|--------------|--------|-------------|--|
| ES-1.37      | TO     | ES-1.38     | EROSION & SEDIMENT PLAN                      |
| SHEET 23     | TO     | SHEET 23    | AERIAL SITE PLAN                             |
|              |        | EP2         | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |        | EP1         | REVISED PER PADEP COMMENTS                   |
|              |        | EP          |  |
| DWG NO       | DWG NO | DESCRIPTION | NO.  |

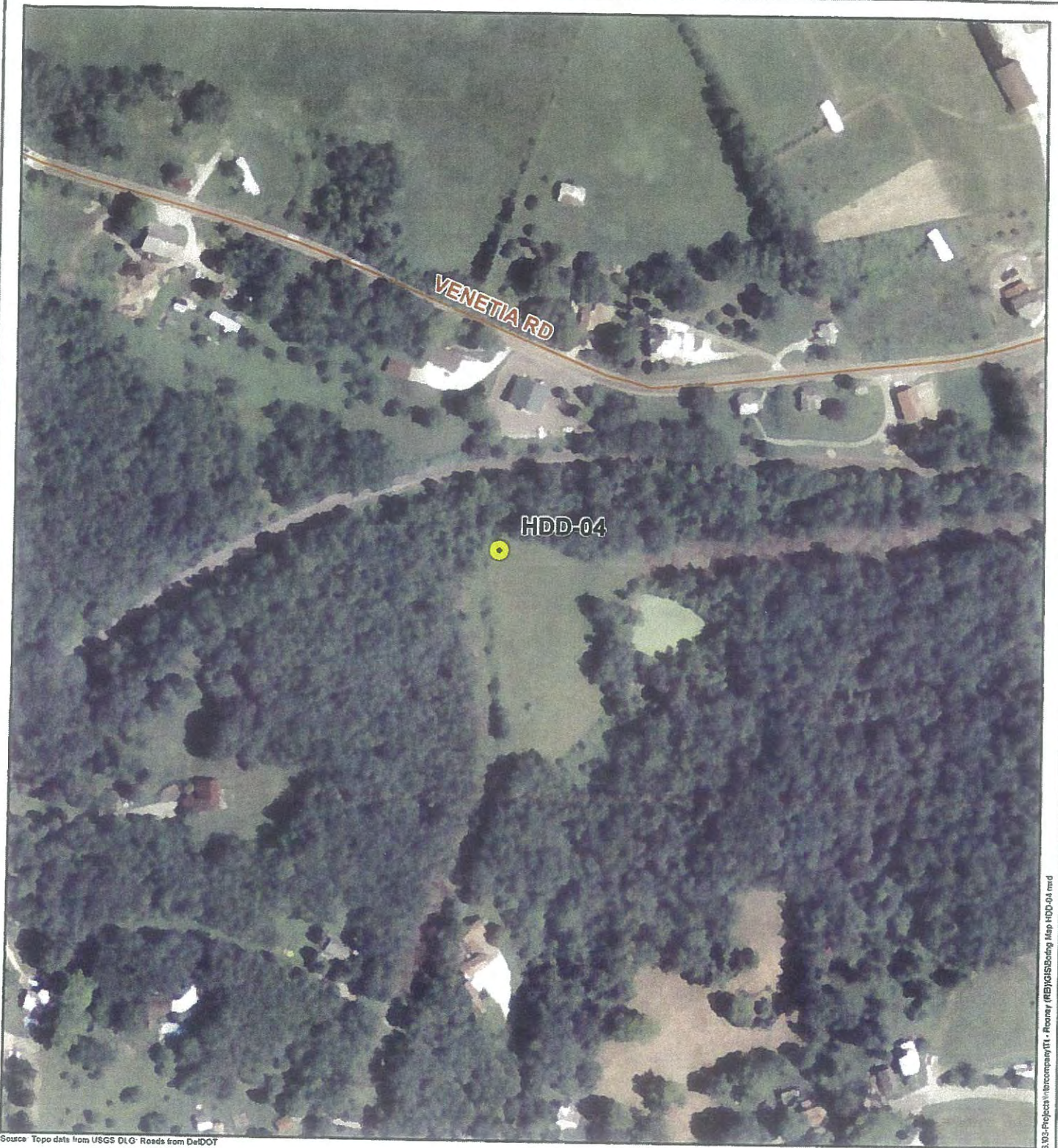
**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
ALLEGHENY VALLEY RR  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'      DWG. NO: PA-WA-0127.0000-RR



S:\03-Projects\tricompany\T1 - Rooney (RB)\GIS\Boring Map HDD-04.mxd

Source: Topo data from USGS DLG Roads from DelDOT



**Figure**  
 Boring Location HDD-04  
 Sunoco Mariner East Project  
 Washington County, PA



1 inch = 250 feet

**Tetra Tech, Inc.**  
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 Toll Free: (800) 462-0910  
 www.tetrattech.com

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 240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
 302.738.7551  
 fax: 302.454.5990

# TEST BORING LOG

|   |                                   |  |                        |  |  |
|---|-----------------------------------|--|------------------------|--|--|
| Project Name: SUNOCO MARINER EAST       |                                   |  | Project No.: 103IP2762 |  |  |
| Project Location: WASHINGTON COUNTY, PA |                                   |  | Page 1 of 1            |  |  |
| Test Boring No.: HDD-04                 | Dates(s) Drilled: 09/03/13        |  | Inspector: E. WATT     |  |  |
| Drilling Contractor: HYNES              | Drilling Method: SPT - ASTM D1586 |  | Driller: M. HYNES      |  |  |
| Surface Elevation (ft):                 | Groundwater Depth (ft): 18.0'     |  | Total Depth (ft): 24.0 |  |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS) | Description of Materials  | 6" Increment Blows * |    |    | N  |
|------------|-------------------|------|-------------------|------|-------------|---------------|---|----------------------|----|----|----|
|            | From              | To   | From              | To   |             |               |   |                      |    |    |    |
|            |                   |      | 0.0               | 0.4  |             |               | TOPSOIL (5")  |                      |    |    |    |
| 1          | 3.5               | 5.0  | 0.0               |      | 9           | CL            | LIGHT BROWN SANDY CLAY, TRACE FINE GRAVEL.  | 2                    | 2  | 3  | 5  |
| 2          | 8.5               | 10.0 |                   |      | 3           | CL            | MOTTLED BROWN AND GRAY SILTY CLAY WITH SOME FINE SAND, TRACE FINE GRAVEL. USCS: CL                | 3                    | 3  | 4  | 7  |
|            |                   |      |                   | 12.0 |             |               |   |                      |    |    |    |
| 3          | 13.5              | 15.0 | 12.0              |      | 15          | SM            | BROWN SILTY FINE TO MEDIUM SAND, THIN SEAM OF EITHER LIGNITE OR COAL.                             | 2                    | 2  | 4  | 6  |
| 4          | 17.5              | 19.0 |                   |      | 18          | SM            | MOTTLED LIGHT GRAY TO BROWN SILTY FINE TO MEDIUM SAND, WITH A LITTLE UNWEATHERED SILTSTONE.       | 6                    | 8  | 8  | 16 |
|            |                   |      |                   | 22.0 |             |               |   |                      |    |    |    |
| 5          | 22.5              | 24.0 | 22.0              | 24.0 | 13          |               | LIGHT GRAY WEATHERED SILTSTONE.   | 8                    | 20 | 30 | 50 |
|            |                   |      |                   |      |             |               | AUGER REFUSAL AT 22.5'. OFF-SET BORING 20' EAST AND CONTINUOUSLY DRILLED TO AUGER REFUSAL AT 21'. |                      |    |    |    |
|            |                   |      |                   |      |             |               | WET ON SPOON AT 18'.  |                      |    |    |    |
|            |                   |      |                   |      |             |               | WATER LEVEL THROUGH AUGERS AT 18'.  |                      |    |    |    |
|            |                   |      |                   |      |             |               | CAVED AT 18.5' AT COMPLETION.   |                      |    |    |    |

Notes/Comments:  
Pocket Pentrometer Testing  
 S1: 3.0 TSF

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more  |
| Cobbles   | 3 to 8 in. diameter   |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve<br>Fine (F) ¾ in. to No. 4 sieve  |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

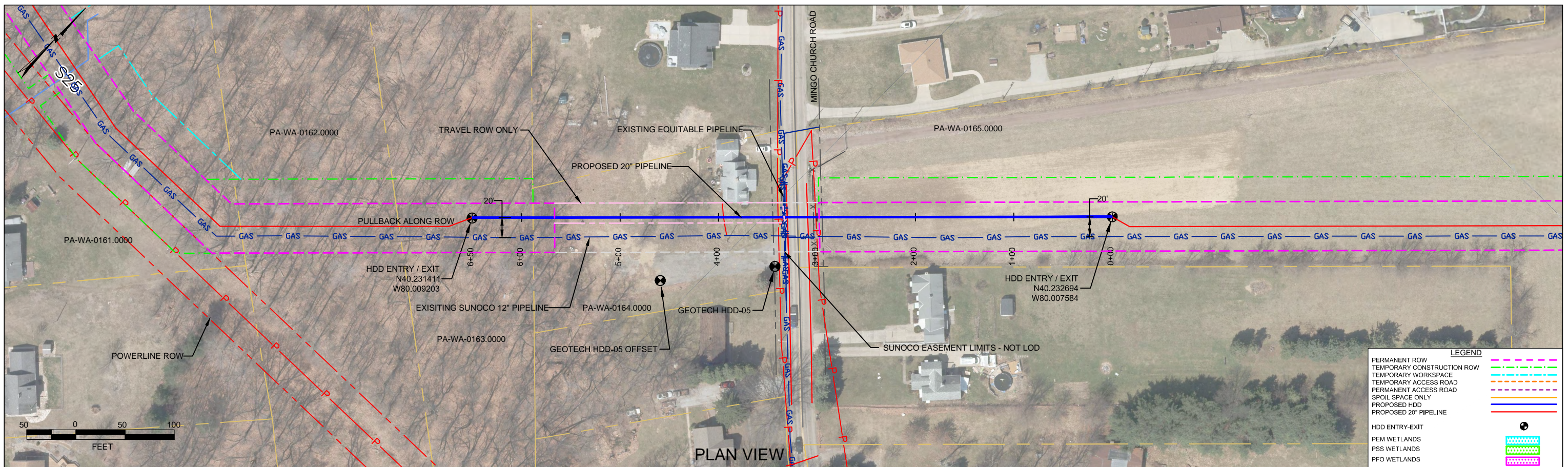
| Major Divisions   |   | Group Symbols  | Typical Descriptions  | Laboratory Classifications  |   |  |  |  |
|---|---|--|---|---|---|--|--|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve)                       | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size)            | Clean gravel<br>(Little or no fines)   | GW<br>Well-graded gravels, gravel-sand mixtures, little or no fines | Determine Percentage of sand and gravel from grain size curve.<br>Depending on Percentage of fines (fraction smaller than No. 200 sieve),<br>coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |  |  |  |
|   |   | GP<br>Poorly graded gravels, gravel-sand mixtures, little or no fines            | Not meeting $C_u$ or $C_c$ requirements for GW                      |   |   |  |  |  |
|   |   | Gravel with fines<br>(Appreciable amount of fines)                               | GM<br>Silty gravels, gravel-sand-silt mixtures                      |   | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols  |  |  |
|   |   |  | GC<br>Clayey gravels, gravel-sand-clay mixtures                     |   | Atterberg limits above A line with $I_p$ greater than 7   |  |  |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)                  | Clean sands<br>(Little or no fines)  | SW<br>Well graded sands, gravelly sands, little or no fines         |   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3             |  |  |  |
|   |   |  | SP<br>Poorly graded sands, gravelly sands, little or no fines       |   | Not meeting $C_u$ or $C_c$ requirements for SW  |  |  |  |
|   |   | Sands with fines<br>(Appreciable amount of fines)                                | SM<br>Silty sands, sand-silt mixtures                               |   | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols  |  |  |
|   |   |  | SC<br>Clayey sands, sand-clay mixtures                              |   | Atterberg limits above A line with $I_p$ greater than 7   |  |  |  |
|   |   |  |   |   |   | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH.<br>When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |  |
|   |   | Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve) | Silt and clays<br>(Liquid limit less than 50)                       |   | ML<br>Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |  |  |  |
| CL<br>Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays |   |  |   |   |   |  |  |  |
| OL<br>Organic silts and organic silty clays of low plasticity   |   |  |   |   |   |  |  |  |
| Silt and Clays (Liquid limit greater than 50)   | MH<br>Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts |  |   |   |   |  |  |  |
|   | CH<br>Inorganic clays of high plasticity, fat clays                                       |  |   |   |   |  |  |  |
|   | OH<br>Organic clays of medium to high plasticity, organic silts                           |  |   |   |   |  |  |  |
| Highly organic soils  | Pt<br>Peat and other highly organic soils   |  |   |   |   |  |  |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0164.0000-RD***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 320 feet from the southern edge of Mingo Church Road and will enter/exit 300 feet from the northern edge. The drill will cross 36 feet below the road. The 20" drill will parallel the existing ME1 12" pipeline drill beneath the road. Geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate below 10 feet is sandstone rock. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



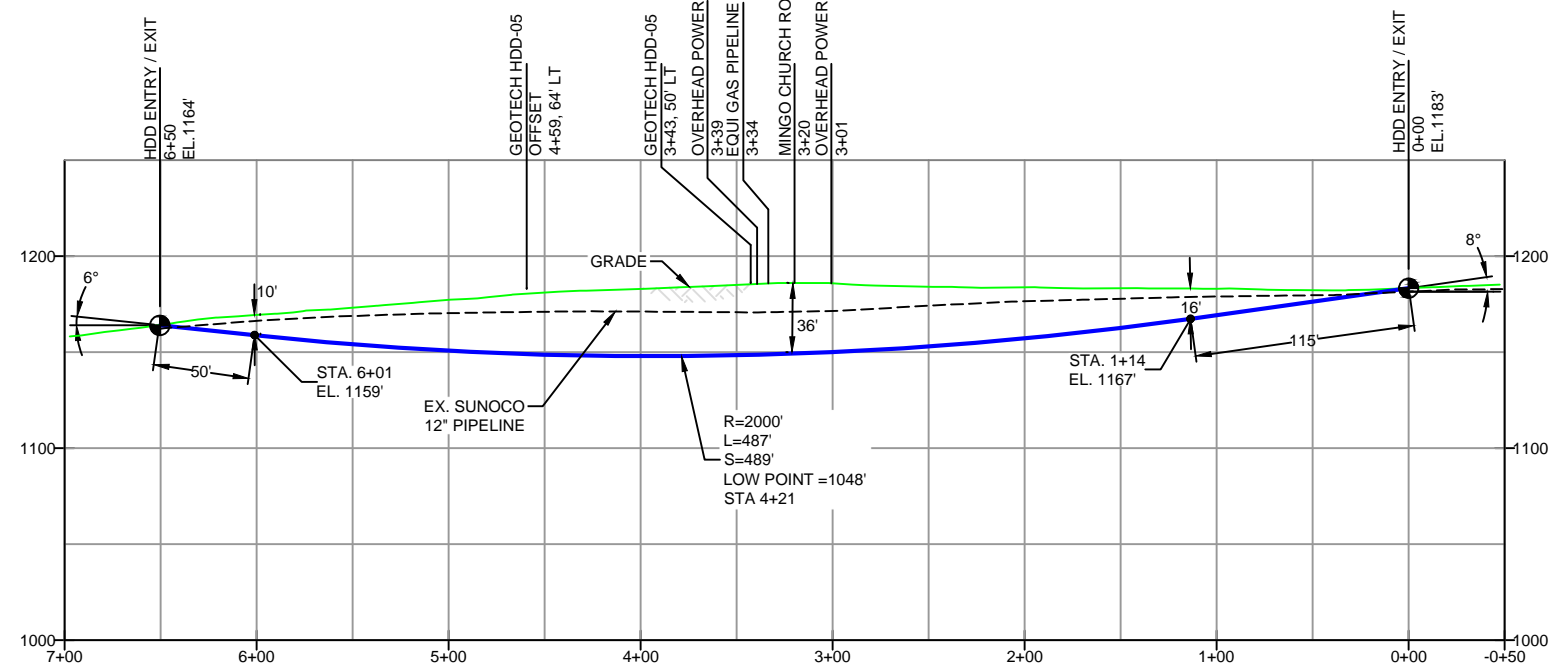
WASHINGTON COUNTY, PENNSYLVANIA - UNION TOWNSHIP  
S1B-0110

PROFILE VIEW

**GEOTECH HDD-05**

- NG EL. 1186'
- CL/SC (0.0' - 7.6')
- COMPLETION DEPTH EL. 1178.4'

NOTE: REFER TO TEST BORING LOG HDD-05 FOR COMPLETE SOIL MATERIAL DESCRIPTION



DESIGN AND CONSTRUCTION:

1. CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
2. THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
3. DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
4. CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L=): 650'  
HDD PIPE LENGTH (S=): 654'  
16" x 0.438" W.T., X-70, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)
5. INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
6. INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
7. PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
8. CARRIER PIPE NOT ENCASED.
9. PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
11. SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
12. SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
13. SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

- NOTES**
1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
  2. STATIONING IS BASED ON HORIZONTAL DISTANCES.
  3. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
  4. CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
  5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |        | REVISIONS   |  |             |    |      |     |      |     |      |
|--------------|--------|-------------|--|-------------|----|------|-----|------|-----|------|
| ES-1.53      | TO     | ES-1.53     | EROSION & SEDIMENT PLAN                      |             |    |      |     |      |     |      |
| SHEET 32     | TO     | SHEET 32    | AERIAL SITE PLAN                             |             |    |      |     |      |     |      |
|              |        | EP2         | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |             |    |      |     |      |     |      |
|              |        | EP1         | REVISED PER PADEP COMMENTS                   |             |    |      |     |      |     |      |
|              |        | EP          |  |             |    |      |     |      |     |      |
| DWG NO       | DWG NO | DESCRIPTION | NO.  | DESCRIPTION | BY | DATE | CHK | DATE | APP | DATE |

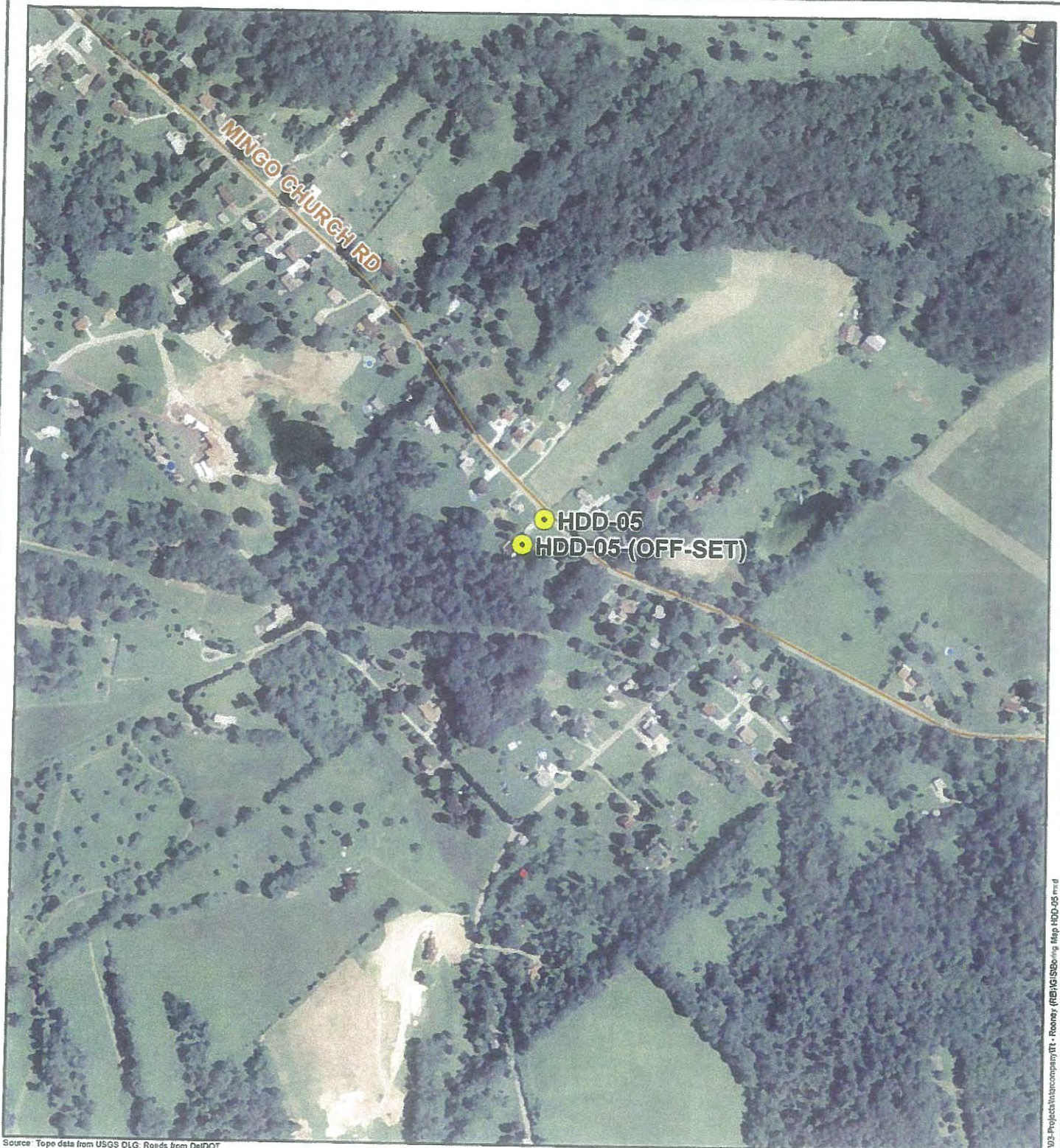
**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
MINGO CHURCH ROAD  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'    DWG. NO: PA-WA-0164.0000-RD



Source: Topo data from USGS DLG, Roads from DelDOT

S:\02-Project\aircompany\T1 - Rooney (RE\MGIS\Boring Map HDD-05.mxd



**Figure**  
**Boring Location HDD-05**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

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 302.738.7651  
 fax: 302.454.5988

# TEST BORING LOG

|   |          |                         |                        |                   |           |
|---|----------|-------------------------|------------------------|-------------------|-----------|
| Project Name: SUNOCO MARINER EAST       |          |                         | Project No.: 103IP2762 |                   |           |
| Project Location: WASHINGTON COUNTY, PA |          |                         | Page 1 of 1            |                   |           |
| Test Boring No.:                        | HDD-05   | Dates(s) Drilled:       | 06/10/13               | Inspector:        | E. WATT   |
| Drilling Contractor:                    | CONNELLY | Drilling Method:        | SPT - ASTM D1586       | Driller:          | T. REDMAN |
| Surface Elevation (ft):                 |          | Groundwater Depth (ft): | Not Encountered        | Total Depth (ft): | 7.6       |

| Sample No. | Sample Depth (ft) |     | Strata Depth (ft) |     | Recov. (in) | Strata (USCS) | Description of Materials   | 6" Increment Blows * |    |    | N  |
|------------|-------------------|-----|-------------------|-----|-------------|---------------|--|----------------------|----|----|----|
|            | From              | To  | From              | To  |             |               |  |                      |    |    |    |
| 1          | 3.5               | 5.0 | 0.0               |     | 11          | CL/SC         | MOTTLED LIGHT BROWN TO MEDIUM BROWN SILTY CLAY AND FINE SAND. TRACE MICA.  | 4                    | 11 | 47 | 58 |
|            |                   |     |                   | 4.5 |             |               | WEATHERED AND BRITTLE SILTSTONE OR SANDSTONE.  |                      |    |    |    |
|            |                   |     |                   | 4.5 |             |               | SANDSTONE  |                      |    |    |    |
| 2          | 7.5               | 7.6 |                   | 7.6 | <1"         |               |  | 50/1"                |    |    |    |
|            |                   |     |                   |     |             |               | AUGER REFUSAL ENCOUNTERED AT 7.5'.   |                      |    |    |    |
|            |                   |     |                   |     |             |               | OFF-SITE BORING 115' TO THE SOUTHWEST (DOWN THE DRIVE WAY) AND CONTINUOUSLY DRILLED TO AUGER REFUSAL AT 6.5'. REFUSAL MATERIAL OBSERVED TO BE SANDSTONE. |                      |    |    |    |

Notes/Comments:  
Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.



# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more  |
| Cobbles   | 3 to 8 in. diameter   |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve<br>Fine (F) ¾ in. to No. 4 sieve  |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

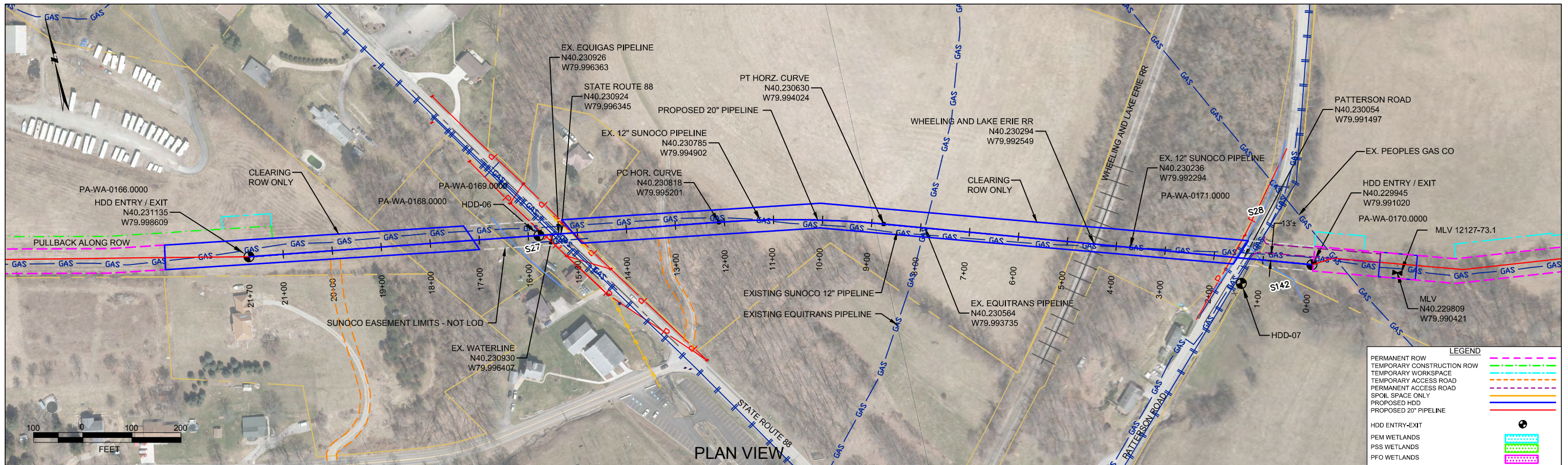
| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |   |   |  |
|---|--|---|---|---|---|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |   |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)       | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |   |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |   |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions  | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                     | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |   |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |   |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |   |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                  | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |   |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |   |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |   |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |   |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0171.0000-RR (S27, S28, S142)***

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 530 feet from the western edge of a Stream 27 (S27) and enter/exit 1,650 feet from the eastern edge. The horizontal directional drill will enter/exit 2,020 feet from the western edge of Stream 28 (S28) and enter/exit 160 feet from the eastern edge. The drill will enter/exit 2,080 feet from the western edge of Stream 142 (S142) and will enter/exit 100 feet from the eastern edge. The drill will cross below S27 at more than 50 feet, and S28 and S142 at about 10 feet. The 20" drill will closely follow the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical reports the primary substrate at the crossings is either rock with limestone and shale (S28) or medium sand with silty clay (S28, S142). Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



PROFILE VIEW

WASHINGTON COUNTY, PENNSYLVANIA - UNION TOWNSHIP  
S1B-0120

**GEOTECH HDD-06**

- NG EL. 978'
- CL (0.0' - 6.0')
- COMPLETION DEPTH EL. 972'

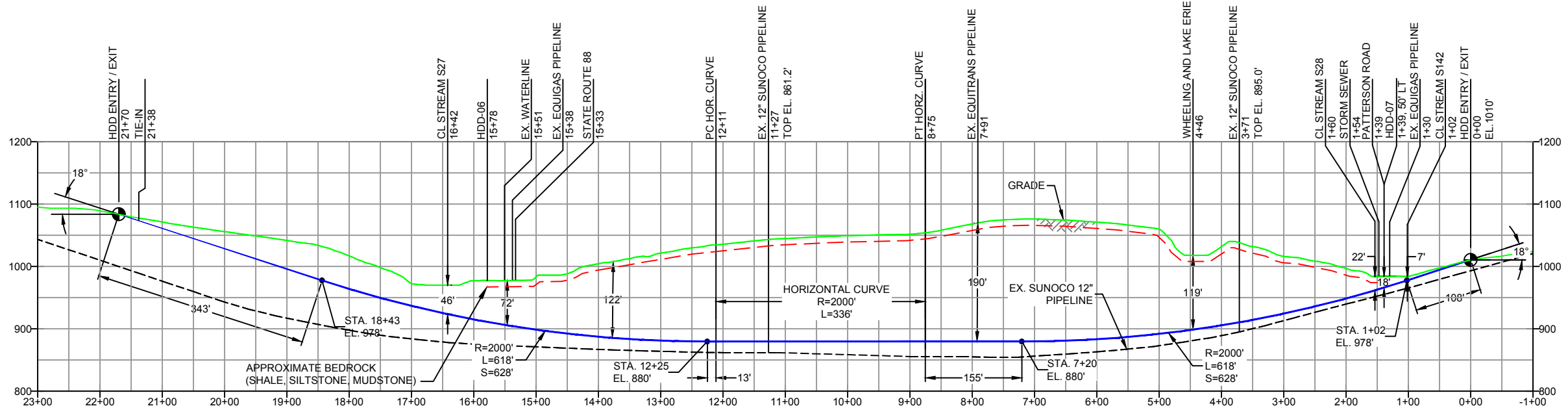
NOTE: REFER TO TEST BORING LOG HDD-06 FOR COMPLETE SOIL MATERIAL DESCRIPTION

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**GEOTECH HDD-07**

- NG EL. 984'
- SM (0.0' - 7.0')
- CL/SC (7.0' - 12.0')
- COMPLETION DEPTH EL. 972'

NOTE: REFER TO TEST BORING LOG HDD-07 FOR COMPLETE SOIL MATERIAL DESCRIPTION



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L-): 2170'  
HDD PIPE LENGTH (S-): 2212'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - PIPELINE AND CROSSING TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LAST APPROVED AMERICAN RAILWAY ENGINEERING AND MAINTENANCE OF WAY ASSOCIATION SPECIFICATIONS FOR PIPELINES CONVEYING FLAMMABLE AND NON-FLAMMABLE SUBSTANCES.
  - BLASTING NOT PERMITTED.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

NOTES

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

REF. DRAWING

|          |    |          |                         |
|----------|----|----------|-------------------------|
| ES-1.55  | TO | ES-1.56  | EROSION & SEDIMENT PLAN |
| SHEET 32 | TO | SHEET 33 | AERIAL SITE PLAN        |

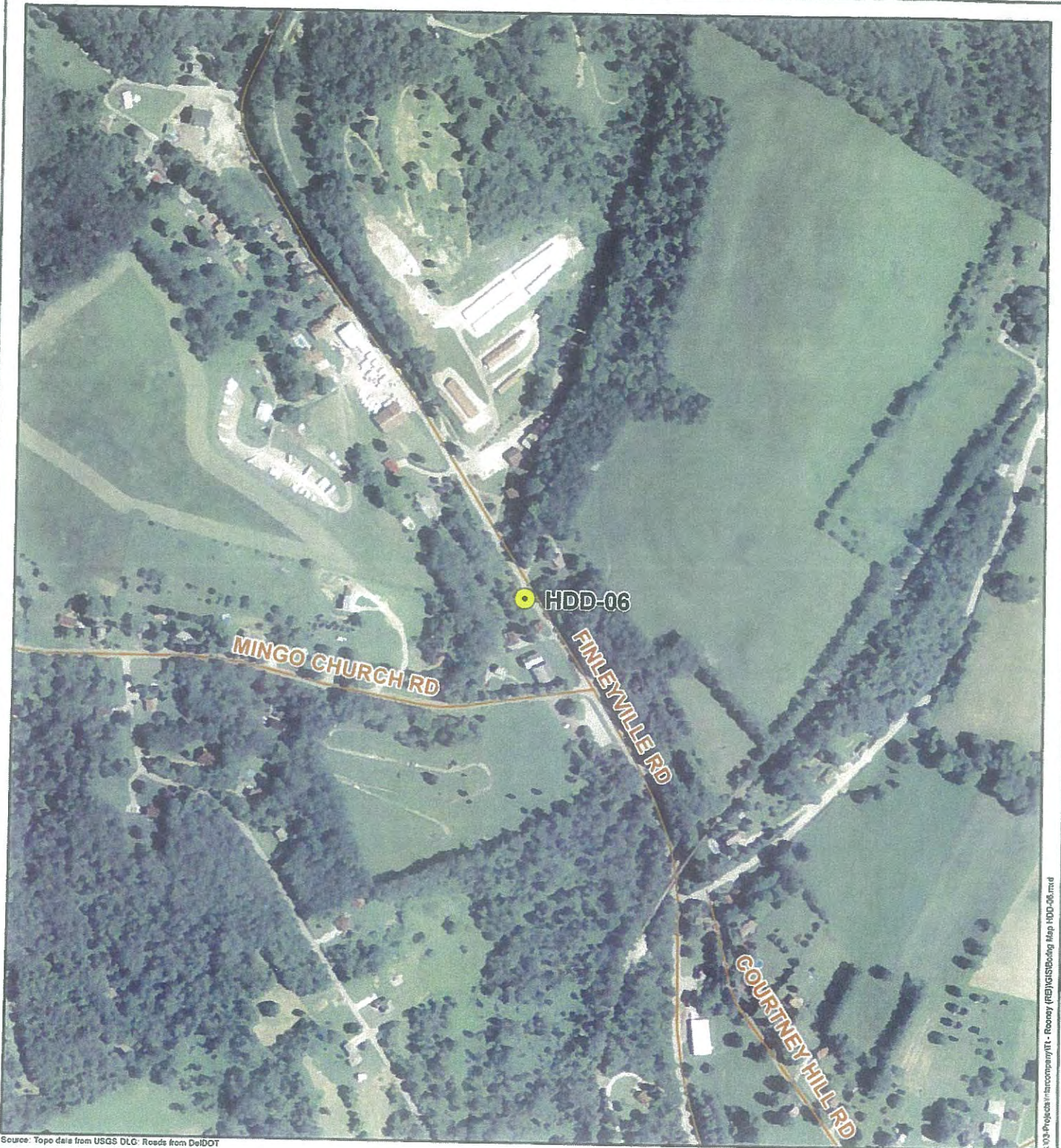
REVISIONS

|        |  |             |          |             |          |             |          |
|--------|--|-------------|----------|-------------|----------|-------------|----------|
| EP2    | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 | MRS         | 09/30/16 | RMB         | 09/30/16 | AAW         | 09/30/16 |
| EP1    | REVISED PER PADEP COMMENTS                   | MRS         | 05/17/16 | RMB         | 05/17/16 | AAW         | 05/17/16 |
| EP     |  | DLM         | 03/15/16 | RMB         | 03/15/16 | AAW         | 03/15/16 |
| DWG NO | DWG NO                                       | DESCRIPTION | NO.      | DESCRIPTION | NO.      | DESCRIPTION | NO.      |

SUNOCO PIPELINE, L.P.

20-INCH HORIZONTAL DIRECTIONAL DRILL  
WHEELING AND LAKE ERIE RR  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=200' DWG. NO. PA-WA-0171.0000-RR



Source: Topo data from USGS DLG; Roads from DelDOT

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**Figure**  
**Boring Location HDD-06**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

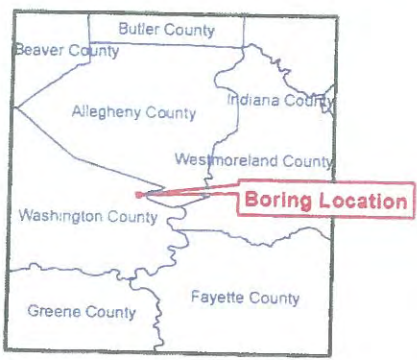
**Tt** Tetra Tech, Inc.  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
 www.tetrattech.com

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Source: \*Topo data from USGS DLG Roads from DelDOT

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**Figure**  
**Boring Location HDD-07**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

**Tetra Tech, Inc.**  
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[www.tetrattech.com](http://www.tetrattech.com)

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# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more  |
| Cobbles   | 3 to 8 in. diameter   |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve<br>Fine (F) ¾ in. to No. 4 sieve  |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

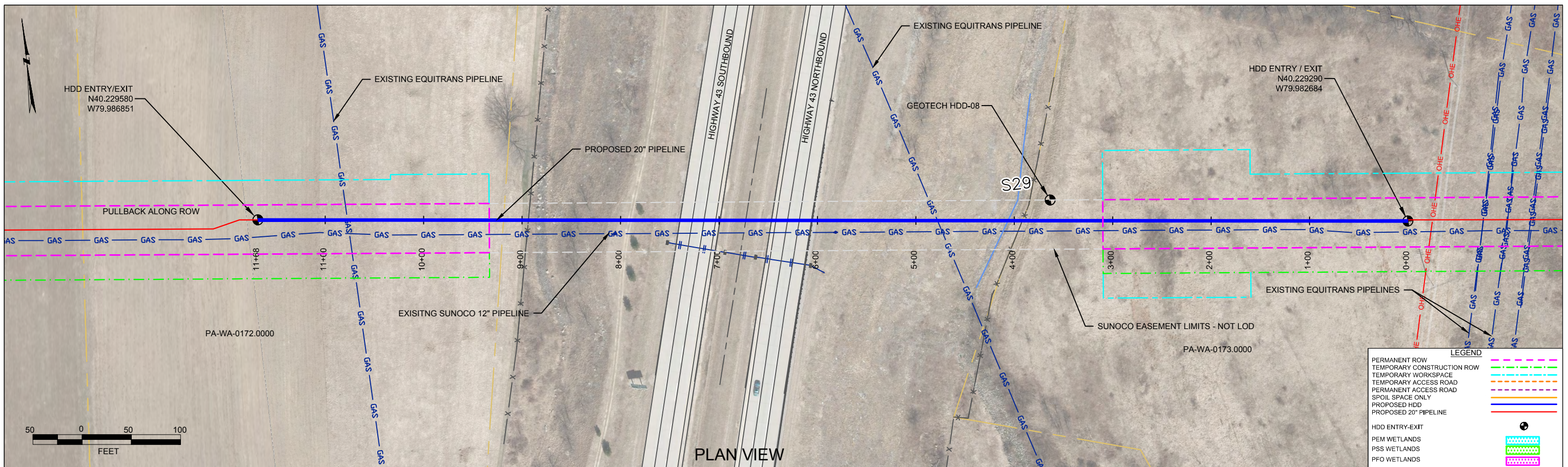
| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |   |   |  |
|---|--|---|---|---|---|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |   |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)       | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |   |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |   |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions  | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                     | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |   |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |   |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |   |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                  | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |   |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |   |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |   |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |   |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0172.0000-RD (S29)***

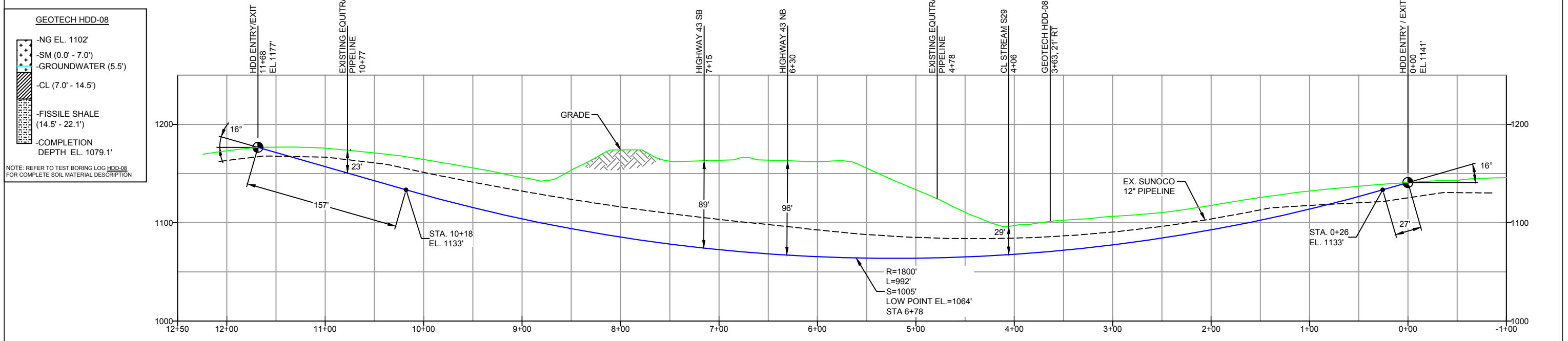
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 430 feet from the western edge of Highway 43 and enter/exit 600 feet from the eastern edge. The horizontal directional drill will enter/exit 760 feet from the western edge of stream 29 (S29) and enter/exit 410 feet from the eastern edge. The drill will cross below the highway at 90 feet and the stream at 30 feet. The 20" drill will parallel the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate at the crossing depth is bedrock with clays and shales above. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



WASHINGTON COUNTY, PENNSYLVANIA - UNION TOWNSHIP S1B-0130

### PROFILE VIEW



- DESIGN AND CONSTRUCTION:**
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
HDD HORZ. LENGTH (L): 1168'  
HDD PIPE LENGTH (S): 1189'  
20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE OR ENGINEER APPROVED EQUAL)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
  - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
  - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

**NOTES**

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |               | REVISIONS               |  |
|--------------|---------------|-------------------------|--|
| ES-1.57      | TO ES-1.58.01 | EROSION & SEDIMENT PLAN |  |
| SHEET 33     | TO SHEET 35   | AERIAL SITE PLAN        |  |
|              |               | EP2                     | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |               | EP1                     | REVISED PER PADEP COMMENTS                   |
|              |               | EP                      |  |
| DWG NO       | DWG NO        | DESCRIPTION             | NO.  |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
(303) 792-5911

**SUNOCO PIPELINE, L.P.**

20-INCH HORIZONTAL DIRECTIONAL DRILL  
HIGHWAY 43  
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=100'      DWG. NO: PA-WA-0172.0000-RD



Source: Topo data from USGS DLG. Roads from DelDOT

S:\US-Projects\withcompany\IT - Rooney (RE)\GIS\Boring Map HDD-08.mxd



**Figure**  
**Boring Location HDD-08**  
**Sunoco Mariner East Project**  
**Washington County, PA**



1 inch = 500 feet

**Tetra Tech, Inc.**  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
[www.tetrattech.com](http://www.tetrattech.com)

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**TETRA TECH**  
 240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
 302.730.7551  
 fax: 302.454.5988

# TEST BORING LOG

|   |                                   |                                   |                        |                        |  |
|---|-----------------------------------|-----------------------------------|------------------------|------------------------|--|
| Project Name: SUNOCO MARINER EAST       |                                   |                                   | Project No.: 103IP2762 |                        |  |
| Project Location: WASHINGTON COUNTY, PA |                                   |                                   | Page 1 of 1            |                        |  |
| Test Boring No.: HDD-08                 | Dates(s) Drilled: 06/26/13        |                                   | Inspector: E. WATT     |                        |  |
| Drilling Contractor: CONNELLY           | Drilling Method: SPT - ASTM D1586 |                                   | Driller: K. Kersh      |                        |  |
| Surface Elevation (ft):                 |                                   | Groundwater Depth (ft): SEE BELOW |                        | Total Depth (ft): 22.5 |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS) | Description of Materials   | 6" Increment Blows *   |       |    | N     |
|------------|-------------------|------|-------------------|------|-------------|---------------|--|--|-------|----|-------|
|            | From              | To   | From              | To   |             |               |  |  |       |    |       |
| 1          | 3.5               | 5.0  | 0.0               |      | 16          | SM            | LIGHT BROWN TO ORANGE BROWN FINE TO MEDIUM SAND WITH SOME CLAY, TRACE F-GRAVEL.  | 5  | 7     | 11 | 18    |
|            |                   |      |                   | 7.0  |             |               |  |  |       |    |       |
| 2          | 8.5               | 10.0 | 7.0               |      | 10          | CL            | MOTTLED ORANGE, BROWN AND GRAY SILTY CLAY WITH SOME FINE SAND, TRACE FINE GRAVEL. USCS: CL   | 3  | 4     | 4  | 8     |
|            |                   |      |                   |      |             |               |  | GRAY, BROWN AND YELLOWISH BROWN SILTY CLAY, TRACE FINE SAND. | 11    | 15 | 50/5" |
| 3          | 13.5              | 15.0 |                   |      | 14          | FS            | HIGHLY WEATHERED FISSILE SHALE.  |  |       |    |       |
|            |                   |      |                   | 14.5 |             |               |  | GRAY FISSILE SHALE.  | 50/2" |    |       |
| 4          | 18.5              | 18.7 |                   |      | 2           |               |  |  |       |    |       |
| 5          | 22.0              | 22.1 |                   | 22.1 | 1           |               | GRAY FISSILE SHALE.  | 50/1"  |       |    |       |
|            |                   |      |                   |      |             |               | AUGER REFUSAL ENCOUNTERED AT 22.0'.  |  |       |    |       |
|            |                   |      |                   |      |             |               | WET ON SPOON AT 9.5'.  |  |       |    |       |
|            |                   |      |                   |      |             |               | WATER LEVEL THROUGH AUGERS AT 5.5'.  |  |       |    |       |
|            |                   |      |                   |      |             |               |  |  |       |    |       |
|            |                   |      |                   |      |             |               | OFF-SET BORING 32' TO THE NORTH, AND CONTINUOUSLY DRILLED TO AN AUGER REFUSAL DEPTH OF 22.5 FEET. NO WATER WAS ENCOUNTERED WITHIN THIS BOREHOLE. |  |       |    |       |

Notes/Comments:  
Pocket Penetrometer Testing  
 S1: >4 TSF  
 S2: 2.75 TSF  
 FS: FISSILE SHALE

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
| Medium Dense   | 11 to 30          |
| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
| Boulders  | 8 in. diameter or more                                  |
| Cobbles   | 3 to 8 in. diameter                                     |
| Gravel    | Coarse (C) 3 in. to ¾ in. sieve                         |
|           | Fine (F) ¾ in. to No. 4 sieve                           |
| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)     |
|           | Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm) |
|           | Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm)   |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)                    |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
|-----------------------------|-------------------------|
| None to Slight              | 0 - 4                   |
| Slight                      | 5 - 7                   |
| Medium                      | 8 - 22                  |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

**\*N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

**Groundwater** observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

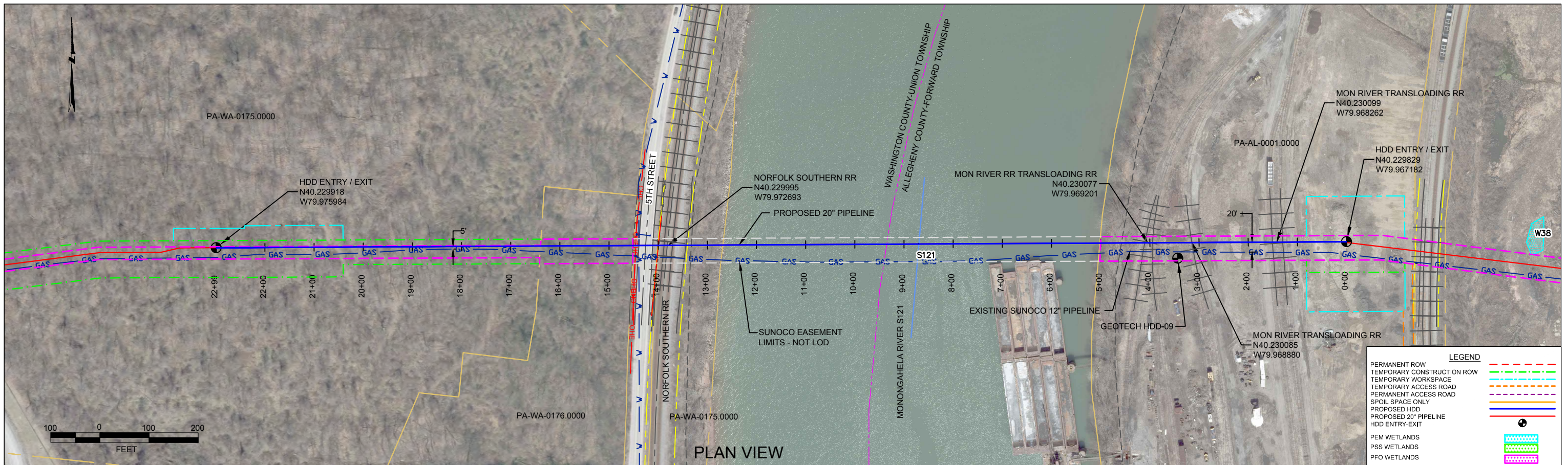
| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |   |   |  |
|---|--|---|---|---|---|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |   |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4  | Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)       | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |   |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4  | Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                | Atterberg limits above A line with $I_p$ greater than 7   |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |   |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions  | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                     | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |   |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |   |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |   |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                  | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |   |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |   |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |   |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |   |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.

***HDD PA-WA-0176.0000-RD (S121)***

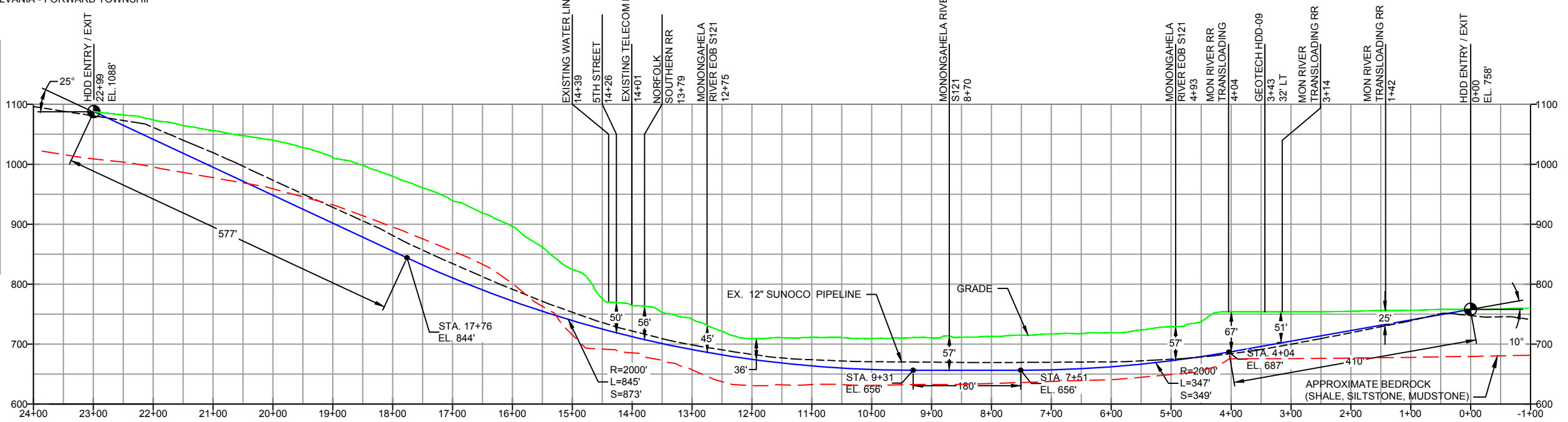
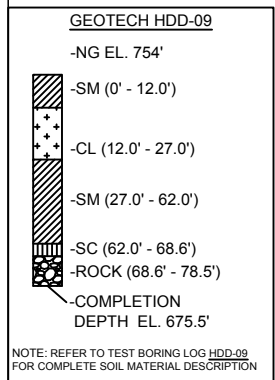
Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 1,000 feet from the western edge of the Monongahela River (S121) and enter/exit 530 feet from the eastern edge and cross at depths greater than 40 feet. The 20" drill will parallel the existing ME1 12" pipeline drill. The geotechnical results from the previous drill, as well as other data points, were used to determine the entry/exit angles, and depths to pass through the best substrates while maintaining the pipe integrity (e.g., no large bends). According to the geotechnical report the primary substrate at the crossing (S121) is estimated to be silt and clay. Based on the geotechnical report, the drill profile, and the previous drill data minimal inadvertent returns are expected.



WASHINGTON COUNTY, PENNSYLVANIA - UNION TOWNSHIP  
 ALLEGHENY COUNTY, PENNSYLVANIA - FORWARD TOWNSHIP  
 S1B-0140

PROFILE VIEW



- DESIGN AND CONSTRUCTION:
- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
  - THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
  - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
  - CROSSING PIPE SPECIFICATION:  
 HDD HORZ. LENGTH (L=): 2299'  
 HDD PIPE LENGTH (S=): 2389'  
 20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW  
 COATING: 14-16 MILS FBE WITH 40 MILS MIN. ARO (POWERCRETE R95)
  - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50 (HOOP STRESS)).
  - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
  - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
  - CARRIER PIPE NOT ENCASED.
  - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
  - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
  - PIPELINE AND CROSSING TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LAST APPROVED AMERICAN RAILWAY ENGINEERING AND MAINTENANCE OF WAY ASSOCIATION SPECIFICATIONS FOR PIPELINES CONVEYING FLAMMABLE AND NON-FLAMMABLE SUBSTANCES.
  - BLASTING NOT PERMITTED.
  - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

NOTES

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

| REF. DRAWING |        | REVISIONS   |  |
|--------------|--------|-------------|--|
| ES-1.59      | TO     | ES-1.01     | EROSION & SEDIMENT PLAN                      |
| SHEET 35     | TO     | SHEET 36    | AERIAL SITE PLAN                             |
|              |        | EP2         | REVISED PER PADEP COMMENTS RECEIVED 09-06-16 |
|              |        | EP1         | REVISED PER PADEP COMMENTS                   |
|              |        | EP          |  |
| DWG NO       | DWG NO | DESCRIPTION | NO.  |

**Sunoco Logistics Partners L.P.**

**TETRA TECH ROONEY**  
 (303) 792-5911

**SUNOCO PIPELINE, L.P.**

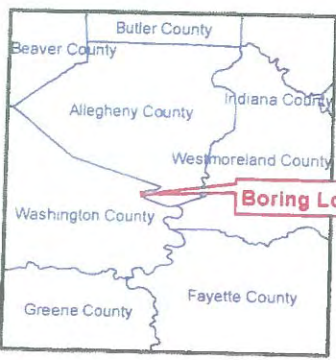
20-INCH HORIZONTAL DIRECTIONAL DRILL  
 NORFOLK SOUTHERN RR, MONONGAHELA RIVER  
 PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=200'    DWG. NO: PA-WA-0176.0000-RR



Source: Topo data from USGS DLG; Roads from DelDOT

S:\05-Projects\14\compaq\m11 - Rooney (WB)\GIS\Boring Map HDD-09.mxd



**Figure**  
**Boring Location HDD-09**  
**Sunoco Mariner East Project**  
**Allegheny County, PA**



1 inch = 500 feet



**Tetra Tech, Inc.**  
 Phone: (302) 738-7551  
 Toll Free: (800) 462-0910  
[www.tetrattech.com](http://www.tetrattech.com)

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**TETRA TECH**  
 240 Continental Drive, Suite 200  
 Newark, Delaware 19713  
 302.733.7551  
 fax: 302.454.5908

# TEST BORING LOG

|  |                                   |  |                        |  |  |
|--|-----------------------------------|--|------------------------|--|--|
| Project Name: SUNOCO MARINER EAST      |                                   |  | Project No.: 103IP2762 |  |  |
| Project Location: ALLEGHANY COUNTY, PA |                                   |  | Page 1 of 1            |  |  |
| Test Boring No.: HDD-09                | Dates(s) Drilled: 09/09/13        |  | Inspector: E. WATT     |  |  |
| Drilling Contractor: CONNELLY          | Drilling Method: SPT - ASTM D1586 |  | Driller: K. KERCH      |  |  |
| Surface Elevation (ft):                | Groundwater Depth (ft): 24.5      |  | Total Depth (ft): 78.5 |  |  |

| Sample No. | Sample Depth (ft) |      | Strata Depth (ft) |      | Recov. (in) | Strata (USCS)                             | Description of Materials   | 6" Increment Blows *   |    |    | N  |    |
|------------|-------------------|------|-------------------|------|-------------|---|--|--|----|----|----|----|
|            | From              | To   | From              | To   |             |   |  |  |    |    |    |    |
| 1          | 3.5               | 5.0  | 0.0               |      | 6           | SM  | LIGHT BROWN FINE TO MEDIUM SAND WITH A TRACE OF SANDSTONE GRAVEL, AND A LITTLE SILT (HISTORIC FILL).                         | 3  | 4  | 5  | 9  |    |
| 2          | 8.5               | 10.0 |                   | 12.0 | 2           |   | LIGHT BROWN FINE TO MEDIUM SAND WITH A TRACE OF SANDSTONE GRAVEL, AND A LITTLE SILT (HISTORIC FILL).                         | 3  | 3  | 5  | 8  |    |
| 3          | 13.5              | 15.0 | 12.0              |      | 10          | CL  | MOTTLED BROWN AND GRAY SILTY CLAY, TRACE MICA, TRACE TO LITTLE SILT. USCS: CL  | 2  | 3  | 3  | 6  |    |
| 4          | 18.5              | 20.0 |                   |      | 18          |   | BROWN SILTY CLAY WITH SOME FINE SAND, TRACE MICA.  | 3  | 3  | 3  | 6  |    |
| 5          | 23.5              | 25.0 |                   | 27.0 | 18          | SM  | BROWN SANDY CLAY, TRACE MICA.  | 1  | 1  | 2  | 3  |    |
| 6          | 28.5              | 30.0 | 27.0              |      | 1           |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT.  | 2  | 3  | 3  | 6  |    |
| 7          | 33.5              | 35.0 |                   |      | 18          |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT.  | 3  | 3  | 6  | 9  |    |
| 8          | 38.5              | 40.0 |                   |      | 18          |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT. 2" BLACK SEAM PRESENT (LIGNITE?)   | 1  | 3  | 4  | 7  |    |
| 9          | 43.5              | 45.0 |                   |      | 18          |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT.  | 3  | 4  | 7  | 11 |    |
| 10         | 48.5              | 50.0 |                   |      | 18          |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT. 2" BLACK SEAM PRESENT (LIGNITE?).  | 3  | 6  | 7  | 13 |    |
| 11         | 53.5              | 55.0 |                   |      | 18          |   | BROWN FINE TO MEDIUM SAND WITH A LITTLE SILT, WITH A LITTLE FINE GRAVEL, INTERLAYERED WITH "LIGNITE?".                       | 3  | 17 | 30 | 47 |    |
| 12         | 58.5              | 60.0 |                   |      | 17          |   | BROWN FINE TO COARSE SAND WITH A LITTLE SILT, WITH A LITTLE FINE GRAVEL.   | 8  | 16 | 17 | 33 |    |
|            |                   |      | 62.0              |      |             |   |  |  |    |    |    |    |
| 13         | 63.5              | 65.0 | 62.0              |      | 18          |   | SC   | DECOMPOSED ROCK WEATHERED TO A MULT-COLORED FINE TO MEDIUM SAND, WITH SOME CLAY. | 19 | 32 | 50 | 82 |
| 14         | 68.5              | 68.6 |                   | 68.6 | <1          | LIGHT GRAY PARTIALLY WEATHERED SILTSTONE. |  | 50/1"  |    |    |    |    |
|            |                   |      |                   |      |             |   | AUGER REFUSAL AT 68.5'.  |  |    |    |    |    |
|            |                   |      |                   |      |             |   | ROCK CORING  |  |    |    |    |    |
| RUN 1      | 68.5              | 73.5 | 68.5              |      |             | ROCK*                                     | ROCK CORING: 88% RECOVERY, 37% RQD   |  |    |    |    |    |
| RUN 2      | 73.5              | 78.5 |                   | 78.5 |             |   | ROCK CORING: 95% RECOVERY, 58% RQD   |  |    |    |    |    |
|            |                   |      |                   |      |             |   | *PREDOMINATELY GRAY AND GREENISH GRAY SILTSTONE, WITH A THIN SEAM OF CALCEROUS CLAYSTONE, AND A THIN SEAM OF GRAY LIMESTONE. |  |    |    |    |    |

Notes/Comments:  
Pocket Penetrometer Testing  
 S3: 0.75 TSF  
 S4: 0.75 TSF  
 S13: > 4 TSF

WET ON SPOON AT 28.0'.  
 WATER LEVEL THROUGH AUGERS AT 24.5'.

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

\* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.  
 N: Number of blows to drive spoon from 6" to 18" interval.

# FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

## GRANULAR SOILS

(Sand, Gravel & Combinations)

| <u>Density</u> | <u>N (blows)*</u> |
|----------------|-------------------|
| Very Loose     | 5 or less         |
| Loose          | 6 to 10           |
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| Dense          | 31 to 50          |
| Very Dense     | 51 or more        |

### Particle Size Identification

|           |   |
|-----------|---|
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| Cobbles   | 3 to 8 in. diameter   |
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| Sand      | Coarse (C) No. 4 to No. 10 sieve<br>(4.75mm-2.00mm)<br>Medium (M) No. 10 to No. 40 sieve<br>(2.00mm – 0.425mm)<br>Fine (F) No. 40 to No. 200 sieve<br>(0.425 – 0.074mm) |
| Silt/Clay | Less Than a No. 200 sieve (<0.074mm)  |

### Relative Proportions

| <u>Description Term</u> | <u>Percent</u> |
|-------------------------|----------------|
| Trace                   | 1 - 10         |
| Little                  | 11 - 20        |
| Some                    | 21 - 35        |
| And                     | 36 - 50        |

## COHESIVE SOILS

(Silt, Clay & Combinations)

| <u>Consistency</u> | <u>N (blows)*</u> |
|--------------------|-------------------|
| Very Soft          | 3 or less         |
| Soft               | 4 to 5            |
| Medium Stiff       | 6 to 10           |
| Stiff              | 11 to 15          |
| Very Stiff         | 16 to 30          |
| Hard               | 31 or more        |

### Plasticity

| <u>Degree of Plasticity</u> | <u>Plasticity Index</u> |
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| Slight                      | 5 - 7                   |
| Medium                      | 8- 22                   |
| High to Very High           | > 22                    |

## ROCK

(Rock Cores)

| <u>Rock Quality Designation (RQD), %</u> | <u>Rock Quality Description</u> |
|--|---------------------------------|
| 0-25                                     | Very Poor                       |
| 25-50                                    | Poor                            |
| 50-75                                    | Fair                            |
| 75-90                                    | Good                            |
| 90-100                                   | Excellent                       |

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**UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]**

| Major Divisions   |  | Group Symbols   | Typical Descriptions  | Laboratory Classifications                                      |  |   |  |
|---|--|---|---|---|--|---|--|
| Coarse Grained Soils<br>(More than half of material is larger than No. 200 sieve) | Gravels<br>(More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravel (Little or no fines)   | GW  | Well-graded gravels, gravel-sand mixtures, little or no fines   | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for GW  |   |  |
|   |  |   | GP  | Poorly graded gravels, gravel-sand mixtures, little or no fines |  |   |  |
|   |  | Gravel with fines (Appreciable amount of fines)   | GM  | Silty gravels, gravel-sand-silt mixtures                        | Atterberg limits below A Line or $I_p$ less than 4<br><br>Atterberg limits above A line with $I_p$ greater than 7<br><br>Limits plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols |   |  |
|   |  |   | GC  | Clayey gravels, gravel-sand-clay mixtures                       |  |   |  |
|   | Sands<br>(More than half of coarse fraction is smaller than No. 4 Sieve)       | Clean sands (Little or no fines)  | SW  | Well graded sands, gravelly sands, little or no fines           | $C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3<br><br>Not meeting $C_u$ or $C_c$ requirements for SW  |   |  |
|   |  |   | SP  | Poorly graded sands, gravelly sands, little or no fines         |  |   |  |
|   |  | Sands with fines (Appreciable amount of fines)  | SM  | Silty sands, sand-silt mixtures                                 | Atterberg limits below A Line or $I_p$ less than 4<br><br>Atterberg limits above A line with $I_p$ greater than 7<br><br>Limits Plotting in hatched zone with $I_p$ between 4 and 7 are borderline cases requiring use of dual symbols |   |  |
|   |  |   | SC  | Clayey sands, sand-clay mixtures                                |  |   |  |
|   |  | Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows:<br><br>Less than 5 percent GW, GP, SW, SP<br>More than 12 percent GM, GC, SM, SC<br>5 to 12 percent Borderline cases requiring dual symbols <sup>(1)</sup> |   |   |  |   |  |
|   |  | Major Divisions   |   | Group Symbols   | Typical Descriptions   | For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$ , $w_L = 60$ gives CH-MH. When $w_L$ is near 50 use CL-CH or ML-MH. Take near as $\pm 2$ percent. |  |
| Fine-grained soils<br>(More than half of material is smaller than No. 200 sieve)  | Silt and clays (Liquid limit less than 50)                                     | ML  | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity |   |  |   |  |
|   |  | CL  | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays                   |   |  |   |  |
|   |  | OL  | Organic silts and organic silty clays of low plasticity   |   |  |   |  |
|   | Silt and Clays (Liquid limit greater than 50)                                  | MH  | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts                                 |   |  |   |  |
|   |  | CH  | Inorganic clays of high plasticity, fat clays   |   |  |   |  |
|   |  | OH  | Organic clays of medium to high plasticity, organic silts   |   |  |   |  |
|   | Highly organic soils   | Pt  | Peat and other highly organic soils   |   |  |   |  |

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.